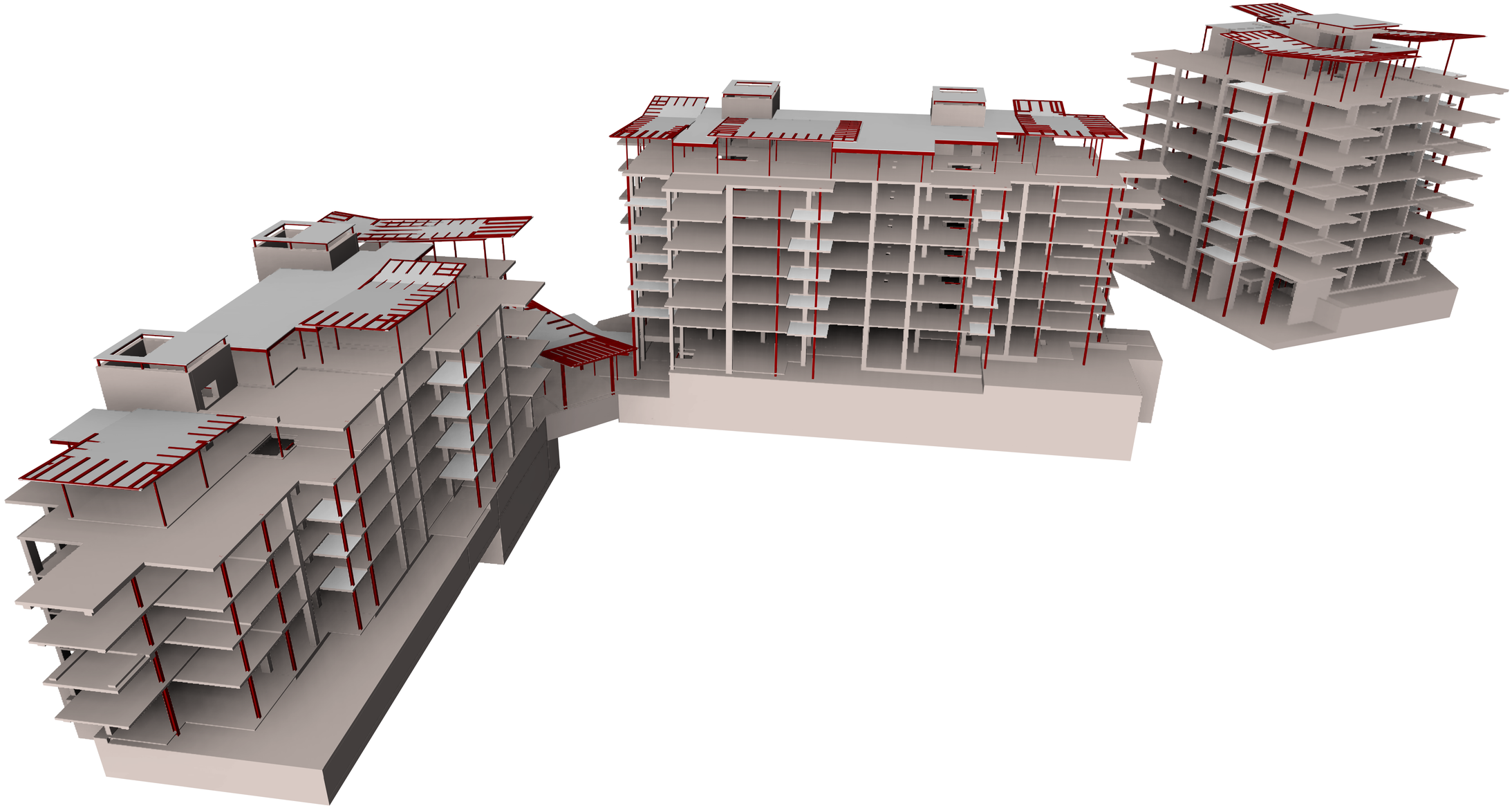

SOMMET BLANC

Magnusson Klemencic Associates

BUILDING PERMIT SET

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project:
SOMMET BLANC - ABC
DEER VALLEY, UTAH

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job no. 20052
date 7/15/2022

revisions:

no.	date	by

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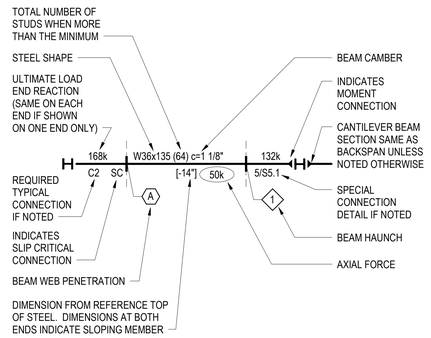
COVER

S0.00

&	AND	L	ANGLE
@	AT	LAB	LABORATORY
°, DEG	DEGREE	LB, #	POUND
ø, DIA	DIAMETER	LF	LINEAL FOOT
#	NUMBER, POUND	LIN	LINEAL, LINEAR
		LL	LIVE LOAD
AB	ANCHOR BOLT	LLBB	LONG LEGS BACK-TO-BACK
ACI	AMERICAN CONCRETE INSTITUTE	LLH	LONG LEG HORIZONTAL
ADDL	ADDITIONAL	LLV	LONG LEG VERTICAL
ADJ	ADJACENT	LOC	LOCATION; LOCATE
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	LONGIT	LONGITUDINAL
		LP	LOW POINT
AGGR	AGGREGATE	LSL	LONG SLOTTED (HOLES)
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LSW	LIGHT GAGE SHEAR WALL
		LTWT	LIGHTWEIGHT
ALT	ALTERNATE	LVL	LEVEL
ALUM	ALUMINUM	LWC	LIGHTWEIGHT CONCRETE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	MAS	MASONRY
APA	AMERICAN PLYWOOD ASSOCIATION	MATL	MATERIAL
APPD	APPROVED	MAX	MAXIMUM
APPROX	APPROXIMATE	MB	MACHINE BOLT
AR	ANCHOR RODS	MC	MISCELLANEOUS CHANNEL
ARCH	ARCHITECTURAL; ARCHITECT	MECH	MECHANICAL
ASSY	ASSEMBLY	MEMB	MEMBRANE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	MEP	MECHANICAL/ ELECTRICAL / PLUMBING
AWS	AMERICAN WELDING SOCIETY	MEZZ	MEZZANINE
		MF	MOMENT FRAME
BAL	BALANCE	MFB	MOMENT FRAME BEAM
BD	BOARD	MFC	MOMENT FRAME COLUMN
BF	BRACED FRAME	MFR	MANUFACTURE; MANUFACTURER
BLDG	BUILDING	MFRG	MANUFACTURING
BLK	BLOCK; BLOCKING	MIN	MINIMUM; MINUTE
BM	BEAM	MISC	MISCELLANEOUS
BMU	BRICK MASONRY UNIT	ML	MATCH LINE
BOS	BOTTOM OF STEEL; BOSOM (WELD)	MO	MASONRY OPENING
BOT	BOTTOM	MS	MECHANICAL SPLICE
BRCG	BRACING		
BRG	BEARING	N	NORTH
BRKT	BRACKET	N-S	NORTH-SOUTH
BSMT	BASEMENT	NF	NEAR FACE
BTWN	BETWEEN	NIC	NOT IN CONTRACT
BU	BUILT-UP	NLT	NAIL LAMINATED TIMBER
		NS	NEAR SIDE
c	CAMBER	NTS	NOT TO SCALE
C	STANDARD CHANNEL	NWC	NORMALWEIGHT CONCRETE
CANT	CANTILEVER		
CC	CENTER TO CENTER	OC	ON CENTER
CG	CENTER OF GRAVITY	OD	OUTSIDE DIAMETER
CIP	CAST-IN-PLACE	OPNG	OPENING
CJ	CONSTRUCTION JOINT	OPP	OPPOSITE (HAND)
CJP	COMPLETE JOINT PENETRATION WELD	OPT	OPTION; OPTIONAL
CL	CENTERLINE	OVS	OVERSIZED (HOLES)
CLR	CLEARANCE; CLEAR	OWJ	OPEN WEB JOIST
CLT	CROSS LAMINATED TIMBER		
CMU	CONCRETE MASONRY UNIT	P	PIPE
COL	COLUMN	PC	PRECAST
COMP	COMPRESSION	PCF	POUNDS PER CUBIC FOOT
CONC	CONCRETE	PCP	PRECAST CONCRETE PANEL
CONFIG	CONFIGURATION	PEN	PENETRATION
CONN	CONNECTION; CONNECT	PERP	PERPENDICULAR
CONST	CONSTRUCTION	PH	PENTHOUSE
CONT	CONTINUE; CONTINUOUS	PJP, PP	PARTIAL JOINT PENETRATION WELD
CONTR	CONTRACTOR	PL	PLATE
COORD	COORDINATE; COORDINATION	PLC	PLACE
CORR	CORRUGATED	PLF	POUNDS PER LINEAL FOOT
CP, CJP	COMPLETE JOINT PENETRATION WELD	PLYWD	PLYWOOD
CTR	CENTER	PP, PJP	PARTIAL JOINT PENETRATION
CTSK	COUNTERSINK; COUNTERSUNK		WELD
CU	CUBIC	PREFAB	PREFABRICATED
		PS	PRESTRESSED
d	PENNY (NAIL)	PSF	POUNDS PER SQUARE FOOT
db	NOMINAL BAR DIAMETER (INCHES)	PSI	POUNDS PER SQUARE INCH
DBA	DEFORMED BAR ANCHOR	PT	POST-TENSIONED
DBL	DOUBLE	PV	PHOTOVOLTAICS
DC	DEMAND CRITICAL WELD	PVC	POLYVINYL CHLORIDE
DEG, °	DEGREE		
DEMO	DEMOLISH; DEMOLITION	R	RADIUS
DEPT	DEPARTMENT	RB	RISER BAR
DET	DETAIL	RC	REINFORCED CONCRETE
DIA, ø	DIAMETER	RCMD	RECOMMEND
DIAG	DIAGONAL	REF	REFERENCE
DIAPH	DIAPHRAGM	REINF	REINFORCE; REINFORCING; REINFORCEMENT
DICA	DRILLED-IN CONCRETE ANCHOR		
DIM	DIMENSION	REQD	REQUIRED
DISC	DISCONTINUED; DISCONTINUOUS	REQT	REQUIREMENT
DL	DEAD LOAD		
DLT	DOWEL LAMINATED TIMBER	S1S	SURFACED ONE SIDE
DN	DOWN	S2S	SURFACED TWO SIDES
DO	DITTO	S4S	SURFACED FOUR SIDES
DWG	DRAWING	S	AMERICAN STANDARD SHAPE; SOUTH
DWL	DOWEL	SB	SPACER BAR; SUPPORT BAR
		SC	SLIP CRITICAL
(E)	EXISTING	SCC	STRUCTURAL CONSULTANT TO THE CONTRACTOR
E	EAST	SCHED	SCHEDULE, SCHEDULED
E-W	EAST-WEST	SDQ	SPECIAL DUCTILE QUALITY
EA	EACH	SECT	SECTION
EF	EACH FACE	SEOR	STRUCTURAL ENGINEER OF RECORD
EJ	EXPANSION JOINT	SFRS	SEISMIC FORCE RESISTING SYSTEM
EL	ELEVATION	SHT	SHEET
ELEC	ELECTRICAL	SHTG	SHEATHING
ELEV	ELEVATOR	SIM	SIMILAR
EMBED	EMBEDDED	SLBB	SHORT LEGS BACK-TO- BACK
ENGR	ENGINEER	SOG	SLAB ON GRADE
EQ	EQUAL; EARTHQUAKE	SOSD	SLAB ON STEEL DECK
EQUIP	EQUIPMENT	SP	SPIRAL
ES	EACH SIDE	SPC	SPACE
ETC	ET CETERA	SPCG	SPACING
EW	EACH WAY	SPEC	SPECIFICATION
EXIST	EXISTING	SQ	SQUARE
EXP	EXPANSION	SSL	SHORT SLOTTED (HOLES)
EXT	EXTERIOR	STD	STANDARD
EXTD	EXTEND; EXTENDED	STIFF	STIFFENER
		STIRR	STIRRUP
F	DEGREES FAHRENHEIT	STL	STEEL
FD	FLOOR DRAIN	STR	STRAIGHT
FDN	FOUNDATION	STRUC	STRUCTURAL
FF	FAR FACE	SUPT	SUPPORT
FFE	FINISH FLOOR ELEVATION	SW	SHEAR WALL
FG	FRICTION GRIP BOLT	SYM	SYMMETRICAL
FIN	FINISH		
FL	FLOOR; FLOOR LINE	T&B	TOP AND BOTTOM
FLG	FLANGE	T&G	TONGUE AND GROOVE
FOS	FACE OF STUD	T&R	TREAD AND RISER
FP	FIREPROOF; FULL PENETRATION	TBD	TO BE DETERMINED
FRMG	FRAMING	TEMP	TEMPERATURE; TEMPORARY
FS	FULL SIZE; FAR SIDE	THK	THICK; THICKNESS
FT	FOOT; FEET	THRU	THROUGH
FTG	FOOTING	TOC	TOP OF CURB; TOP OF CONCRETE
FV	FIELD VERIFY	TOF	TOP OF FOOTING
		TOS	TOP OF STEEL
GA	GAGE, GAUGE	TOW	TOP OF WALL
GALV	GALVANIZED	TRANS	TRANSVERSE
GB	GRADE BEAM	TYP	TYPICAL
GFRC	GLASS FIBER REINFORCED CONCRETE		
GL	GLUED LAMINATED (BEAM)	UB	UNIVERSAL BEAM
GR	GRADE	UBC	UNIFORM BUILDING CODE
GRND	GROUND	UC	UNIVERSAL COLUMN
		UL	UNDERWRITERS' LABORATORY, INC.
H	HORIZONTAL	UNO	UNLESS NOTED OTHERWISE
HEF	HORIZONTAL EACH FACE	UT	ULTRASONIC TEST
HGR	HANGER		
HIF	HORIZONTAL INSIDE FACE	V, VERT	VERTICAL
HOF	HORIZONTAL OUTSIDE FACE	VEF	VERTICAL EACH FACE
HORIZ	HORIZONTAL	VG	VERTICAL GRAIN
HP	HP SHAPES; HIGH POINT	VIF	VERTICAL INSIDE FACE
HS	HIGH STRENGTH	VOF	VERTICAL OUTSIDE FACE
HSS	HOLLOW STRUCTURAL SECTION		
HT	HEIGHT	W	WIDE FLANGE; WIDE; WEST
		W/	WITH
ICC	INTERNATIONAL CODE COUNCIL	W/O	WITHOUT
ID	INSIDE DIAMETER	WD	WOOD
IN	INCH	WF	WIDE FLANGE
INCL	INCLUDE	WH	WEEP HOLE
INFO	INFORMATION	WL	WORK LINE
INSUL	INSULATION	WP	WORK POINT
INT	INTERIOR	WPJ	WEAKENED PLANE JOINT
		WT	WEIGHT; STRUCTURAL TEE CUT FROM W SHAPE
JST	JOIST	WWR	WELDED WIRE REINFORCING
JT	JOINT		
		YD	YARD
K	KIP (1,000 POUNDS)		
KO	KNOCK-OUT		
KSI	KIPS PER SQUARE INCH		

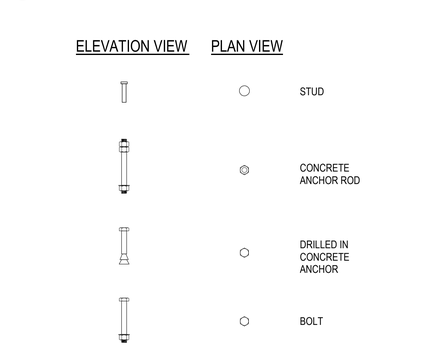
16 ABBREVIATIONS



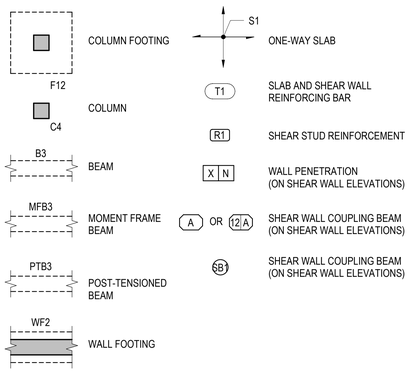
NOTES:

1. NO REACTION AT EITHER END INDICATES MINIMUM CONNECTION FOR BEAM DEPTH. SEE "GENERAL NOTES FOR STEEL CONNECTIONS".
2. SPACE STUDS PER "TYPICAL SHEAR STUD PLACEMENT" DETAIL.
3. "M" IN PLACE OF STEEL SHAPE INDICATES W10x12 WITH MINIMUM CONNECTION.
4. "HSS" IN PLACE OF STEEL SHAPE INDICATES HSS8x4x1/4.
5. "E" IN PLACE OF STEEL SHAPE INDICATES FRAMING PER "TYPICAL EXTENDED DECK EDGE" DETAILS.
6. WHERE NO BEAM SIZE IS CALLED OUT ADJACENT TO FLOOR OR ROOF OPENING, REFER TO TYPICAL STEEL DETAILS FOR SIZES AND CONNECTIONS.

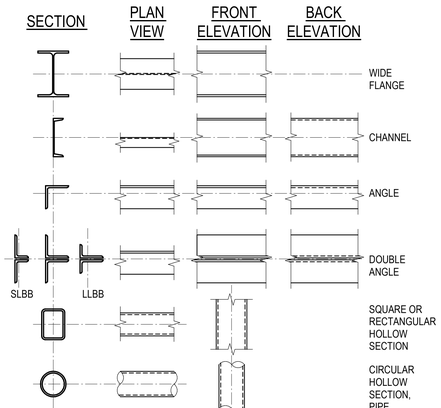
7 BEAM CALLOUT KEY



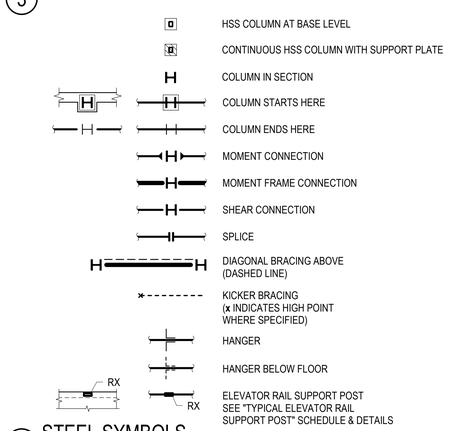
12 CONNECTORS



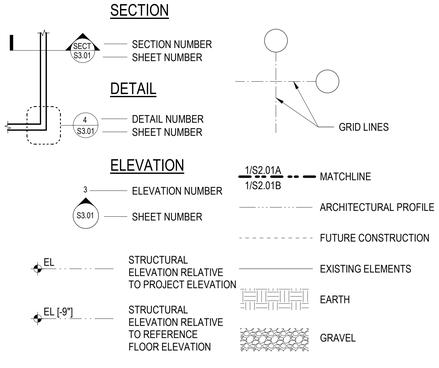
17 CONCRETE SCHEDULE MARKS



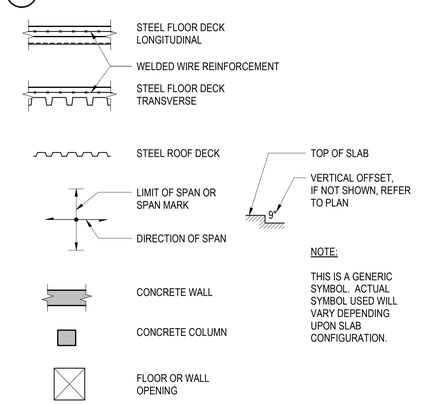
3 STEEL MEMBERS



8 STEEL SYMBOLS



13 MISCELLANEOUS SYMBOLS



18 CONCRETE SYMBOLS

19 DRAWING LIST

SHEET NUMBER	SHEET NAME	TOWER C FOUNDATION
S0.00	COVER	X
S0.01	ABBREVIATIONS, LEGENDS, AND DRAWING LIST	X
S0.02	REINFORCING DETAILS	X
S0.03	GENERAL NOTES	X
S0.04	GENERAL NOTES	X
S0.05	ISOMETRIC VIEWS	X
S0.06	ISOMETRIC VIEWS	X
S0.07	ISOMETRIC VIEWS	X
S0.08	ISOMETRIC VIEWS	X
S1.00	LOAD MAPS	X
S1.01	LOAD MAPS	
S1.02	LOAD MAPS	
S1.03	LOAD MAPS	
S1.04	LOAD MAPS	
S1.05	LOAD MAPS	
S1.06	LOAD MAPS	X
S1.07	LOAD MAPS	X
S1.08	LOAD MAPS	X
S1.09	LOAD MAPS	X
S2.01	TOWER A & B LEVEL B1 COMPOSITE FRAMING PLAN	
S2.02	TOWER A & B LEVEL P2 COMPOSITE FRAMING PLAN	
S2.11	TOWER A LEVEL 1 & TOWER B LEVEL P1 COMPOSITE FRAMING PLAN	
S2.12	TOWER A LEVEL 2 & TOWER B LEVEL 1 COMPOSITE FRAMING PLAN	
S2.A.01	TOWER A LEVEL B1 FRAMING PLAN	
S2.A.01.B	TOWER A LEVEL B1 LONGITUDINAL REINFORCING PLAN	
S2.A.01.V	TOWER A LEVEL B1 SHEAR REINFORCING PLAN	
S2.A.02	TOWER A LEVEL P2 FRAMING PLAN	
S2.A.02.B	TOWER A LEVEL P2 MAT LONGITUDINAL REINFORCING PLAN	
S2.A.02.R	TOWER A LEVEL P2 REINFORCING PLAN	
S2.A.02.V	TOWER A LEVEL P2 MAT SHEAR REINFORCING PLAN	
S2.A.11	TOWER A LEVEL 1 FRAMING PLAN	
S2.A.11.R	TOWER A LEVEL 1 REINFORCING PLAN	
S2.A.12	TOWER A LEVEL 2 FRAMING PLAN	
S2.A.12.R	TOWER A LEVEL 2 REINFORCING PLAN	
S2.A.13	TOWER A LEVEL 3 FRAMING PLAN	
S2.A.13.R	TOWER A LEVEL 3 REINFORCING PLAN	
S2.A.14	TOWER A LEVEL 4 FRAMING PLAN	
S2.A.14.R	TOWER A LEVEL 4 REINFORCING PLAN	
S2.A.15	TOWER A LEVEL 5 FRAMING PLAN	
S2.A.15.R	TOWER A LEVEL 5 REINFORCING PLAN	
S2.A.16	TOWER A LEVEL 6 FRAMING PLAN	
S2.A.16.R	TOWER A LEVEL 6 REINFORCING PLAN	
S2.A.17	TOWER A ROOF FRAMING PLAN	
S2.A.50	TOWER A PARTIAL PLANS	
S2.AB.01	TOWER A & B PARKING LEVEL 2 FRAMING PLAN	
S2.AB.01.B	TOWER A & B PARKING LEVEL 2 LONGITUDINAL REINFORCING PLAN	
S2.AB.11	AB CONNECTOR LEVEL 1 FRAMING PLAN	
S2.AB.11.R	AB CONNECTOR LEVEL 1 REINFORCING PLAN	
S2.AB.12	AB CONNECTOR ROOF LEVEL FRAMING PLAN	
S2.B.01	TOWER B LEVEL B1 FRAMING PLAN	
S2.B.01.B	TOWER B LEVEL B1 LONGITUDINAL REINFORCING PLAN	
S2.B.01.V	TOWER B LEVEL B1 SHEAR REINFORCING PLAN	
S2.B.02	TOWER B LEVEL P2 FRAMING PLAN	
S2.B.02.B	TOWER B LEVEL P2 MAT LONGITUDINAL REINFORCING PLAN	
S2.B.02.R	TOWER B LEVEL P2 REINFORCING PLAN	
S2.B.02.V	TOWER B LEVEL P2 MAT SHEAR REINFORCING PLAN	
S2.B.03	TOWER B LEVEL P1 FRAMING PLAN	
S2.B.03.R	TOWER B LEVEL P1 REINFORCING PLAN	
S2.B.11	TOWER B LEVEL 1 FRAMING PLAN	
S2.B.11.R	TOWER B LEVEL 1 REINFORCING PLAN	
S2.B.12	TOWER B LEVEL 2 FRAMING PLAN	
S2.B.12.R	TOWER B LEVEL 2 REINFORCING PLAN	
S2.B.13	TOWER B LEVEL 3 FRAMING PLAN	
S2.B.13.R	TOWER B LEVEL 3 REINFORCING PLAN	
S2.B.14	TOWER B LEVEL 4 FRAMING PLAN	
S2.B.14.R	TOWER B LEVEL 4 REINFORCING PLAN	
S2.B.15	TOWER B LEVEL 5 FRAMING PLAN	
S2.B.15.R	TOWER B LEVEL 5 REINFORCING PLAN	
S2.B.16	TOWER B LEVEL 6 FRAMING PLAN	
S2.B.16.R	TOWER B LEVEL 6 REINFORCING PLAN	
S2.B.17	TOWER B LEVEL 7 FRAMING PLAN	
S2.B.17.R	TOWER B LEVEL 7 REINFORCING PLAN	
S2.B.18	TOWER B ROOF FRAMING PLAN	
S2.B.50	TOWER B PARTIAL PLANS	
S2.C.01	TOWER C FOUNDATION LEVEL FRAMING PLAN	X
S2.C.01.B	TOWER C FOUNDATION LONGITUDINAL REINFORCING PLAN	X
S2.C.01.V	TOWER C FOUNDATION SHEAR REINFORCING PLAN	X
S2.C.11	TOWER C LEVEL 1 FRAMING PLAN	X
S2.C.11.R	TOWER C LEVEL 1 REINFORCING PLAN	X
S2.C.12	TOWER C LEVEL 2 FRAMING PLAN	X
S2.C.12.R	TOWER C LEVEL 2 REINFORCING PLAN	X
S2.C.13	TOWER C LEVEL 3 FRAMING PLAN	
S2.C.13.R	TOWER C LEVEL 3 REINFORCING PLAN	
S2.C.14	TOWER C LEVEL 4 FRAMING PLAN	
S2.C.14.R	TOWER C LEVEL 4 REINFORCING PLAN	
S2.C.15	TOWER C LEVEL 5 FRAMING PLAN	
S2.C.15.R	TOWER C LEVEL 5 REINFORCING PLAN	
S2.C.16	TOWER C LEVEL 6 FRAMING PLAN	
S2.C.16.R	TOWER C LEVEL 6 REINFORCING PLAN	
S2.C.17	TOWER C LEVEL 7 FRAMING PLAN	
S2.C.17.R	TOWER C LEVEL 7 REINFORCING PLAN	
S2.C.18	TOWER C LEVEL 8 FRAMING PLAN	
S2.C.18.R	TOWER C LEVEL 8 REINFORCING PLAN	
S2.C.19	TOWER C ROOF LEVEL FRAMING PLAN	

SHEET NUMBER	SHEET NAME	TOWER C FOUNDATION
S2.C.50	TOWER C PARTIAL PLANS	
S3.30	TOWER A & B BASEMENT WALL ELEVATIONS	
S3.31	TOWER A & B BASEMENT WALL ELEVATIONS	
S3.32	TOWER A & B BASEMENT WALL ELEVATIONS	
S3.33	TOWER A & B BASEMENT WALL ELEVATIONS	
S3.35	TOWER A & B BASEMENT WALL SECTIONS	
S3.40	TOWER C BASEMENT WALL ELEVATIONS	X
S3.45	TOWER C BASEMENT WALL SECTIONS	X
S3.A1	TOWER A WEST CORE WALL ELEVATIONS	
S3.A2	TOWER A EAST CORE WALL ELEVATIONS	
S3.A10	TOWER A WEST CORE WALL SECTIONS	
S3.A11	TOWER A WEST CORE WALL SECTIONS	
S3.A20	TOWER A EAST CORE WALL SECTIONS	
S3.A21	TOWER A EAST CORE WALL SECTIONS	
S3.B1	TOWER B NORTH CORE WALL ELEVATIONS	
S3.B2	TOWER B SOUTH CORE WALL ELEVATIONS	
S3.B10	TOWER B NORTH CORE WALL SECTIONS	
S3.B11	TOWER B NORTH CORE WALL SECTIONS	
S3.B20	TOWER B SOUTH CORE WALL SECTIONS	
S3.B21	TOWER B SOUTH CORE WALL SECTIONS	
S3.C1	TOWER C SHEAR WALL ELEVATIONS	X
S3.C2	TOWER C SHEAR WALL ELEVATIONS	X
S3.C10	TOWER C SHEAR WALL SECTIONS	X
S3.C20	TOWER C SHEAR WALL SECTIONS	X
S4.00	COLUMN SCHEDULES	X
S4.01	TYPICAL CONCRETE COLUMN DETAILS	X
S4.02	TYPICAL CONCRETE DETAILS	X
S4.03	TYPICAL CONCRETE BEAM DETAILS AND SCHEDULE	X
S4.04	TYPICAL MILD SLAB DETAILS	X
S4.05	TYPICAL POST-TENSIONED SLAB DETAILS	X
S4.06	TYPICAL STUD RAIL DETAILS AND SCHEDULE	X
S4.07	TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE	X
S4.08	TYPICAL SHEAR WALL DETAILS	X
S4.09	TYPICAL COUPLING BEAM DETAILS AND SCHEDULES	X
S4.11	TYPICAL STEEL DETAILS	X
S4.12	TYPICAL STEEL DETAILS	X
S4.13	TYPICAL STEEL BEAM CONNECTIONS TO CONCRETE	X
S4.14	TYPICAL STEEL DECK DETAILS	X
S4.15	TYPICAL STEEL DECK DETAILS	X
S4.16	TYPICAL STEEL DETAILS	X
S4.21	TYPICAL NON-LOAD BEARING CMU WALL DETAILS	X
S4.22	TYPICAL NON-LOAD BEARING CMU WALL DETAILS	X
S4.A.10	TYPICAL STEEL COLUMN SCHEDULE	
S4.B.10	TYPICAL STEEL COLUMN SCHEDULE	
S4.C.10	TOWER C STEEL COLUMN SCHEDULE	X
S5.00	TOWER A & B CONCRETE SECTIONS AND DETAILS	
S5.05	TOWER C CONCRETE SECTIONS AND DETAILS	X
S5.06	TOWER C CONCRETE SECTIONS AND DETAILS	
S6.00	TOWER A & B STEEL SECTIONS AND DETAILS	
S6.05	TOWER C STEEL SECTIONS AND DETAILS	

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project manager
drawn by
checked by
job no. 20052
date 7/15/2022

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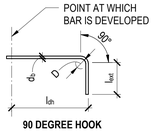
7/15/2022

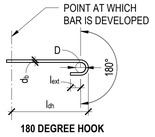
ABBREVIATIONS, LEGENDS, AND DRAWING LIST

S0.01

STANDARD HOOK			
ALL GRADES (D) FINISHED BEND DIAMETER			
BAR SIZE	D	90° HOOKS	180° HOOKS
		lext	lext
#3	2.25	4.5	2.5
#4	3	6	2.5
#5	3.75	7.5	2.5
#6	4.5	9	3
#7	5.25	10.5	3.5
#8	6	12	4
#9	9	13.5	4.5
#10	10.25	15.25	5
#11	11.25	17	5.75
#14	17	20.25	6.75
#18	22.5	27	9



90 DEGREE HOOK



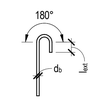
180 DEGREE HOOK

2 TYPICAL STANDARD HOOK GEOMETRY

STANDARD HOOK FOR STIRRUPS, TIES, AND HOOPS				
ALL GRADES (D) FINISHED BEND DIAMETER				
BAR SIZE	D	90° HOOKS	135° HOOKS	180° HOOKS
		lext	lext	lext
#3	1.5	3	3	2.5
#4	2	3	3	2.5
#5	2.5	3.75	3.75	2.5
#6	4.5	9	4.5	3
#7	5.25	10.5	5.25	3.5
#8	6	12	6	4

90 DEGREE HOOK

135 DEGREE HOOK



180 DEGREE HOOK

3 STANDARD HOOK GEOMETRY FOR STIRRUPS, TIES, AND HOOPS

NOTES:

1. NOTATIONS:
   - db: NOMINAL BAR DIAMETER (INCHES)
   - Ld: TENSION DEVELOPMENT LENGTH (INCHES) FOR REINFORCEMENT SATISFYING THE FOLLOWING REQUIREMENTS:
     - SLABS AND WALLS: CLEAR SPACING > 2db, AND CONCRETE CLEAR COVER > db
     - BEAMS AND COLUMNS: CLEAR SPACING > db, AND CONCRETE CLEAR COVER > db
   - Lt: DEVELOPMENT LENGTH OF BARS IN THICK CONCRETE = 1.3 X Ld (INCHES)
   - Lb: DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = 19 X db (INCHES)
   - Lc: TIED COLUMN LAP SPLICE IN COMPRESSION = 30 X db (INCHES)
   - Lcs: SPIRAL COLUMN LAP SPLICE IN COMPRESSION = 22.5 X db (INCHES)
   - Lsb: TYPICAL LAP SPLICE LENGTH = 1.3 X Ld (INCHES)
   - Lsbt: LAP SPLICE LENGTH OF HORIZONTAL BARS IN THICK CONCRETE = 1.69 X Ld (INCHES)
   - Ldh: DEVELOPMENT LENGTH IN TENSION OF STANDARD HOOK, WITH SIDE COVER ≥ 2 1/2" AND END COVER ≥ 2"
   - Ldt: DEVELOPMENT LENGTH IN TENSION OF HEADED BAR
2. MULTIPLY VALUES IN THE TABLE BY 1.5 IF CLEAR SPACING OR CONCRETE COVER DO NOT MEET THE REQUIREMENTS FOR Ld IN NOTE 1.
3. "HORIZONTAL BARS IN THICK CONCRETE" REFERS TO BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW. THIS INCLUDES BEAMS, SLABS, FOUNDATIONS, AND WALLS.
4. #14 AND #18 BARS SHALL NOT BE LAP SPLICED. SEE "GENERAL NOTES."
5. MULTIPLY VALUES IN THE TABLE BY 1.33 FOR USE WITH LIGHTWEIGHT AGGREGATE CONCRETE. FOR EPOXY COATED REINFORCEMENT, MULTIPLY VALUES IN THE TABLES BY 1.5 AND WITH EXCEPTION TO Ldh WHICH IS TO BE MULTIPLIED BY 1.2.
6. FOR EPOXY COATED REINFORCEMENT, MULTIPLY VALUES IN THE TABLES BY 1.5 AND WITH EXCEPTION TO Ldh WHICH IS TO BE MULTIPLIED BY 1.2.
7. WHEN BARS OF DIFFERENT SIZES ARE LAP SPLICED IN TENSION, SPLICE LENGTH SHALL BE THE LARGER OF Ld OF LARGER BAR AND Lsb OF SMALLER BAR, OR Lt AND Lsbt FOR BARS IN THICK CONCRETE.

4 REINFORCING BAR DEVELOPMENT AND SPLICE LENGTH NOTES

f'c = 4,000 PSI / GRADE 60						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	15	19	19	25	6	7
#4	19	25	25	33	8	11
#5	24	31	31	41	11	15
#6	29	37	37	49	15	20
#7	42	54	54	71	18	25
#8	48	62	62	81	22	30
#9	54	70	70	91	27	36
#10	61	79	79	102	32	43
#11	67	87	87	114	37	51
#14	81	105	-	-	-	66
#18	108	140	-	-	-	102

f'c = 5,000 PSI / GRADE 60						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	13	17	17	22	6	7
#4	17	23	23	29	8	11
#5	22	28	28	36	11	15
#6	26	34	34	44	14	19
#7	38	49	49	63	18	24
#8	43	56	56	72	22	29
#9	48	63	63	81	26	35
#10	54	71	71	92	31	42
#11	60	78	78	102	36	49
#14	72	94	-	-	-	64
#18	96	125	-	-	-	98

f'c = 6,000 PSI / GRADE 60						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	12	16	16	20	6	7
#4	16	21	21	27	8	10
#5	20	26	26	33	11	14
#6	24	31	31	40	14	19
#7	34	45	45	58	17	24
#8	39	51	51	66	21	29
#9	44	57	57	74	25	34
#10	50	64	64	84	30	41
#11	55	71	71	93	35	48
#14	66	86	-	-	-	63
#18	88	114	-	-	-	96

ALL CONCRETE STRENGTHS / GRADE 60			
BAR SIZE	Lb	Lc	Lcs
#3	8	12	9
#4	9	15	12
#5	12	19	15
#6	14	23	17
#7	16	27	20
#8	18	30	23
#9	21	34	26
#10	23	39	29
#11	26	43	32
#14	31	-	-
#18	41	-	-

f'c = 8,000 PSI / GRADE 60						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	12	14	14	18	6	6
#4	14	18	18	23	7	9
#5	17	22	22	29	9	13
#6	21	27	27	35	12	16
#7	30	39	39	50	15	20
#8	34	44	44	57	18	25
#9	38	50	50	64	22	30
#10	43	56	56	72	26	35
#11	48	62	62	80	30	41
#14	57	74	-	-	-	54
#18	76	99	-	-	-	83

f'c = 10,000 PSI / GRADE 60						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	12	12	12	16	6	6
#4	12	16	16	21	6	8
#5	15	20	20	26	8	11
#6	18	24	24	31	11	15
#7	27	35	35	45	14	18
#8	30	39	39	51	16	22
#9	34	44	44	58	20	27
#10	39	50	50	65	23	32
#11	43	55	55	72	27	37
#14	51	67	-	-	-	49
#18	68	89	-	-	-	74

8 REINFORCING BAR DEVELOPMENT LENGTHS AND LAP SPLICES - GRADE 60

f'c = 8,000 PSI / GRADE 80						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	16	21	21	27	6	8
#4	21	27	27	35	9	12
#5	26	34	34	44	12	17
#6	31	41	41	53	16	22
#7	46	59	59	77	20	27
#8	52	67	67	87	24	33
#9	59	76	76	99	29	39
#10	66	85	85	111	35	47
#11	73	95	95	123	40	55
#14	88	114	-	-	-	72
#18	117	151	-	-	-	111

f'c = 6,000 PSI / GRADE 80						
BAR SIZE	Ld	Lt	Lsb	Lsbt	Ldt	Ldh
#3	18	24	24	31	7	9
#4	24	31	31	41	10	14
#5	30	39	39	51	14	19
#6	36	47	47	61	18	25
#7	52	68	68	88	23	31
#8	60	78	78	101	28	38
#9	67	88	88	114	33	45
#10	76	99	99	128	40	54
#11	84	109	109	142	47	63
#14	101	131	-	-	-	83
#18	135	175	-	-	-	128

ALL CONCRETE STRENGTHS / GRADE 80			
BAR SIZE	Lb	Lc	Lcs
#3	9	18	14
#4	12	24	18
#5	15	30	23
#6	18	36	27
#7	21	42	32
#8	24	48	36
#9	28	55	41
#10	31	61	46
#11	34	68	51
#14	41	-	-
#18	55	-	-

13 REINFORCING BAR DEVELOPMENT LENGTHS AND LAP SPLICES - GRADE 80

Reserved for permit stamp

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REINFORCING DETAILS

S0.02

GENERAL

ALL TYPICAL DETAILS AND NOTES SHOWN ON DRAWINGS SHALL APPLY UNLESS NOTED OTHERWISE. TYPICAL DETAILS MAY NOT NECESSARILY BE INDICATED ON THE PLANS BUT SHALL STILL APPLY AS SHOWN OR DESCRIBED IN THE DETAILS. WHERE TYPICAL DETAILS ARE NOTED ON THE DRAWINGS, THE SPECIFIED TYPICAL DETAIL SHALL BE USED. WHERE NO DETAIL IS NOTED, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO CHOOSE THE APPROPRIATE TYPICAL DETAIL FROM THOSE PROVIDED. THE CONTRACTOR SHALL SUBMIT ALL PROPOSED ALTERNATE TYPICAL DETAILS TO THOSE PROVIDED WITH RELATED CALCULATIONS TO THE ENGINEER FOR APPROVAL PRIOR TO SHOP DRAWING PRODUCTION AND FIELD USE.

3D VIEWS (INCLUDING AXONOMETRICS, ISOMETRICS, PERSPECTIVES, ETC.) ARE PROVIDED FOR REFERENCE PURPOSES ONLY. IN THE EVENT OF ANY DISCREPANCIES BETWEEN INFORMATION REPRESENTED BY BOTH A 3D VIEW AND BY A NON-3D VIEW WITHIN THE CONSTRUCTION DOCUMENTS, THE NON-3D VIEW SHALL GOVERN IN ALL CASES. INFORMATION REPRESENTED BY 3D VIEWS, BUT NOT REPRESENTED ELSEWHERE IN THE CONSTRUCTION DOCUMENTS IS NOT INTENDED TO BE PART OF THE CONSTRUCTION DOCUMENTS.

BUILDING CODE

ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. THE PUBLICATIONS LISTED BELOW ARE THE GOVERNING CODES AND STANDARDS AND ARE REFERENCED BY THEIR BASIC DESIGNATION. IN THE CASE OF CONFLICTING REQUIREMENTS, THE BUILDING CODE SHALL GOVERN.

APPLICABLE CODES AND STANDARDS

BUILDING CODE	INTERNATIONAL BUILDING CODE (IBC), 2018 EDITION (INCLUDING THE STATE OF UTAH BUILDING CODE AMENDMENTS)
ACI 318	AMERICAN CONCRETE INSTITUTE, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," 2014 EDITION
TMS 402/602	THE MASONRY SOCIETY, "BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES," 2016 EDITION
RCSC	RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS, "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS," 2014 EDITION
AISC 341	AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS," 2016 EDITION
AISC 360	AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," 2016 EDITION
ASCE 7	AMERICAN SOCIETY OF CIVIL ENGINEERS, "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES," 2016 EDITION,
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM INTERNATIONAL)
AWS A2.4	AMERICAN WELDING SOCIETY, "STANDARD SYMBOLS FOR WELDING, BRAZING, AND NONDESTRUCTIVE EVALUATION," 2012 EDITION
AWS D1.1	AMERICAN WELDING SOCIETY, "STRUCTURAL WELDING CODE - STEEL," 2015 EDITION
AWS D1.4	AMERICAN WELDING SOCIETY, "STRUCTURAL WELDING CODE - REINFORCING STEEL INCLUDING METAL INSERTS AND CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION," 2018 EDITION
AWS D1.8	AMERICAN WELDING SOCIETY, "STRUCTURAL WELDING CODE - SEISMIC SUPPLEMENT," 2016 EDITION
ICC	INTERNATIONAL CODE COUNCIL, INTERNATIONAL CODE COUNCIL - EVALUATION SERVICES (ICC-ES)
IAPMO	INTERNATIONAL ASSOCIATION OF PLUMBING AND MECHANICAL OFFICIALS – UNIFORM EVALUATION SERVICE (IAPMO-UES)

STRUCTURAL DESIGN DATA

LOAD COMBINATIONS: LOAD COMBINATIONS ARE IN ACCORDANCE WITH SECTION 1605 OF THE BUILDING CODE.

LIVE LOADS: LIVE LOADS SHALL BE IN ACCORDANCE WITH THE LOAD DIAGRAMS.

SNOW LOADS: SNOW LOADING AND SNOW DRIFT LOADING SHALL BE IN ACCORDANCE WITH THE BUILDING CODE (SECTION 1608), , INCLUDING SITE-SPECIFIC DETERMINATION OF GROUND SNOW LOAD PER UTAH STATE AMENDMENT TO SECTION 1608.2.1.

GROUND SNOW LOAD: Pg = 194 PSF

IMPORTANCE FACTOR: Is = 1.0

SNOW EXPOSURE FACTOR: Ce = 1.0

THERMAL FACTOR: Ct = 1.0

FLAT-ROOF SNOW LOAD: Pf = 136 PSF

WIND LOADS: WIND PRESSURE SHALL BE IN ACCORDANCE WITH THE BUILDING CODE (SECTION 1609).

BASIC WIND SPEED (3-SECOND GUST): Vult = 115 MPH
BASIC WIND SPEED (3-SECOND GUST): Vasd = 85 MPH

RISK CATEGORY: II

EXPOSURE CATEGORY: B

INTERNAL PRESSURE COEFFICIENT: GCpi = 0.18

SEISMIC LOADS: SEISMIC LOADING SHALL BE IN ACCORDANCE WITH THE BUILDING CODE.

BUILDING LOCATION: LATITUDE: 40.615° N
LONGITUDE: 111.508° W

RISK CATEGORY: II

IMPORTANCE FACTOR: Ie = 1.0

MAPPED SPECTRAL ACCELERATION PARAMETERS: Ss = 0.60, S1 = 0.21

SITE CLASS: C

SITE COEFFICIENTS: Fa = 1.263, Fv = 1.5

SPECTRAL RESPONSE COEFFICIENTS: Sds = 0.50, Sd1 = 0.21

SEISMIC DESIGN CATEGORY: D

LATERAL SYSTEM:
  TOWERS A, B & C: SPECIAL REINFORCED CONCRETE SHEAR WALLS
  TOWER AB CONNECTOR: STEEL SPECIAL CANTILEVER COLUMNS

RESPONSE MODIFICATION COEFFICIENT:
  TOWERS A, B & C: R=6
  TOWER AB CONNECTOR: R= 2 1/2"

SEISMIC RESPONSE COEFFICIENT:
  TOWER A:
    NORTH-SOUTH: Cs = 0.078
    EAST-WEST: Cs = 0.046
  TOWER B:
    NORTH-SOUTH: Cs = 0.034
    EAST-WEST: Cs = 0.049
  AB CONNECTOR:
    NORTH-SOUTH: Cs = 0.202
    EAST-WEST: Cs = 0.202
  TOWER C:
    NORTH-SOUTH: Cs = 0.061
    EAST-WEST: Cs = 0.033

DESIGN BASE SHEAR:
  TOWER A:
    NORTH-SOUTH: V = 853 KIPS
    EAST-WEST: V = 1440 KIPS
  TOWER B:
    NORTH-SOUTH: V = 1071 KIPS
    EAST-WEST: V = 1537 KIPS
  AB CONNECTOR:
    NORTH-SOUTH: V = 115 KIPS
    EAST-WEST: V = 168 KIPS
  TOWER C:
    NORTH-SOUTH: V = 1477 KIPS
    EAST-WEST: V = 796 KIPS

ANALYSIS PROCEDURE USED: MODAL RESPONSE SPECTRUM ANALYSIS

LOAD PATH FOR LATERAL FORCES: LATERAL FORCES ARE CARRIED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR WALLS. MOMENTS, SHEARS, AND ROTATIONAL FORCES ARE DELIVERED TO THE FOUNDATION BY THE SHEAR WALLS IN PROPORTION TO THEIR ABILITY TO RESIST LATERAL DEFORMATION.

STORY DRIFT

THE PRIMARY STRUCTURE WILL EXPERIENCE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS. THE STORY DRIFTS PERPENDICULAR AND/OR PARALLEL TO THE PRIMARY STRUCTURE ARE AS FOLLOWS:

SEISMIC DRIFT

LEVELS	SERVICE LEVEL STORY DRIFT	DESIGN STORY DRIFT, IeDp
TOWERS A, B, & C:		
ALL	1/2"	1 1/2"
AB CONNECTOR:	1/2"	3"

WIND DRIFT

LEVELS	SERVICE LEVEL STORY DRIFT	DESIGN STORY DRIFT
TOWERS A, B, & C:		
ALL	1/2"	1"
AB CONNECTOR:	1/2"	1 1/2"

WHERE REQUIRED BY THE BUILDING CODE, NON-STRUCTURAL COMPONENTS INCLUDING EXTERIOR CLADDING; STAIRS, ELEVATORS, AND MISCELLANEOUS METALS; MECHANICAL/ELECTRICAL/PLUMBING SYSTEM SUPPORTS; INTERIOR METAL STUD FRAMING; AND ANY OTHER ELEMENTS REQUIRED BY THE BUILDING CODE SHALL BE DESIGNED TO ACCOMMODATE THE PRIMARY STRUCTURE STORY DRIFTS WITH ANY APPLICABLE ELEMENT-SPECIFIC MODIFICATIONS PER CHAPTER 13 OF ASCE 7.

FOUNDATIONS

THE FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL ENGINEERING DESIGN REPORT ENTITLED "GEOTECHNICAL INVESTIGAION - CONDOMINIUM DEVELOPMENT LOTS 1 AND 2, B2 EAST PROPERTY - 9300 MARSAC AVENUE - PARK CITY, UTAH" DATED SEPTEMBER 11, 2019, PREPARED BY APPLIED GEOTECHNICAL ENGINEERING CONSULTANTS, INC. REFER TO THIS REPORT FOR ALL GEOTECHNICAL REQUIREMENTS AND ANTICIPATED CONDITIONS BELOW GRADE.

COLUMN DOWELS SHALL BE INSTALLED WITH A TEMPLATE TO HOLD BARS IN THE PROPER POSITION AND SHALL BE PLACED WITH A TOLERANCE OF +/—1/4 INCH.

MAT FOUNDATION AND SPREAD FOOTINGS: DESIGN SOIL BEARING PRESSURE = 8,500 PSF. ALL FOOTINGS SHALL BEAR ON SUITABLE UNDISTURBED SOIL AND AND/OR PREPARED BASE MATERIALS APPROVED BY THE GEOTECHNICAL ENGINEER. WHERE SUITABLE UNDISTURBED SOILS ARE NOT FOUND AT THE SPECIFIED FOOTING ELEVATION, OVER-EXCAVATE TO THE DEPTHS REQUIRED BY THE GEOTECHNICAL ENGINEER AND REPLACE MATERIALS WITH STRUCTURAL FILL, LEAN CONCRETE, OR PROVIDE OTHER PREPARATION AS DIRECTED BY THE GEOTECHNICAL ENGINEER TO ACHIEVE THE REQUIRED BEARING CAPACITY.

STRUCTURAL FILL

ALL FILL PLACED TO SUPPORT SLABS ON GRADE, BEHIND PERMANENT WALLS, AND AROUND ALL DRAINS SHALL CONSIST OF WELL GRADED, GRANULAR MATERIAL PER THE SPECIFICATIONS. SOILS FOR STRUCTURAL FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. STRUCTURAL FILL SHALL BE PLACED ON SOUND NATIVE MATERIAL. PROOF-ROLL CUT AREAS WHICH PROVIDE SUPPORT FOR PERMANENT STRUCTURES. AREAS WHICH ARE EXCESSIVELY YIELDING, AS DETERMINED BY THE CONTINUOUS OBSERVATION OF THE GEOTECHNICAL ENGINEER, SHALL BE OVEREXCAVATED AND REPLACED WITH STRUCTURAL FILL. STRUCTURAL FILL SHALL BE PLACED PER THE SPECIFICATION.

LATERAL PRESSURE ON SUBGRADE WALLS

THE DESIGN PRESSURES FOR SUBGRADE WALLS ARE BASED ON A "DRAINED" CONDITION. SEE CIVIL AND MECHANICAL DRAWINGS FOR SUBGRADE DRAINAGE SYSTEM. SEE GEOTECHNICAL REPORT FOR COMPACTION REQUIREMENTS AT SUBGRADE WALLS. SUBGRADE WALLS AND SUPPORTING SLABS SHALL HAVE ATTAINED THEIR FULL CONCRETE STRENGTH BEFORE PLACING ANY BACKFILL. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACES FOR WALLS IF BACKFILL IS PLACED BEFORE WALLS AND SLABS ACHIEVE FULL CONCRETE STRENGTH.

BACKFILLED, RESTRAINED BASEMENT WALLS ARE DESIGNED USING AN "APPARENT" EARTH PRESSURE AS SHOWN IN THE LOAD MAPS.

CONCRETE

MIXING, BATCHING, TRANSPORTING, PLACING, AND CURING OF ALL CONCRETE, AND SELECTION OF CONCRETE MATERIALS, SHALL CONFORM TO ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE," EXCEPT AS NOTED BELOW. PROPORTIONS OF AGGREGATE TO CEMENTITIOUS PASTE SHALL BE SUCH AS TO PRODUCE A DENSE, WORKABLE MIX THAT CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER.

MIX DESIGNS LISTED BELOW SHALL BE SUBMITTED TO THE ARCHITECT AND APPROVED PRIOR TO USE. SELECTION OF CONCRETE MIX PROPORTIONS SHALL BE IN ACCORDANCE WITH ACI 301. MIX PROPORTIONS SHALL MEET OR EXCEED THE REQUIREMENTS LISTED BELOW FOR THE LOCATIONS NOTED. THE MORE STRINGENT OF THE REQUIREMENTS LISTED SHALL GOVERN.

PROVIDE ASTM C150 (TYPE I OR TYPE II) OR ASTM C595 (TYPE IL, IS, IP, OR IT) CEMENT UNLESS NOTED OTHERWISE. THE CEMENTITIOUS MATERIAL CONTENT SHALL BE ADEQUATE FOR THE SPECIFIED REQUIREMENTS FOR STRENGTH, WATER-CEMENT RATIO, DURABILITY, AND FINISH ABILITY.

MAXIMUM COMPLEMENTARY CEMENTING MATERIALS (EX. FLY ASH, SLAG, SILICA FUME) AS A PERCENTAGE OF TOTAL WEIGHT OF CEMENTITIOUS MATERIAL SHALL BE 50 PERCENT. FLY ASH SHALL MEET ASTM C618 REQUIREMENTS, AND SHALL NOT EXCEED 30 PERCENT OF TOTAL CEMENTITIOUS MATERIAL. WATER/CEMENT RATIO SHALL BE BASED ON TOTAL CEMENTITIOUS MATERIAL, INCLUDING COMPLEMENTARY CEMENTING MATERIALS. MAXIMUM SIZE OF AGGREGATE SHALL BE AS LISTED BELOW.

ALL CONCRETE USED IN HORIZONTAL SURFACES EXPOSED TO THE WEATHER SHALL CONTAIN AN ACCEPTABLE ADMIXTURE TO PRODUCE AIR-ENTRAINED CONCRETE WITH TOTAL AIR CONTENT AS NOTED IN THE CONCRETE MIX SPECIFICATION TABLE. TOLERANCE FOR AIR CONTENT SHALL BE +/—1.5 PERCENT. AIR CONTENT SHALL BE MEASURED AT THE DISCHARGE OF THE TRUCK. IF CONCRETE IS PUMPED, AIR CONTENT SHALL BE MEASURED AT THE DISCHARGE END OF THE PUMP LINE. TESTS FOR AIR CONTENT SHALL MEET ASTM C172 REQUIREMENTS.

THE CONTRACTOR SHALL DETERMINE SLUMP. EACH CONCRETE MIX SUBMITTED SHALL HAVE THE SLUMP SPECIFIED. SLUMP SHALL BE MEASURED AT THE DISCHARGE OF THE TRUCK. IF CONCRETE IS PUMPED, SLUMP SHALL BE MEASURED AT THE DISCHARGE END OF THE PUMP LINE. SLUMPS SHALL BE WITHIN +1 INCH AND —2 INCHES OF THE SPECIFIED SLUMP.

THE USE OF SUPER PLASTICIZERS AND WATER REDUCERS IS ALLOWED, BUT NOT REQUIRED. ALL ADMIXTURES SHALL BE CHLORIDE-FREE UNLESS OTHERWISE APPROVED BY THE ENGINEER.

CONCRETE MIX SPECIFICATION TABLE

LOCATION	f'c MIN (PSI)	TEST AGE (DAYS)	MAX W/C RATIO	MAX AGGREGATE SIZE	AIR CONTENT PERCENT
MISCELLANEOUS CONCRETE, CURBS, SIDEWALKS	4,000	28	0.50	1"	4.5
EXTERIOR EXPOSED SLABS ON GRADE	4,000	28	0.45	1"	4.5
INTERIOR SLABS ON GRADE	4,000	28	0.50	1"	-
MAT FOUNDATION, SPREAD FOOTINGS	6,000	56	0.44	1"	-
BASEMENT WALLS	6,000	56	0.44	3/4"	-
CONCRETE ON STEEL DECK	4,000	28	0.44	3/4"	-
MILD REINFORCED BEAMS AND SLABS	6,000	28	0.44	3/4"	-
EXTERIOR EXPOSED POST-TENSIONED BEAMS AND SLABS	6,000	28	0.40	3/4"	5.0
INTERIOR POST-TENSIONED BEAMS AND SLABS	6,000	28	0.40	3/4"	-
COLUMNS	PER COL SCHED	56	0.44	3/4"	-
SHEAR WALLS	PER WALL ELEV	56	0.44	3/4"	-

SHOTCRETE

STRUCTURAL SHOTCRETE MAY BE USED FOR WALLS IN LIEU OF CAST-IN-PLACE CONCRETE. SHOTCRETE SHALL CONFORM STRICTLY TO THE REQUIREMENTS OF THE BUILDING CODE, SECTION 1908. NON-CONTACT LAP SPLICES SHALL BE DETAILED IN ACCORDANCE WITH THE CODE AND SUBMITTED ON SHOP DRAWINGS IN ACCORDANCE WITH THE SPECIFICATIONS. TEST PANELS ARE REQUIRED FOR HEAVILY REINFORCED AREAS SUCH AS PILASTERS, WALL COLUMNS, ETC. THE CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS FOR SHOTCRETING.

MASSIVE CONCRETE

CONCRETE PLACED IN MONOLITHIC PLACEMENTS WHERE THE MINIMUM OF ALL THREE DIMENSIONS EXCEEDS 4'-0" SHALL BE CONSIDERED "MASSIVE CONCRETE" AND SHALL BE SUBJECT TO THE APPLICABLE REQUIREMENTS OF ACI 301, CHAPTER 8.

ASTM C150 TYPE III CEMENT IS PROHIBITED. UNLESS OTHERWISE SPECIFIED, USE MODERATE OR LOW HEAT OF HYDRATION CEMENT, BLENDED HYDRAULIC CEMENT WITH MODERATE OR LOW HEAT OF HYDRATION PROPERTIES, OR PORTLAND CEMENT WITH FLY ASH, POZZOLAN, OR GROUND-GRANULATED BLAST-FURNACE SLAG. ADDITIVES CONTAINING CALCIUM CHLORIDE ARE PROHIBITED. APPROVED RETARDING, RETARDING HIGH-RANGE WATER REDUCING, OR RETARDING PLASTICIZING ADMIXTURE SHALL BE USED.

THE TEMPERATURE OF CONCRETE AT TIME OF PLACEMENT SHALL NOT EXCEED 95 DEGREES FAHRENHEIT. THE MAXIMUM INTERNAL TEMPERATURE DURING CURING SHALL NOT EXCEED 160 DEGREES FAHRENHEIT. THE MAXIMUM TEMPERATURE DIFFERENCE BETWEEN CENTER AND SURFACE OF PLACEMENT SHALL NOT EXCEED 50 DEGREES FAHRENHEIT. CONFORM TO THE REQUIREMENTS OF ACI 305.1 AND ACI 306.1 FOR HOT-WEATHER AND COLD-WEATHER CONCRETING, RESPECTIVELY. IF COOLING METHODS ARE EMPLOYED, THEY SHALL NOT INCREASE THE WATER-CEMENT RATIO OR SLUMP BEYOND ALLOWABLE LIMITS. THE CONCRETE SHALL BE COOLED GRADUALLY SO THAT THE SURFACE TEMPERATURE DROP DOES NOT EXCEED 20 DEGREES FAHRENHEIT IN ANY 24-HOUR PERIOD AFTER PLACEMENT.

SUBMIT DETAILED PROCEDURES, MATERIALS, MIX DESIGNS, AND TEST RESULTS INCLUDING HEAT OF HYDRATION TEST DATA PER ASTM C1702 TO THE ENGINEER BEFORE CONSTRUCTION OF MASSIVE CONCRETE.

REINFORCING STEEL

ALL REINFORCING SHALL BE NEW BILLET STOCK ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. REINFORCING REQUIRED TO BE EPOXY COATED SHALL BE ASTM A775, GRADE 60, UNLESS NOTED OTHERWISE. BARS SHALL BE SECURELY TIED IN PLACE WITH #16 GAGE MINIMUM ANNEALED BLACK WIRE. EPOXY-COATED REINFORCING BARS SHALL BE FASTENED WITH NYLON-, EPOXY-, OR PLASTIC-COATED TIE WIRE OR OTHER ACCEPTABLE MATERIALS. BARS SHALL BE SUPPORTED ON CHAIRS IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE. THE CONTRACTOR SHALL COORDINATE REINFORCING STEEL PLACEMENT DETAILS AND PROVIDE TEMPLATES FOR PLACING STEEL IN CONGESTED AREAS AS NECESSARY. SHOP DRAWINGS (INCLUDING PLACING PLANS AND ELEVATIONS) SHALL BE SUBMITTED TO, AND REVIEWED BY, THE ARCHITECT/ENGINEER BEFORE STARTING FABRICATION.

REINFORCING BARS SHALL BE LAP SPLICED FOR TENSION (LSB) UNLESS NOTED OTHERWISE ON THE DRAWINGS. #14 AND #18 BARS SHALL BE SPLICED USING MECHANICAL COUPLINGS INCLUDING SPLICES WITH SMALLER BARS. #14 AND #18 BARS SHALL NOT BE LAP SPLICED. AT THE CONTRACTOR'S OPTION, MECHANICAL COUPLINGS MAY BE USED FOR ANY BAR SIZE, PROVIDED A CURRENT ICC—ES (OR IAPMO-UES EQUIVALENT) REPORT DEMONSTRATES THAT THE PRODUCT CAN ACHIEVE A MINIMUM TENSILE STRENGTH OF 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE BAR. NO REINFORCING BARS SHALL BE SPLICED BY WELDING. FOR REINFORCING WITHIN SHEAR WALLS, AND REINFORCING THAT CONNECTS THE SLABS TO THE SHEAR WALLS, MECHANICAL SPLICES MAY BE USED IF THE MECHANICAL SPLICE STRENGTH IS INCREASED TO DEVELOP 100 PERCENT OF THE SPECIFIED TENSILE STRENGTH OF THE SPLICED BAR. SPLICE DEVICES SHALL HAVE A CURRENT ICC-ES (OR IAPMO-UES EQUIVALENT) REPORT THAT SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. HEADED BARS OR TERMINATORS SHALL BE PROVIDED WHERE INDICATED ON THE DRAWINGS OR AT THE CONTRACTOR'S OPTION FOR CONGESTED AREAS OF REINFORCEMENT ANCHORAGE SUBJECT TO THE ENGINEER'S APPROVAL. HEADED BARS OR TERMINATORS SHALL MEET THE REQUIREMENTS OF ACI 318 AND ASTM A970, AND HAVE A CURRENT ICC-ES (OR IAPMO-UES EQUIVALENT) REPORT.

WELDING OR TACK WELDING OF REINFORCING BARS TO OTHER BARS OR TO PLATES, ANGLES, ETC, IS PROHIBITED, EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. WHERE WELDING IS APPROVED, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E9018 OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.4.

MINIMUM CAST-IN-PLACE CONCRETE COVER OVER REINFORCING STEEL, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:

1. CONCRETE CAST AGAINST EARTH:

   ALL BAR SIZES: 3 INCHES

2. CONCRETE EXPOSED TO EARTH OR WEATHER:

   #6 BAR OR LARGER: 2 INCHES
   #5 BAR OR SMALLER: 1 1/2 INCHES

3. OTHER CONCRETE:

   SLABS:
     #14 AND #18 BARS: 1-1/2 INCHES
     #11 BARS AND SMALLER:
       TOP BARS: 3/4 INCH
       BOTTOM BARS: 1 INCH

   WALLS:
     #14 AND #18 BARS: 1-1/2 INCHES
     #11 BARS AND SMALLER: 1 INCH

   BEAMS AND COLUMNS - TIES, STIRRUPS, SPIRALS:
     ALL BAR SIZES: 1-1/2 INCHES

SPECIFIED CONCRETE COVER SHALL BE MAINTAINED TO ALL REINFORCEMENT AT CONCRETE REVEALS AND INSETS. SHOP DRAWINGS SHOWING CONCRETE REVEALS AND OTHER INSETS SHALL BE SUBMITTED FOR REVIEW.

SPECIAL DUCTILE QUALITY REINFORCING STEEL

VERTICAL REINFORCING IN COLUMNS AND SHEAR WALLS, LONGITUDINAL AND DIAGONAL REINFORCING IN COUPLING BEAMS, AND ALL OTHER REINFORCING MARKED "SDQ" SHALL BE LOW-ALLOY STEEL DEFORMED ASTM A706. BILLET STEEL ASTM A615, GRADE 60 REINFORCEMENT MAY BE USED IN THESE MEMBERS IF (1) THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED THE SPECIFIED YIELD STRENGTH BY MORE THAN 18,000 PSI AND (2) THE RATIO OF THE ACTUAL ULTIMATE TENSILE STRENGTH TO THE ACTUAL TENSILE YIELD STRENGTH IS NOT LESS THAN 1.25. IF MILL REPORTS ARE NOT AVAILABLE, THE REINFORCING SHALL BE TESTED PER THE SPECIFICATIONS AT THE CONTRACTOR'S EXPENSE. (3) MINIMUM ELONGATION IN 8-INCH SHALL BE AT LEAST 14 PERCENT FOR BAR SIZES NO. 3 THROUGH 6, AT LEAST 12 PERCENT FOR BAR SIZES NO. 7 THROUGH 11, AND AT LEAST 10 PERCENT FOR BAR SIZES NO. 14 AND 18.

WELDED WIRE REINFORCEMENT

WELDED WIRE REINFORCEMENT (WWR) SHALL BE ELECTRICALLY WELDED AND CONFORM TO ASTM A1064. LAP EDGES AND ENDS OF REINFORCEMENT A MINIMUM OF ONE MESH SPACING PLUS 2 INCHES, BUT NOT LESS THAN 6 INCHES. WELDED WIRE REINFORCEMENT SHALL BE SUPPORTED ON CHAIRS IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.

POST-TENSIONED PRESTRESSED CONCRETE

POST-TENSIONING SHOP DRAWINGS, INCLUDING PLANS AND DETAILS, SHALL BE SUBMITTED TO AND REVIEWED BY THE ARCHITECT BEFORE STARTING FABRICATION.

IN ADDITION TO THE REQUIREMENTS IN THE CONCRETE MIX SPECIFICATION TABLE NOTED ABOVE, ALL POST-TENSIONED CONCRETE SHALL HAVE A MINIMUM STRENGTH OF 3,000 PSI AT TIME OF INITIAL STRESS. THE CONTRACTOR SHALL SUBSTANTIATE CONCRETE STRENGTH BEFORE POST-TENSIONING.

POST-TENSIONING REINFORCEMENT SHALL BE 1/2—INCH-DIAMETER, UNBONDED, LOW RELAXATION, 270—KSI HIGH-TENSILE WIRE STRAND CONFORMING TO ASTM A416.

POST-TENSIONING TENDONS SHALL BE ENCASED IN WATERPROOF POLYETHYLENE PLASTIC SHEATHING OF 50 MILS MINIMUM THICKNESS. SHEATHING SHALL BE OF SUFFICIENT STRENGTH TO PREVENT UNREPAIRABLE DAMAGE DURING FABRICATION, TRANSPORTATION, INSTALLATION, STORAGE, CONCRETE PLACEMENT, AND TENSIONING. AN ENCAPSULATED TENDON SYSTEM SHALL BE USED AT ALL LOCATIONS.

THE MINIMUM NUMBER OF TENDONS REQUIRED FOR INSTALLATION IS SHOWN ON THE DRAWINGS AND IS BASED ON A FINAL EFFECTIVE PRESTRESS FORCE OF 26.8 KIPS PER TENDON. IF THE MINIMUM FINAL EFFECTIVE PRESTRESS FORCE, CONSIDERING ALL LOSSES NOTED BELOW, IS LESS THAN 26.8 KIPS AT ANY LOCATION ALONG THE TENDON LENGTH, THE CONTRACTOR SHALL PROVIDE ADDITIONAL TENDONS. TENDONS SHALL BE INSTALLED WITH A PARABOLIC DRAPE UNLESS NOTED OTHERWISE AND HELD IN THEIR DESIGNED POSITIONS AS SHOWN ON THE DRAWINGS. A MINIMUM OF TWO TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION AT SLABS WITHOUT BEAMS. DISTRIBUTED TENDONS MAY BE GROUPED WHILE PROVIDING THE SAME TOTAL COUNT, ALTHOUGH SPACING BETWEEN SUCH GROUPS SHALL NOT EXCEED 5 FEET NOR EIGHT TIMES THE SLAB THICKNESS. TENDON DEAD END AND LIVE END SYMBOLS ARE SHOWN ONLY TO REPRESENT POTENTIAL POST-TENSIONING DIRECTION. AT THE CONTRACTOR'S OPTION BUT SUBJECT TO THE ENGINEER'S APPROVAL, LIVE ENDS MAY BE SWITCHED OR ALTERNATED, DOUBLE-ENDED TENSIONING MAY BE USED, AND INTERMEDIATE TENSIONING MAY BE USED. THE FINAL POST-TENSIONING DIRECTION AND SEQUENCE SHALL BE SELECTED BY THE CONTRACTOR TO ACHIEVE THE REQUIRED MINIMUM EFFECTIVE PRESTRESS FORCE, BUT IS SUBJECT TO THE ENGINEER'S FINAL APPROVAL.

THE POST-TENSIONED SLABS ARE DESIGNED BASED ON TENDONS BEING CONTINUOUS BETWEEN EDGES OF SLABS AS SHOWN ON THE DRAWINGS. ANY ADDITIONAL INTERMEDIATE STRESSING JOINTS OR CLOSURE STRIPS REQUIRED BY THE CONTRACTOR MAY REQUIRE ADDITIONAL REINFORCEMENT AND SHALL BE SUBMITTED AND REVIEWED BY THE STRUCTURAL ENGINEER BEFORE STARTING CONSTRUCTION.

THE CONTRACTOR SHALL PROVIDE THE FOLLOWING INFORMATION STAMPED BY AN ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED: (1) THE DETAILED DESIGN OF TENDON END ANCHORAGES; (2) THE CALCULATION OF STRESS LOSSES DUE TO CREEP, SHRINKAGE, TENDON RELAXATION, ANCHORAGE SLIP, WOBBLE FRICTION, AND FRICTION DUE TO VERTICAL AND HORIZONTAL TENDON CURVATURE; (3) POST-TENSIONING SHOP DRAWINGS. THIS INFORMATION SHALL CONFORM TO ACI 318.

NO CONCRETE SHALL BE PLACED UNTIL THE POST-TENSIONING TENDONS AND REINFORCEMENT LOCATION HAVE BEEN INSPECTED AND APPROVED BY THE TESTING AGENCY. CONTINUOUS INSPECTION AND RECORDING OF ELONGATION IS REQUIRED DURING ALL STRESSING OPERATIONS. DO NOT CUT TENDON ENDS UNTIL THE ENTIRE SLAB HAS BEEN SATISFACTORILY STRESSED AND THE ENGINEER HAS REVIEWED ELONGATIONS.

WHERE REQUIRED BY OWNER, THE POSITION OF TENDONS SHALL BE MARKED ON THE FORMS IMMEDIATELY BEFORE NEW CONCRETE IS PLACED WITH A MATERIAL WHICH LEAVES A PHYSICAL IMPRESSION ON THE UNDERSIDE OF THE SLAB.

DRILLED-IN CONCRETE ANCHORS AND POWER-DRIVEN FASTENERS SHALL BE PLACED A MINIMUM DISTANCE EQUAL TO THE SLAB THICKNESS AWAY FROM TENDON LOCATIONS AND FOUR TIMES THE SLAB THICKNESS FROM THE FACE OF ANY COLUMN. WHERE TENDON MARKING IS NOT SUCCESSFUL, TENDONS SHALL BE LOCATED BY SLAB SCANNING PRIOR TO ANCHOR INSTALLATION. EXCEPTION: POWDER ACTUATED FASTENERS WITH EMBEDMENT OF 5/8 INCH OR LESS MAY BE INSTALLED AT ANY LOCATION ON THE SLAB. NO REBAR OR POST-TENSIONING TENDONS SHALL BE DAMAGED BY ANCHORS AND FASTENERS.

FORM CAMBER

IN ADDITION TO ANY CAMBER NOTED IN THE STRUCTURAL DRAWINGS, CONCRETE FORMWORK SHALL BE CAMBERED TO COMPENSATE FOR FORM SAG UNDER WET CONCRETE LOAD. CAMBERS OF LESS THAN 1/8 INCH MAY BE NEGLECTED.

PRECAST CONCRETE

REFER TO ARCHITECTURAL DRAWINGS FOR FINISH REQUIREMENTS, REVEALS, OPENINGS, DETAILS, AND DIMENSIONS NOT SHOWN. HORIZONTAL PRECAST MEMBERS SHALL BE CAST, STRESSED, TRANSPORTED, AND ERECTED IN A HORIZONTAL, UPRIGHT POSITION. SUPPORTS DURING TRANSPORTATION AND ERECTION SHALL APPROXIMATE THOSE IN THE FINAL STRUCTURE. WHERE NECESSARY, THE CONTRACTOR SHALL PROVIDE AND INSTALL ADDITIONAL REINFORCING (STEEL STIFFENERS, BRACING, LIFTING INSERTS, ETC) TO RESIST ERECTION AND TRANSPORTATION STRESSES. LIFTING INSERTS SHALL BE INSTALLED AT LOCATIONS WHERE THEY WILL BE HIDDEN BY CONSTRUCTION OR OTHERWISE COVERED. FINISH ALL PANELS WHICH WILL BE COVERED WITH TOPPING SLABS WITH ROUGH SCREED FINISH TO ENSURE BONDING OF THE APPLIED CONCRETE. SUBMIT DETAILED SHOP DRAWINGS OF ALL PRECAST CONCRETE WORK TO THE ARCHITECT FOR REVIEW BEFORE STARTING FABRICATION.

ALL PRECAST CONCRETE CONNECTIONS (PLATES, ANGLES, ETC) SHALL BE GALVANIZED STEEL. REMOVE GALVANIZING WHERE FIELD WELDING IS REQUIRED, AND PAINT FIELD WELDS WITH ONE COAT OF "GALVACON" OR APPROVED EQUAL. SEE SHEAR STUD CONNECTORS NOTE FOR STUDS WELDED TO CONNECTIONS. FIELD WELDING SHALL BE DONE BY AWS-CERTIFIED WELDERS IN ACCORDANCE WITH AWS D1.1.

CONSTRUCTION JOINTS

ALL CONSTRUCTION JOINTS IN SLABS, BEAMS, AND WALLS SHALL BE KEYED IN ACCORDANCE WITH THE TYPICAL DETAILS OR, AT THE CONTRACTOR'S OPTION, SHALL BE INTENTIONALLY ROUGHENED IN ACCORDANCE WITH THE FOLLOWING: THE SURFACE OF ROUGHENED JOINTS SHALL BE SAND BLASTED OR ROUGHENED WITH A CHIPPING HAMMER TO EXPOSE THE AGGREGATE EMBEDDED IN THE PREVIOUS POUR. THE EXPOSED AGGREGATE SHALL PROTRUDE A MINIMUM OF 1/4 INCH. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED.

VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL BE HELD TO A MAXIMUM SPACING OF 40'—0".

ALL CONSTRUCTION JOINTS FOR BEAMS AND SLABS SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAILS. BEAMS AND SLABS HAVE BEEN DESIGNED ASSUMING ANY CONSTRUCTION JOINTS ARE LOCATED IN THE MIDDLE THIRD OF THE SPAN.

ALL CONSTRUCTION JOINTS IN SLABS ON STEEL DECK SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAILS. STEEL BEAMS AND GIRDERS HAVE BEEN DESIGNED ASSUMING THE CONSTRUCTION JOINTS ARE LOCATED IN THE MIDDLE THIRD OF THE BEAM, GIRDER, OR SLAB SPAN.

ALL CONSTRUCTION JOINTS IN SLABS, BEAMS, AND WALLS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE STARTING CONSTRUCTION. PROVIDE JOINTS AT LOCATIONS SPECIFICALLY NOTED ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS.

SLEEVES

EXCEPT AS DETAILED ON STRUCTURAL DRAWINGS, NO CONCRETE FOOTINGS, BEAMS, OR GIRDERS SHALL BE SLEEVED FOR PIPING OR DUCTS, UNLESS APPROVED BY THE ENGINEER.

ANCHORAGE TO HARDENED CONCRETE

ANCHORAGE TO HARDENED CONCRETE SHALL INCLUDE MECHANICAL AND ADHESIVE ANCHORS OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE DRAWINGS. HOLES SHALL BE DRILLED AND CLEANED AND ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTRUCTIONS AND AN APPROVED ICC—ES (OR IAPMO-UES EQUIVALENT) REPORT. INSPECTION AND TESTING SHALL BE PROVIDED IN ACCORDANCE WITH THE GENERAL NOTES AND THE APPROVED ICC—ES (OR IAPMO-UES EQUIVALENT) REPORT.

WHERE THE ANCHOR TYPE IS SPECIFIED ON THE DRAWINGS, SUBSTITUTION FOR A DIFFERENT TYPE OF ANCHORAGE (INCLUDING SUBSTITUTING FOR CAST-IN-PLACE ANCHORAGE) SHALL NOT BE PERMITTED WITHOUT PRIOR CONSENT OF THE ENGINEER.

ACCEPTABLE ANCHORS SHALL HAVE A CURRENT ICC—ES (OR IAPMO-UES EQUIVALENT) REPORT INDICATING THAT THE ANCHOR IS PERMITTED FOR RESISTING SEISMIC LOADS IN CRACKED CONCRETE. UNLESS NOTED OTHERWISE, ANCHORS SHALL BE ASTM A36 THREADED ROD OR ASTM A615, GRADE 60 REINFORCING STEEL DOWELS.

WHEN EMBEDMENT IS NOTED ON THE DRAWINGS, THE ANCHOR EFFECTIVE EMBEDMENT DEPTH SHALL EQUAL OR EXCEED THE NOTED EMBEDMENT DEPTH. WHERE NO EMBEDMENT IS NOTED ON THE DRAWINGS, THE MINIMUM EFFECTIVE ANCHOR EMBEDMENT DEPTH SHALL BE 6.5 ANCHOR DIAMETERS, MINIMUM DISTANCE TO THE NEAREST CONCRETE EDGE SHALL BE 12 ANCHOR DIAMETERS, AND MINIMUM ANCHOR SPACING SHALL BE 8 ANCHOR DIAMETERS.

STAINLESS STEEL ANCHORS SHALL BE USED AT ALL EXTERIOR LOCATIONS AND WHERE SPECIFICALLY INDICATED ON THE DRAWINGS. NO STEEL REINFORCEMENT SHALL BE CUT TO INSTALL ANCHORS. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT OR AN INJECTABLE ADHESIVE MATCHING THE ADJACENT CONCRETE COMPRESSIVE STRENGTH. NOTIFY THE STRUCTURAL ENGINEER OF DEFECTIVE OR ABANDONED HOLES IN WALLS AND COLUMNS. THESE ELEMENTS MAY REQUIRE NON-SHRINK GROUT WITH A COMPRESSIVE MODULUS OF ELASTICITY MATCHING THAT OF THE ADJACENT CONCRETE.

HOLES SHALL BE DRILLED WITH ROTARY IMPACT HAMMER OR EQUIVALENT METHOD TO PRODUCE A HOLE WITH A ROUGH INSIDE SURFACE. CORE DRILLING HOLES IS NOT PERMITTED. THE ADHESIVE SHALL BE MIXED, APPLIED, AND CURED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS IN THE ICC—ES (OR IAPMO-UES EQUIVALENT) REPORT. ALL PLACEMENT AND CURING SHALL BE CONDUCTED WITH CONCRETE AND AIR TEMPERATURES ABOVE 50 DEGREES FAHRENHEIT. ADHESIVE SHALL BE APPLIED ONLY TO CLEAN, DRY CONCRETE. POSITIVE PROTECTION SHALL BE PROVIDED SO THAT ANCHORS ARE NOT DISTURBED DURING THE CURING PERIOD. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT OR AN INJECTABLE ADHESIVE MATCHING THE ADJACENT CONCRETE COMPRESSIVE STRENGTH. NOTIFY THE STRUCTURAL ENGINEER OF DEFECTIVE OR ABANDONED HOLES IN WALLS AND COLUMNS. THESE ELEMENTS MAY REQUIRE NON-SHRINK GROUT WITH A COMPRESSIVE MODULUS OF ELASTICITY MATCHING THAT OF THE ADJACENT CONCRETE.

NONSHRINK GROUT FOR BASE PLATES, SLEEVES, AND EMBEDDED STEEL

GROUT SHALL BE AN APPROVED NONSHRINK CEMENTITIOUS GROUT CONTAINING NATURAL AGGREGATES DELIVERED TO THE JOB SITE IN FACTORY PREPACKAGED CONTAINERS REQUIRING ONLY THE ADDITION OF WATER. THE MINIMUM 28—DAY COMPRESSIVE STRENGTH SHALL BE AT LEAST 1,000 PSI HIGHER THAN THE SUPPORTING CONCRETE STRENGTH, UNLESS NOTED OTHERWISE. GROUT SHALL BE MIXED, APPLIED, AND CURED STRICTLY IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTRUCTIONS. FOR GROUTING UNDER BASE PLATES, GROUT SHALL BE PROPORTIONED AS A FLOWABLE MIX. WHEN A FLOWABLE MIX DOES NOT PROVIDE THE REQUIRED STRENGTH OR WHEN A MINIMUM STRENGTH OF 10,000 PSI IS REQUIRED, AN EPOXY GROUT SHALL BE USED.

EMBEDDED ELECTRICAL CONDUIT

ELECTRICAL CONDUIT SHALL BE RIGID STEEL CONDUIT OR FLEXIBLE PLASTIC CONDUIT. ALUMINUM CONDUIT IS PROHIBITED.

FOR CONDUIT PLACED IN CONCRETE FLAT SLABS OR SLABS THAT ARE PART OF A CONCRETE SLAB AND BEAM SYSTEM, CONDUIT SHALL HAVE A MAXIMUM OUTSIDE DIAMETER OF 1/6 TIMES THE SLAB THICKNESS AND SHALL BE EMBEDDED WITHIN THE MIDDLE THIRD OF THE SLAB DEPTH. MINIMUM CLEAR DISTANCE BETWEEN CONDUITS SHALL BE THREE TIMES THE CONDUIT DIAMETER. CONDUIT SHALL BE ROUTED TO MAINTAIN A MINIMUM CLEAR DISTANCE FROM PRESTRESSING TENDONS OF 1'-0" HORIZONTAL PARALLEL TO THE TENDONS AND 1 INCH VERTICAL PERPENDICULAR TO THE TENDONS. SEE THE TYPICAL CONDUIT PLACEMENT CRITERIA DETAIL.

FOR CONDUIT PLACED IN SLABS ON STEEL DECKING, CONDUIT SHALL BE RUN ABOVE THE STEEL DECK FLUTES PER THE TYPICAL CONDUIT IN SLAB ON STEEL DECK DETAIL. WHERE THE REQUIREMENTS OF THE TYPICAL CONDUIT IN SLAB ON STEEL DECK DETAIL CANNOT BE MET, CONDUIT SHALL ROUTE UNDER THE SLAB ON STEEL DECK OR AN ALTERNATE LOCATION WHICH SHALL BE COORDINATED BY THE CONTRACTOR WITH THE ARCHITECT AND OTHER TRADES.

CONDUIT SHALL BE FIRMLY CHAIRED AND TIED TO PREVENT DISPLACEMENT DURING POURING. PLACE #4 AT 12 INCHES ADDITIONAL REINFORCING ABOVE AND BELOW CONDUIT IN CONCRETE SLABS, PERPENDICULAR TO THE CONDUIT. THE ADDED REINFORCING SHALL EXTEND 1'-0" PAST THE CONDUIT ON BOTH SIDES. PLACE ADDITIONAL REINFORCING ABOVE CONDUIT RUNNING ABOVE STEEL DECK FLUTES AS REQUIRED BY THE TYPICAL CONDUIT IN SLAB ON STEEL DECK DETAIL.

POLYSTYRENE/RIGID INSULATION FOR BUILT-UP SLABS

POLYSTYRENE OR RIGID INSULATION PLACED BELOW CONCRETE SLABS SHALL CONSIST OF RIGID CELLULAR POLYSTYRENE CONFORMING TO ASTM D6817. POLYSTYRENE SHALL HAVE A MINIMUM COMPRESSIVE RESISTANCE OF 3.6 PSI AT 1 PERCENT DEFORMATION UNLESS NOTED OTHERWISE. SECURE POLYSTYRENE IN PLACE PER THE MANUFACTURER'S RECOMMENDATIONS. THE BLOCKS OF POLYSTYRENE SHALL BE PLACED TO OFFSET JOINTS 24 INCHES BETWEEN THE ADJACENT LAYERS.

AT THE CONTRACTOR'S OPTION, IN LIEU OF POLYSTYRENE CONFORMING TO ASTM D6817, PROVIDE POLYSTYRENE CONFORMING TO ASTM C578 TYPE XIV RATED FOR 40 PSI COMPRESSIVE RESISTANCE AT 10 PERCENT DEFORMATION WITH A MINIMUM THICKNESS OF 2 INCHES PER LAYER.

Reserved for permit stamp

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GENERAL NOTES

S0.03

STRUCTURAL STEEL

ALL STEEL SHALL CONFORM TO THE FOLLOWING:

W-SHAPES	ASTM A992, Fy=50 KSI ASTM A913, Fy=50 KSI
ALL ANGLES AND CHANNELS UNLESS NOTED OTHERWISE	ASTM A36, Fy=36 KSI
SQUARE OR RECTANGULAR STRUCTURAL TUBE (HSS)	ASTM A500, GRADE C, Fy=50 KSI
ROUND STRUCTURAL TUBE (HSS)	ASTM A500, GRADE C, Fy=46 KSI
STEEL PIPE DIAMETER LESS THAN OR EQUAL TO 12 INCHES	ASTM A53, TYPE E OR S, GRADE B, Fy=35 KSI
MATERIAL CALLED OUT ON PLANS AS (Fy=36 KSI)	ASTM A36, Fy=36 KSI
MATERIAL CALLED OUT ON PLANS AS (Fy=65 KSI)	ASTM A572, Fy=65 KSI
ALL OTHER STEEL UNLESS NOTED OTHERWISE	ASTM A572, Fy=50 KSI ASTM A588, Fy=50 KSI

GENERAL NOTES FOR STEEL CONNECTIONS SHALL APPLY TO ALL STEEL CONNECTIONS UNLESS NOTED OTHERWISE.

ALL WORK SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATION. SHOP DRAWINGS SHALL BE SUBMITTED AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE COMMENCING FABRICATION. ALL STEEL ANCHORS AND TIES AND OTHER MEMBERS EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED. DIMENSIONAL TOLERANCE FOR BUILT-UP MEMBERS SHALL BE PER AWS D1.1.

FOR ASTM A6 HOT-ROLLED SHAPES WITH A FLANGE THICKNESS OF 2 INCHES OR GREATER OR BUILT-UP SHAPES WITH A PLATE THICKNESS OF 2 INCHES OR GREATER THAT ARE SPLICED OR CONNECTED USING COMPLETE JOINT PENETRATION GROOVE WELDS, CHARPY V—NOTCH TESTING SHALL BE PROVIDED IN ACCORDANCE WITH ASTM A6 SUPPLEMENTARY REQUIREMENT S30, AS APPLICABLE, WITH A MINIMUM VALUE OF 20 FOOT—POUNDS AT 70 DEGREES FAHRENHEIT.

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AT THE MAXIMUM DECK SPAN LOCATION, UNLESS NOTED OTHERWISE. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 7/8-INCH-DIAMETER GR A325 BOLTS IN SINGLE SHEAR.

BOLTS DESIGNATED "GR A325" IN THE DRAWINGS REFER TO ASTM F3125 GRADE A325 HEAVY HEX BOLTS OR GRADE F1852 TWIST-OFF STYLE TENSION-CONTROL BOLTS. BOLTS DESIGNATED "GR A490" IN THE DRAWINGS REFER TO ASTM F3125 GRADE A490 HEAVY HEX BOLTS OR GRADE F2280 TWIST-OFF STYLE TENSION-CONTROL BOLTS. BOLT STYLE MAY BE SELECTED AT CONTRACTOR OPTION. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE RCSC.

BOLTS IN CONNECTIONS OF BEAM-TO-BEAM/GIRDER MAY BE SNUG-TIGHT, UNLESS SPECIFICALLY CALLED OUT PRETENSIONED OR SLIP-CRITICAL. SNUG-TIGHT CONNECTIONS SHALL BE INSTALLED PER THE CRITERIA FOR SNUG-TIGHT BOLTS.

ALL OTHER BOLTED CONNECTIONS SHALL BE PRETENSIONED. PRETENSIONED AND SLIP-CRITICAL CONNECTIONS MAY USE TURN-OF-NUT PRETENSIONING, TWIST-OFF-TYPE TENSION-CONTROL BOLT PRETENSIONING, OR DIRECT-TENSION-INDICATOR PRETENSIONING. WHERE SLIP-CRITICAL CONNECTIONS ARE NOTED, FAYING SURFACES SHALL BE CLASS A, UNLESS SPECIFICALLY CALLED OUT AS CLASS B.

ALL HIGH-STRENGTH BOLTS SHALL HAVE WASHERS INSTALLED PER THE RCSC, UNLESS NOTED OTHERWISE.

ALL ASTM A307 BOLTS SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF-LOCKING NUTS.

ALL BOLT HOLES SHALL BE STANDARD SIZE, UNLESS NOTED OTHERWISE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.

STRUCTURAL STEEL WELDING

STRUCTURAL STEEL SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY AWS CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1 AND AWS D1.8. WELDS SHOWN ON THE DRAWINGS ARE THE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE THICKNESS. THE MINIMUM WELD SIZE SHALL BE 3/16 INCH. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AWS D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS. ALL PARTIAL JOINT PENETRATION GROOVE WELD SIZES SHOWN ON THE DRAWINGS REFER TO EFFECTIVE THROAT THICKNESS, UNLESS NOTED OTHERWISE. FOR BASE METALS WITH MAXIMUM YIELD STRENGTH EQUAL TO 50 KSI, ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM 70 KSI). FOR BASE METALS WITH YIELD STRENGTH HIGHER THAN 50 KSI, ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES FROM WITHIN THE SAME GROUP PER AWS D1.1 TABLE 3.1. LOW HYDROGEN SMAW ELECTRODES SHALL BE STORED IN AN OVEN OR USED WITHIN THE ATMOSPHERIC TIME PERIODS SPECIFIED IN AWS D1.1 TABLE 5.1, OR SHALL BE REBAKED PER AWS D1.1 CLAUSE 5.3. ELECTRODES SHALL BE REBAKED NO MORE THAN ONE TIME, AND ELECTRODES THAT HAVE BEEN WET SHALL NOT BE USED.

FILLER METALS FOR ALL COMPLETE JOINT PENETRATION GROOVE WELDED T- AND CORNER JOINTS WITH BACKING LEFT IN PLACE AND COMPLETE JOINT PENETRATION GROOVE WELDED SPLICES IN HEAVY SECTIONS AS DEFINED IN AISC 360 A3.1c SHALL HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FOOT-POUNDS AT 40 DEGREES FAHRENHEIT.

ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1 AND AWS D1.8. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW PRIOR TO STARTING FABRICATION OR ERECTION. COPIES OF THE WPS SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.

ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATE LESS THAN OR EQUAL TO 1/4 INCH THICK, WHICH SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, AND TAPERS AND TRANSITIONS OF UNEQUAL PARTS.

FIREPROOFING STRUCTURAL STEEL

REFER TO ARCHITECTURAL PLANS FOR MINIMUM HOURLY VALUES OF STEEL FIRE PROTECTION FOR DETERMINING THE THICKNESS OF SPRAY APPLIED FIREPROOFING. THE STRUCTURAL FRAME CONSISTS OF COLUMNS AND GIRDERS, BEAMS, TRUSSES, AND SPANDRELS HAVING DIRECT CONNECTIONS TO THE COLUMNS AND BRACING MEMBERS DESIGNED TO CARRY GRAVITY LOADS. FLOOR OR ROOF MEMBERS THAT HAVE NO CONNECTION TO COLUMNS SHALL BE CONSIDERED SECONDARY MEMBERS.

ANCHOR RODS

ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 WITH CLASS 2A THREADS, UNLESS NOTED OTHERWISE. FURNISH ANCHOR RODS PREFABRICATED WITH MATCHING DOUBLE HEAVY HEX NUTS JAMMED AT THE END EMBEDDED IN CONCRETE. FURNISH PLATE WASHERS AND MATCHING HEAVY HEX NUTS FOR SECURING THE BASE PLATE TO THE ANCHOR RODS. HOOKED ANCHOR RODS SHALL NOT BE USED EXCEPT WHERE NOTED. A RIGID STEEL TEMPLATE SHALL BE USED TO LOCATE ANCHOR RODS WHILE PLACING CONCRETE. ANCHOR RODS SHALL HAVE SUFFICIENT LENGTH TO PROVIDE THE MINIMUM EMBEDMENT SHOWN ON THE DRAWINGS, MEASURED FROM THE FACE OF THE CONCRETE TO THE NEAR FACE OF THE DOUBLE NUT, WITH ADEQUATE EXTENSION AS REQUIRED TO RECEIVE THE BASE PLATE WITH ADEQUATE THREAD PROJECTION FOR FULL NUT ENGAGEMENT. ANCHOR ROD INSTALLATION SHALL BE COORDINATED WITH REINFORCING AND FORMWORK. LEVELING NUTS SHALL NOT BE USED EXCEPT AFTER EVALUATION BY THE CONTRACTOR'S ERECTION ENGINEER. AFTER BASE INSTALLATION, ANCHOR ROD NUTS SHALL BE INSTALLED TO A SNUG-TIGHT CONDITION. NO HEATING OR BENDING OF THE ANCHOR RODS IS PERMITTED. HOLES IN THE BASE MATERIAL SHALL NOT BE ENLARGED BY BURNING.

COMPOSITE FLOOR SYSTEM

FLOOR SLABS SHALL BE CONSTRUCTED TO THE THICKNESS SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR FLOOR TOLERANCES. THE CONTRACTOR SHALL INCLUDE THE QUANTITIES OF THE ADDED CONCRETE DUE TO THE STEEL DECK DEFLECTION. DESIGN CAMBER SHOWN FOR THE STEEL BEAMS HAS BEEN CALCULATED BASED ON THE DEFLECTION OF THE BEAM DUE TO THE WEIGHT OF THE STEEL AND CONCRETE SLAB.

MINIMUM SLAB REINFORCING IS WWR 6x6-W2.9xW2.9, UNLESS NOTED OTHERWISE.

SHEAR CONNECTOR STUDS

ALL SHEAR CONNECTOR STUDS SHALL BE 3/4 INCH IN DIAMETER UNLESS NOTED OTHERWISE. ACCEPTABLE TYPES SHALL BE "TRU-WELD" (ICC—ES ER—2577 OR IAPMO-UES EQUIVALENT) OR "NELSON" (ICC—ES ER—2856 OR IAPMO-UES EQUIVALENT). SHEAR CONNECTOR STUDS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER OF STUDS. STEEL STUD MATERIAL, WELDING, AND INSPECTION SHALL BE IN ACCORDANCE WITH AWS D1.1. SHEAR STUDS SHALL BE PLACED AT A MAXIMUM SPACING OF 2'—0" ON CENTER FOR ALL BEAMS SUPPORTING A STEEL DECK WITH CONCRETE FILL OR A CAST-IN-PLACE CONCRETE SLAB. THIS SPACING SHALL ALSO APPLY WHEN THE NUMBER OF STUDS IS NOT INDICATED ON THE PLANS. SEE "SHEAR STUD PLACEMENT" FOR LAYOUT CRITERIA. STEEL DECK SHOP DRAWINGS DETAILING THE SHEAR STUD PLACEMENT SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE INSTALLATION.

STEEL COMPOSITE DECK

THE STEEL DECK SHALL BE OF DEPTH SHOWN ON THE STRUCTURAL DRAWINGS. GAGE OF DECK SHALL BE DETERMINED BY THE CONTRACTOR BASED ON THE SPAN CONDITIONS, SHORING REQUIREMENTS, CONSTRUCTION LOADS, DEFLECTION REQUIREMENTS, AND THE SUPERIMPOSED LOADS SHOWN ON THE DRAWINGS, LOAD DIAGRAMS, AND NOTES. MINIMUM GAGE IS 20. MAXIMUM DEAD LOAD DEFLECTION IS 3/4 INCH OR L/180. WRITTEN VERIFICATION OF CONFORMANCE FOR ALL CONDITIONS IN THE STRUCTURE SHALL BE SUBMITTED FOR ACCEPTANCE PRIOR TO FABRICATION. THE CAPACITIES OF THE DECK SHALL BE BASED ON CURRENT ICC—ES (OR IAPMO-UES EQUIVALENT) REPORTS. SHOP DRAWINGS SHALL BE SUBMITTED SHOWING DECK GAGE, LAYOUT, FASTENING, STUD LAYOUT, AND CLOSURES. IF ANY SHORING IS TO BE USED, IT SHALL BE APPROVED BY THE GENERAL CONTRACTOR AND SHALL BE SHOWN ON THE SHOP DRAWINGS. UNITS SHALL SPAN OVER FOUR SUPPORTS, CONTINUOUS OVER THREE OR MORE SPANS, EXCEPT WHERE FRAMING DOES NOT PERMIT. THE AISI SPECIFICATIONS SHALL GOVERN THE DESIGN OF ALL DECK UNITS. STEEL DECK AND ALL OF ITS FLASHINGS SHALL CONFORM TO ASTM A653. THE STEEL SHALL HAVE RECEIVED, BEFORE BEING FORMED, A METAL PROTECTIVE COATING OF ZINC CONFORMING TO ASTM A653—G60. ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3.

CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL. UNLESS SHOWN OTHERWISE, UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE ENDS OF THE UNITS AND AT INTERMEDIATE SUPPORTS BY 3/4-INCH DIAMETER PUDDLE WELDS AT 1'-0" ON CENTER; WHERE TWO UNITS ABUT, EACH UNIT SHALL BE SO FASTENED TO THE STEEL SUPPORTS. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY 1 1/2—INCH TOP SEAM WELDS AT 2'—0" ON CENTER OR BUTTON PUNCHED AT 2'—0" ON CENTER. DECK UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4—INCH-DIAMETER PUDDLE WELDS AT 1'—0" ON CENTER. 3/4—INCH-DIAMETER SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN PLACE OF 3/4—INCH-DIAMETER PUDDLE WELDS. DESIGN AND PROVIDE FLASHING AND CLOSURE PLATES AT WALL ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. COORDINATE ALL CLOSURES WITH ELEVATOR, STAIR, ESCALATOR AND OTHER ARCHITECTURAL DETAILS. THE DECK INSTALLATION, WHEN COMPLETE, SHALL BE READY TO RECEIVE CONCRETE.

STEEL DECK TYPES SHALL BE VERCO TYPE W, ASC TYPE W, OR APPROVED EQUAL.

CONTRACTOR SHALL USE THE NECESSARY CONCRETE PLACEMENT AND FINISH METHODS TO ACHIEVE THE SPECIFIED CONCRETE THICKNESS AND SHALL TAKE THE NECESSARY MEASURES DURING CONCRETE PLACEMENT SO AS NOT TO OVERLOAD THE DECK.

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STEEL ROOF DECK

THE STEEL DECK SHALL BE OF DEPTH AND GAGE SHOWN ON THE STRUCTURAL DRAWINGS. STEEL DECK AND ALL OF ITS FLASHINGS SHALL CONFORM TO ASTM A653 AND SHALL HAVE CURRENT ICC-ES (OR IAPMO-UES EQUIVALENT) REPORTS. THE STEEL DECK SHALL HAVE RECEIVED, BEFORE BEING FORMED, A METAL PROTECTIVE COATING OF ZINC CONFORMING TO ASTM A653—G60. ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3. UNITS SHALL SPAN OVER FOUR SUPPORTS, CONTINUOUS OVER THREE OR MORE SPANS, EXCEPT WHERE THE FRAMING DOES NOT PERMIT.

UNLESS NOTED OTHERWISE, NONCOMPOSITE UNITS SHALL BE CONNECTED AS FOLLOWS:

CONNECT DECK TO THE STEEL SUPPORTS AT THE ENDS OF THE UNITS AND AT INTERMEDIATE SUPPORTS BY A MINIMUM OF FOUR 3/4-INCH PUDDLE WELDS PER 3'—0" OF WIDTH. WHERE TWO UNITS ABUT, EACH UNIT SHALL BE SO CONNECTED TO THE STEEL FRAMING.

THE SIDE LAPS OF ADJACENT UNITS SHALL BE CONNECTED BETWEEN SUPPORTS BY 1-1/2" TOP SEAM WELDS AT A MAXIMUM SPACING OF 2'—0" ON CENTER.

DECK UNITS SHALL BE CONNECTED TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES WITH 3/4 INCH PUDDLE WELDS AT THE SAME SPACING AS THE SIDE LAP CONNECTIONS.

WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND CONNECT DECK TO STEEL WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.

STEEL DECK THAT IS TO BE COVERED WITH INSULATING CONCRETE SHALL BE SLOTTED OR PERFORATED TO PROVIDE A MINIMUM OF 1.5 PERCENT UNIFORMLY DISTRIBUTED VENTING. PROVIDE FLASHING AND CLOSURE PLATES AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. THE DECK INSTALLATION, WHEN COMPLETE, SHALL BE READY TO RECEIVE INSULATING CONCRETE.

SHOP DRAWINGS SHALL BE SUBMITTED SHOWING DECK DEPTH, GAGE, LAYOUT, CONNECTIONS, AND CLOSURES

STEEL DECK TYPES SHALL BE VERCO TYPE N—24, ASC TYPE N, OR APPROVED EQUAL.

MASONRY

CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE BUILDING CODE. ALL HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, NORMAL WEIGHT. MINIMUM REQUIRED BLOCK COMPRESSIVE STRENGTH IS 2,000 PSI. ALL CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLID WITH CONCRETE GROUT. GROUT MIX SHALL CONTAIN PORTLAND CEMENT, AGGREGATE, AND A GROUT-ENHANCING SHRINKAGE-COMPENSATING ADDITIVE. MAXIMUM SIZE OF AGGREGATE SHALL BE 3/8 INCH. SLUMP SHALL BE 8 TO 11 INCHES. WATER-REDUCING ADMIXTURES MAY BE USED. MINIMUM GROUT COMPRESSIVE STRENGTH BASED ON 28—DAY TESTS SHALL EQUAL OR EXCEED THE SPECIFIED f'm AND BE GREATER THAN OR EQUAL TO THE SPECIFIED MINIMUM DESIGN STRENGTH. GROUT SHALL BE VIBRATED WHILE PLACING TO ENSURE THAT CELLS ARE COMPLETELY FILLED. SUBMIT GROUT MIXES TO ARCHITECT FOR REVIEW BEFORE COMMENCING MASONRY CONSTRUCTION. ALL UNITS SHALL BE LAID IN RUNNING BOND USING TYPE S MORTAR WITH HEAD JOINTS. MASONRY MINIMUM DESIGN STRENGTH IS f'm = 2,000 PSI.

REQUIRED MORTAR PROPORTIONS BY VOLUME

TYPE	PORTLAND CEMENT	HYDRATED LIME	AGGREGATE MEASURED IN A DAMP, LOOSE CONDITION
S	1	OVER 1/4 TO 1/2	NOT LESS THAN 2 1/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE VOLUMES OF THE CEMENT

COLUMN SHORTENING AND BEAM DEFLECTION

COLUMN SHORTENING WILL OCCUR DUE TO THE WEIGHT OF THE CONSTRUCTION ABOVE. THIS SHORTENING WILL CONTINUE UNTIL ALL OF THE DEAD LOAD IS ON THE STRUCTURE, INCLUDING THE CLADDING. THE COLUMNS SHALL BE FABRICATED LONGER THAN THE FINAL LENGTHS SHOWN IN THE CONSTRUCTION DOCUMENTS TO COMPENSATE FOR THIS SHORTENING. IN ADDITION, THE CONTRACTOR SHALL SUPPLY SHIMMING OR MILLING AS REQUIRED DUE TO NORMAL CONSTRUCTION TOLERANCES AND ERECTION PROCEDURES. DIFFERENTIAL COLUMN SHORTENING OCCURS WHEN COLUMNS STOP AT DIFFERENT LEVELS OR ARE SUBJECT TO TRANSFER BEAM DEFLECTION.

FLOOR BEAMS, ESPECIALLY EDGE BEAMS, TRANSFER GIRDERS, AND CANTILEVERS WILL CONTINUE TO DEFLECT WHEN ADDITIONAL LOAD IS APPLIED. THESE MEMBERS HAVE BEEN CAMBERED TO COMPENSATE FOR THE THEORETICAL DEFLECTION. HOWEVER, THIS MAY NOT OCCUR UNTIL ALL THE DEAD LOAD IS APPLIED TO THE MEMBER. THE CONTRACTOR SHALL COORDINATE THE ATTACHMENT OF ANY ITEMS TO MEMBERS WHICH WILL CONTINUE TO SHORTEN OR DEFLECT DUE TO LATER STAGES OF CONSTRUCTION.

EXTERIOR CLADDING

THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE CLADDING SYSTEMS, INCLUDING THEIR STRUCTURAL INTEGRITY, WATERPROOFING SYSTEMS, AND CONNECTION TO THE PRIMARY STRUCTURE.STRUCTURAL ELEMENTS AT THE BUILDING PERIMETER HAVE BEEN DESIGNED FOR THE VERTICAL LOADS SHOWN ON THE LOAD MAPS. CLADDING ATTACHMENTS SHALL NOT APPLY MOMENTS TO SLAB EDGES OR LATERAL LOADS TO STEEL BEAMS OR INTRODUCE TORSIONAL LOADS INTO STEEL BEAMS OR COLUMNS. BRACES, ADDED REINFORCING, AND/OR TIES SHALL BE DESIGNED AND SUPPLIED BY THE CONTRACTOR FOR LOAD ECCENTRICITIES AND LATERAL LOADS. THE CONTRACTOR SHALL SUPPLY ALL CONNECTION MATERIAL, BRACES, ETC. EXPANSION BOLTING TO POST-TENSIONED SLABS IS NOT PERMITTED EXCEPT AS NOTED IN THE "POST-TENSIONED PRESTRESSED CONCRETE" NOTES. SUBMITTED DOCUMENTS SHALL INDICATE MAGNITUDE AND LOCATION OF ALL LOADS IMPOSED ON THE PRIMARY STRUCTURE.

EXTERIOR CLADDING CONNECTIONS SHALL ACCOUNT FOR STRUCTURAL DEFLECTION, COLUMN SHORTENING, AND CONSTRUCTION TOLERANCE. IN ADDITION, THE CLADDING DESIGN SHALL ACCOMMODATE A TYPICAL VERTICAL MOVEMENT AT EACH FLOOR OF 1/2 INCH DUE TO VARIABLE LIVE LOADING. THIS DISPLACEMENT WILL OCCUR AT THE FREE END OF CANTILEVER BEAMS AND AT MIDSPAN OF EDGE SLABS AND BEAMS.

THE CLADDING SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS PERPENDICULAR AND/OR PARALLEL TO THE WALL AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES. THE CLADDING SHALL REMAIN UNDAMAGED UNDER THE SERVICE-LEVEL STORY DRIFT AND SHALL NOT FALL FROM THE BUILDING UNDER THE DESIGN STORY DRIFT.

STAIRS, ELEVATORS, AND MISCELLANEOUS METALS

UNLESS SHOWN AND DETAILED IN THE STRUCTURAL DRAWINGS, ALL STAIRS ARE TO CONSIST OF A PRE-FABRICATED AND PRE-ENGINEERED STAIR, LANDING, AND RAILING SYSTEM DESIGNED BY THE CONTRACTOR OR STAIR SUPPLIER. SEE THE ARCHITECT FOR STAIR SYSTEM LAYOUT, DIMENSIONS, AND CONFIGURATION OF RISE AND RUN. THE CONTRACTOR SHALL BE RESPONSIBLE TO DESIGN AND PROVIDE THE STAIR SYSTEM INCLUDING ALL CONNECTIONS AND SECONDARY SUPPORT FRAMING. WHERE REQUIRED BY THE BUILDING CODE, THE STAIRS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES. UNDER THE SERVICE LEVEL STORY DRIFTS, ALL STAIRS MUST REMAIN FUNCTIONAL. UNDER THE DESIGN STORY DRIFTS ALL EGRESS STAIRS MUST REMAIN FUNCTIONAL AND ALL OTHER STAIRS MUST REMAIN CONNECTED TO THE BUILDING.

ALL ELEVATOR MACHINE BEAMS, HOIST BEAMS, SILLS, DOOR SUPPORTS, AND RAILS AND THEIR CONNECTIONS TO THE PRIMARY STRUCTURE ARE TO BE DESIGNED BY THE ELEVATOR MANUFACTURER. THE CONTRACTOR SHALL PROVIDE ADDITIONAL FRAMING AS NECESSARY FOR MACHINE ROOM FLOOR PENETRATIONS PER THE TYPICAL DETAILS. THE ELEVATOR MACHINE BEAMS SHALL BE DESIGNED FOR THE TRIBUTARY LOADS INDICATED IN THE LOAD MAPS IN ADDITION TO THE WEIGHT OF THE SUPPORTED EQUIPMENT AND SELF-WEIGHT OF THE MACHINE ROOM FLOOR/ROOF STRUCTURE. CONNECTIONS BETWEEN ELEVATOR MACHINE BEAMS AND PRIMARY STRUCTURES SHALL BE COORDINATED WITH THE STRUCTURAL ENGINEER OF RECORD. WHERE REQUIRED BY THE BUILDING CODE, THE ELEVATORS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES.

THE CONTRACTOR SHALL DESIGN AND SUPPLY ALL ADDITIONAL MISCELLANEOUS METALS THAT ARE INDICATED IN THE ARCHITECTURAL DRAWINGS OR THOSE METALS WHICH ARE FOUND TO BE NECESSARY TO SUPPORT THE ARCHITECTURAL FINISHES OR OTHER BUILDING SYSTEMS. WHERE REQUIRED BY THE BUILDING CODE, THE MISCELLANEOUS METALS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES.

ALL FRAMING AND CONNECTIONS DESIGNED BY THE CONTRACTOR SHALL NOT RESULT IN ECCENTRIC LOADS BEING APPLIED TO THE PRIMARY STRUCTURE NOR LATERAL LOADS BEING APPLIED TO THE BOTTOM FLANGE OF STEEL BEAMS. THE CONTRACTOR'S DESIGN SHALL VERIFY THAT THE CONNECTIONS DO NOT RESULT IN ADVERSE LOCAL CONNECTION STRESSES OCCURRING WITHIN THE PRIMARY STRUCTURE. SUBMIT CALCULATIONS STAMPED BY AN ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED AND SHOP DRAWINGS INDICATING IMPOSED LOADS ON THE PRIMARY STRUCTURE.

MECHANICAL/ELECTRICAL/PLUMBING SYSTEM SUPPORTS

THE CONTRACTOR SHALL DESIGN AND SUPPLY ALL ADDITIONAL MISCELLANEOUS METALS AND SYSTEM SUPPORT COMPONENTS THAT ARE NECESSARY TO SUPPORT ALL MECHANICAL, ELECTRICAL (TELECOM, AUDIO VISUAL, ETC), AND PLUMBING/FIRE-PROTECTION SYSTEMS. SUCH METALS AND SUPPORT COMPONENTS AND THEIR CONNECTIONS SHALL BE PROVIDED AS NECESSARY TO DIRECTLY AND CONCENTRICALLY IMPOSE LOADS ON THE PRIMARY STRUCTURE. THE CONNECTIONS TO THE PRIMARY STRUCTURE ARE SUBJECT TO THE REQUIREMENTS OF THE MISCELLANEOUS METALS SECTION ABOVE. THESE SYSTEMS MAY BE SUPPORTED DIRECTLY FROM STEEL ROOF AND COMPOSITE FLOOR/ROOF SLABS SUBJECT TO THE FOLLOWING LIMITATIONS: 250 POUNDS MAY HANG FROM COMPOSITE SLAB ON DECK, 50 POUNDS MAY HANG FROM STEEL ROOF DECK. LOADS SHALL BE LOCATED NO CLOSER THAN 5 FEET FROM ANY ADJACENT HANGING LOAD, AND THE CONTRACTOR SHALL COORDINATE THE SUPPORT AND HANGING LOADS FROM ALL BUILDING SYSTEMS. WHERE REQUIRED BY THE BUILDING CODE, THE MECHANICAL/ELECTRICAL/PLUMBING SYSTEM SUPPORTS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES.

INTERIOR METAL STUD FRAMING

INTERIOR PARTITIONS SHALL CONSIST OF METAL STUD TYPE FRAMING THAT HAS CURRENT ICC—ES (OR IAPMO-UES EQUIVALENT) EVALUATION REPORTS. CONNECTION OF STUDS, TRACK, AND OTHER ITEMS BY MEANS OF EITHER DRILLED-IN ANCHORAGE OR POWDER DRIVEN FASTENERS SHALL OCCUR WITH FASTENERS AS INDICATED IN THE METAL STUD ICC—ES (OR IAPMO-UES EQUIVALENT) REPORTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE STRUCTURAL DESIGN OF SOFFITS, SUSPENDED WALLS, CEILINGS, OR CONDITIONS WHERE THE STUD FRAMING IS USED TO SUPPORT CASEWORK OR SIZEABLE DOOR/WINDOW HARDWARE; THE METAL STUD FRAMING; AND ANY MISCELLANEOUS STEEL FRAMING THAT IS DETERMINED TO BE NECESSARY BASED ON THE CONTRACTOR'S DESIGN.

INTERIOR METAL STUD FRAMING AND ITS CONNECTIONS TO THE STRUCTURE SHALL BE DESIGNED TO ACCOMMODATE UP TO 3/4" OF VERTICAL MOVEMENT DUE TO VARIABLE LIVE LOADING. WHERE REQUIRED BY THE BUILDING CODE, INTERIOR METAL STUD FRAMING SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES.

SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS INDICATING IMPOSED LOADS ON THE PRIMARY STRUCTURE FOR THESE CONDITIONS. SUBMITTED DOCUMENTS SHALL BEAR THE STAMP AND SIGNATURE OF AN ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED.

BUILDING TOLERANCES

STANDARD TOLERANCES SHALL BE BASED ON THE REQUIREMENTS OF THE AISC CODE OF STANDARD PRACTICE AND ACI 117, "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS".

STRENGTH AND STABILITY DURING CONSTRUCTION

DURING CONSTRUCTION, THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE.

A COMPLETED STRUCTURE IS REQUIRED TO PROVIDE GLOBAL STABILITY, TO PROVIDE LOCAL STABILITY OF INDIVIDUAL STRUCTURAL COMPONENTS (SLABS, DECKS, BEAMS, COLUMNS, WALLS, ETC.), AND TO RESIST IMPOSED LOADS.

THE STRUCTURE WAS ANALYZED AND DESIGNED BY MKA CONSIDERING ITS COMPLETED STATE ONLY. THE DESIGN DID NOT EVALUATE PARTIALLY COMPLETED CONSTRUCTION STAGES.

THE CONTRACTOR SHALL CONSIDER ALL ASPECTS OF CONSTRUCTION SEQUENCING. CONSIDERATIONS SHALL INCLUDE BUT NOT BE LIMITED TO STEEL ERECTION AND CONCRETE PLACEMENT, CRANE REQUIREMENTS, TEMPORARY SHORING, BRACING/STRENGTHENING, TEMPORARY CONSTRUCTION LOADS, SAFETY PROCEDURES, TEMPERATURE CHANGE, AND MOISTURE EFFECTS.

THE CONTRACTOR SHALL EVALUATE THE NEED FOR AND RETAIN AS NECESSARY AN ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED TO REVIEW ALL STAGES OF CONSTRUCTION SEQUENCING, VALIDATE ALL TEMPORARY CONSTRUCTION LOADS, AND PREPARE A COMPREHENSIVE CONSTRUCTION/ERECTION PLAN TO ADDRESS BOTH STABILITY AND RESISTANCE TO IMPOSED LOADS UNTIL THE STRUCTURE IS COMPLETE.

TEMPORARY SUPPORTS, TEMPORARY CONNECTIONS, AND/OR CONSTRUCTION/ ERECTION AIDS SHALL BE REMOVED BY THE CONTRACTOR AFTER THEY ARE NO LONGER REQUIRED.

EXISTING STRUCTURE

EXISTING STRUCTURAL DIMENSIONS AND MEMBER SIZES ARE FOR REFERENCE ONLY. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO FABRICATION. THE CONTRACTOR SHALL VERIFY THE ACTUAL CONFIGURATION OF EXISTING CONSTRUCTION AND THE CONDITION OF THE STRUCTURE BEFORE BEGINNING WORK. ANY DISCREPANCIES OR UNSOUND CONDITIONS SHALL BE REPORTED TO THE ARCHITECT FOR RESOLUTION BEFORE BEGINNING WORK. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS, EMBEDMENTS, AND OPENINGS NOT SHOWN. REFER TO MECHANICAL AND ELECTRICAL PLANS FOR DUCTS, PIPING, EMBEDMENTS, AND OPENINGS NOT SHOWN.

TEMPORARY SHORING AND BRACING MAY BE NECESSARY IN ORDER TO PERFORM THE NECESSARY STRUCTURAL MODIFICATIONS TO THE EXISTING STRUCTURE SHOWN ON THE STRUCTURAL AND ARCHITECTURAL PLANS AND DETAILS. THE CONTRACTOR MUST RETAIN AN ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, WHO SHALL INVESTIGATE WHERE THIS TEMPORARY SHORING/BRACING IS REQUIRED AND SHALL DESIGN THIS TEMPORARY SHORING/BRACING.

MISCELLANEOUS

REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL, ELEVATOR, OR OTHER SPECIALTY ENGINEERING DRAWINGS FOR DIMENSIONS NOT SHOWN, INCLUDING BUT NOT LIMITED TO: SIZE AND LOCATION OF CURBS, EQUIPMENT HOUSEKEEPING PADS, WALL AND FLOOR OPENINGS, BLOCKOUTS, FLOOR DEPRESSIONS, SUMPS, DRAINS, ANCHOR BOLTS, EMBEDDED ITEMS, ARCHITECTURAL TREATMENT, ETC. THE CONTRACTOR SHALL VERIFY DIMENSIONS AND RESOLVE DISCREPANCIES OR CONFLICTS PRIOR TO CONSTRUCTION.

WHERE SECTIONS ARE INDICATED ON THE PLAN BY A NUMBER AND A DRAWING NUMBER THUS, 1/S5.01, THE INDICATED SECTION (1) IS SHOWN ON STRUCTURAL DRAWING S5.01.

SHOP DRAWINGS

SHOP DRAWINGS FOR REINFORCING STEEL AND STRUCTURAL STEEL SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.

THE CONTRACTOR SHALL SUBMIT CONCRETE WALL ELEVATION DRAWINGS OF AT LEAST 1/8" = 1'—0" SCALE INDICATING LOCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS FOR REVIEW PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL COORDINATE WITH REINFORCEMENT DRAWINGS.

REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL CONCRETE DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DEVELOP DETAILED SLAB EDGE AND CONCRETE OUTLINE DRAWINGS THAT ARE BASED ON THE ARCHITECTURAL, STRUCTURAL, AND MEP DRAWINGS. THE DETAILED EDGE AND OUTLINE DRAWINGS SHALL BE SUBMITTED FOR REVIEW. SUBMITTED DRAWINGS SHALL CONTAIN ALL CONCRETE CURBS, FORM OUTLINES, AND EMBEDDED ITEMS. DIMENSIONS AND OUTLINES DEVELOPED BY THE CONTRACTOR MAY VARY FROM THOSE SHOWN BY THE ARCHITECT AND ENGINEER AS NECESSARY BASED ON THE DEPENDENCY ON ADJACENT MATERIALS THAT ARE DETERMINED BY THE CONTRACTOR AND/OR SUPPLIER (EXTERIOR CLADDING, ELEVATOR EQUIPMENT, FINAL MEP SHAFT SIZES, ETC.). CONCRETE OUTLINES SHALL BE ADJUSTED AS NECESSARY TO ACCOUNT FOR CONSTRUCTION METHODS AND FOR SLAB SHRINKAGE. THE CONCRETE OUTLINE DEVELOPED BY THE CONTRACTOR SHALL NOT MATERIALLY ALTER THE DESIGN INTENT SHOWN IN THE STRUCTURAL DRAWINGS.

DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD; THEREFORE, THEY SHALL BE VERIFIED BY THE CONTRACTOR. THE CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY THE ENGINEER OF RECORD. THE CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.

SUBMITTALS SHALL BE PROVIDED ELECTRONICALLY WHENEVER POSSIBLE AND WILL BE MARKED AND RETURNED ELECTRONICALLY. WHEN HARD COPY SUBMITTALS ARE REQUIRED, ONE ORIGINAL AND ONE COPY SHALL BE PROVIDED; THE REPRODUCIBLE COPY WILL BE MARKED AND RETURNED.

SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED, AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWINGS SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.

SHOP DRAWINGS FOR DEFERRED SUBMITTALS THAT ARE DEFINED AS DESIGN-BUILD COMPONENTS IN THE CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY AN ENGINEER LICENSED TO PERFORM WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. DESIGN CALCULATIONS SHALL BE INCLUDED IN THE SUBMITTAL.

DEFERRED STRUCTURAL SUBMITTALS

SOME STRUCTURAL SYSTEMS ARE DEFINED AS VENDOR-DESIGNED COMPONENTS PER THE STRUCTURAL DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. THE CONTRACTOR WILL BE REQUIRED TO SUBMIT THE STAMPED COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL.

DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT, WHO SHALL REVIEW THEM FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE CONTRACTOR SHALL SUBMIT THESE REVIEWED DEFERRED SUBMITTAL DOCUMENTS TO THE BUILDING OFFICIAL. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

THE FOLLOWING LIST INCLUDES THE ITEMS THAT ARE DEFINED AS DEFERRED STRUCTURAL SUBMITTAL COMPONENTS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND CIVIL DRAWINGS FOR ADDITIONAL DEFERRED SUBMITTAL COMPONENTS.

DEFERRED STRUCTURAL SUBMITTAL COMPONENTS:

ANCHORAGE FOR MECHANICAL/ELECTRICAL/PLUMBING SYSTEMS

EXTERIOR CLADDING

GUARDRAILS AND HANDRAILS

METAL STAIRS AND LANDINGS

METAL STUD SYSTEMS

POST-TENSIONING SYSTEMS

SPECIAL INSPECTION

THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION AND TESTING PER IBC SECTION 1705. THIS WORK SHALL BE PERFORMED BY A SPECIAL INSPECTOR CERTIFIED BY THE CITY OF PARK CITY TO PERFORM THE TYPES OF INSPECTIONS AND TESTS SPECIFIED. THE FREQUENCY OF INSPECTIONS AND TESTING SHALL BE AS OUTLINED IN THE IBC TABLE ITEMS LISTED BELOW. DEFICIENCIES SHALL BE REPORTED DAILY TO THE CONTRACTOR. SUMMARY REPORTS SHALL BE DISTRIBUTED WEEKLY TO THE OWNER, ARCHITECT, CONTRACTOR, BUILDING OFFICIAL, AND STRUCTURAL ENGINEER. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTION AND TESTING.

ITEM	DESCRIPTION (REFER TO IBC SECTION 1705)	IBC TABLE REQUIREMENTS
STRUCTURAL STEEL AND WELDING	STRUCTURAL STEEL THAT IS PART OF THE STRUCTURE.	SECTION 1705.2
HIGH STRENGTH BOLTING	SEE SPECIFICATIONS FOR PROCEDURES FOR INSPECTION AND TESTING.	SECTION 1705.2
CONCRETE	CONCRETE THAT IS PART OF THE STRUCTURE.	SECTION 1705.2, TABLE 1705.3, ITEMS 5, 6, 7, 8, 11, 12
SHOTCRETE		TABLE 1705.3, ITEMS 5, 6, 7, 8
ANCHORS CAST IN CONCRETE		TABLE 1705.3, ITEM 3
ANCHORS POST-INSTALLED IN HARDENED CONCRETE	INSTALLATION OF MECHANICAL AND ADHESIVE ANCHORS.	TABLE 1705.3, ITEM 4
REINFORCING STEEL AND PRESTRESSING STEEL TENDONS	A. STRESSING AND GROUTING OF TENDONS.	TABLE 1705.3, ITEM 9
	B. PLACEMENT OF REINFORCING STEEL AND PRESTRESSING TENDONS.	TABLE 1705.3, ITEM 1
	C. SPLICING OF REINFORCING BY WELDING OR THREADED COUPLERS.	TABLE 1705.3, ITEM 1, 2
	D. PRESTRESSING TENDON ANCHORAGES/STRESSING POCKETS: PERIODIC SPECIAL INSPECTION SHALL INCLUDE THE FOLLOWING: VERIFY THAT GREASE COVERS ARE SECURELY IN PLACE AND THAT THE CORROSION PROTECTION IS INTACT FOR ENCAPSULATED TENDONS. OBSERVE THAT THE STRESSING POCKET HAS BEEN CLEANED, COATED WITH APPROVED BONDING AGENT, AND FILLED WITH AN APPROVED NON-SHRINK GROUT.	N/A
STRUCTURAL MASONRY	ALL MASONRY SHOWN ON STRUCTURAL DRAWING INCLUDING MASONRY SHOWN IN TYPICAL DETAILS BUT LOCATED ON ARCHITECTURAL DRAWINGS.	SECTION 1705.4
SOILS		SECTION 1705.6, TABLE 1705.6
SEISMIC RESISTANCE		SECTION 1705.12

STRUCTURAL OBSERVATION

THE ENGINEER OF RECORD SHALL PROVIDE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM, FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS, AT SIGNIFICANT CONSTRUCTION STAGES AND AT THE COMPLETION OF THE STRUCTURAL SYSTEM. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY IBC SECTIONS 110, 1705, OR OTHER SECTIONS OF THE INTERNATIONAL BUILDING CODE. STRUCTURAL OBSERVATION REPORTS SHALL BE ISSUED TO THE OWNER, ARCHITECT, CONTRACTOR, AND BUILDING OFFICIAL AT SIGNIFICANT CONSTRUCTION STAGES.

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date 7/15/2022

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GENERAL NOTES

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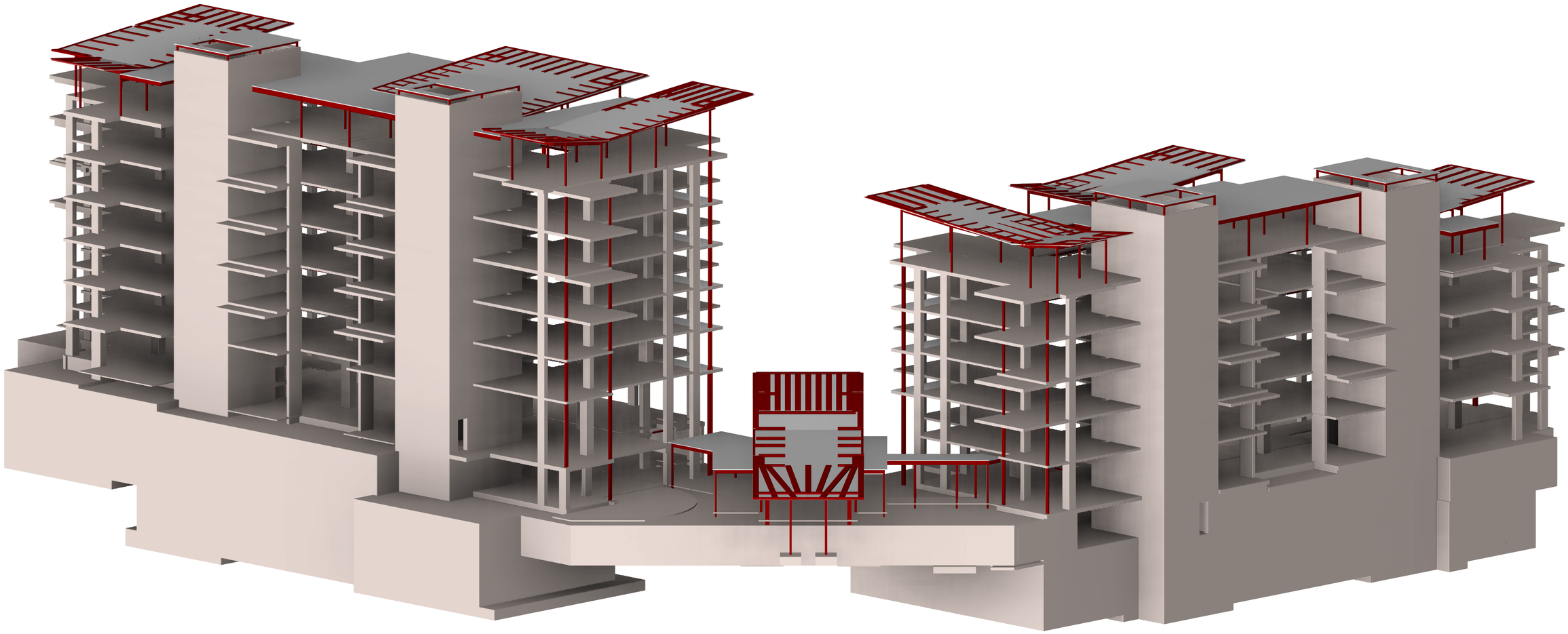

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ISOMETRIC VIEWS

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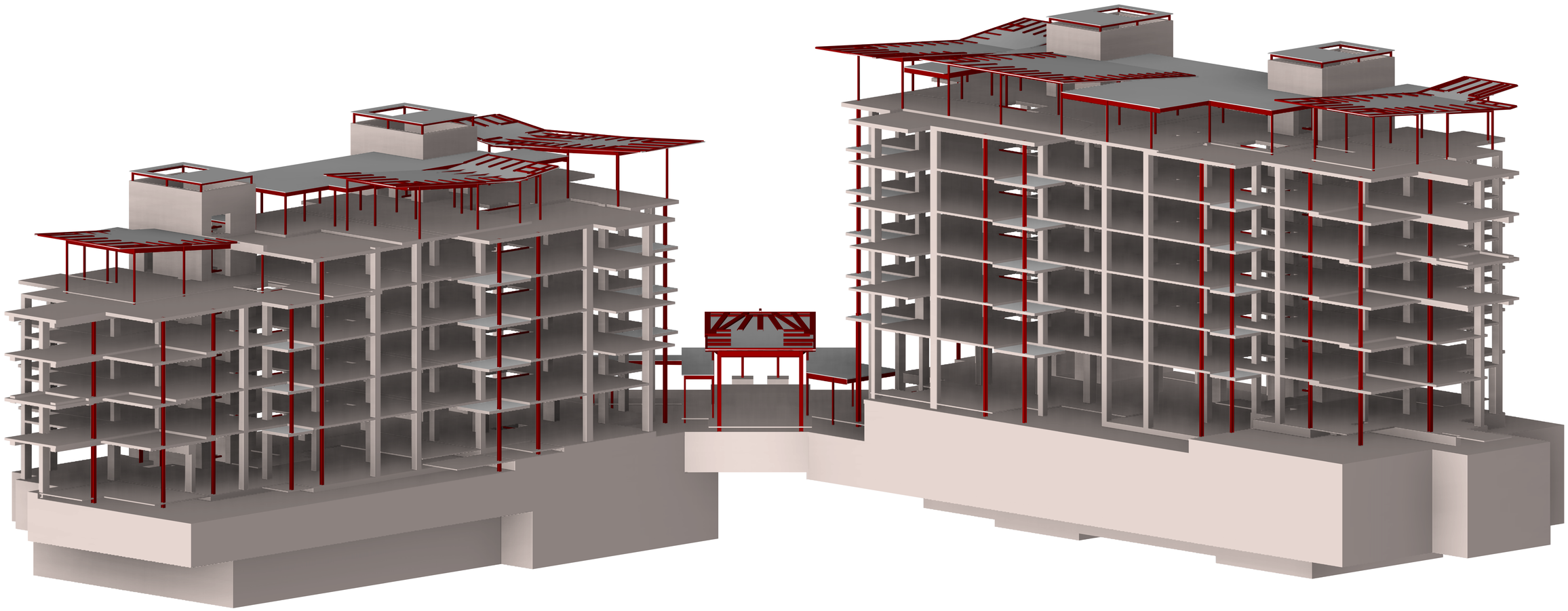

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ISOMETRIC VIEWS

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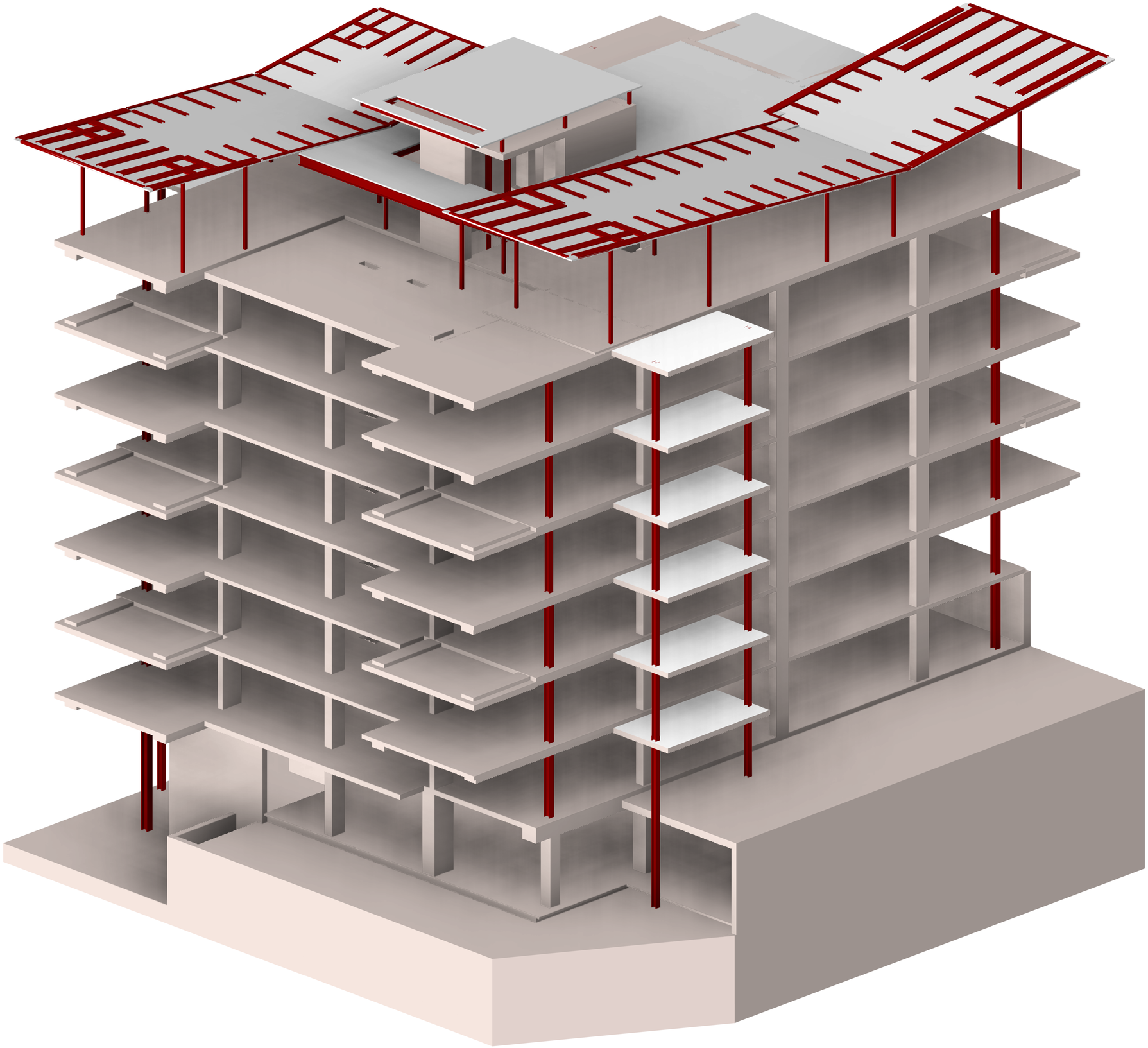

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ISOMETRIC VIEWS

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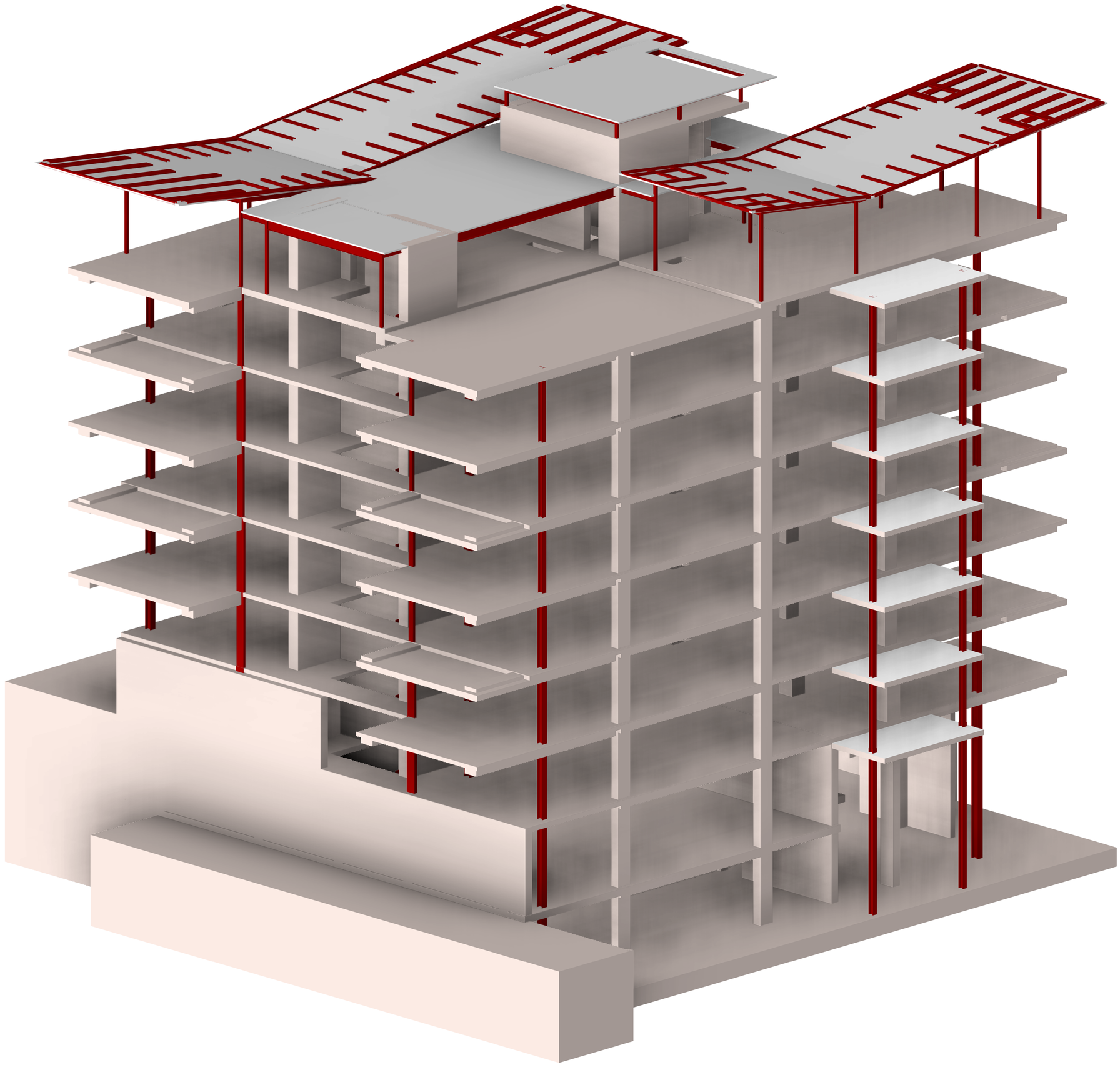

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ISOMETRIC VIEWS

S0.08

LOAD MAP KEY:

A 1

NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK

LETTER INDICATES LIVE LOAD MARK

INDICATES CLADDING LOAD IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

LIVE LOAD (LL) DESIGNATIONS

MARK	USE	LIVE LOAD (PSF)
A	RESIDENTIAL	40(R)
B	MECHANICAL / ELECTRICAL	125
C	PARKING	40(R) (20%)
D	LIGHT STORAGE	125
E	ASSEMBLY / CORRIDORS	100
F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
N	GROUND FLOOR TERRACE	100

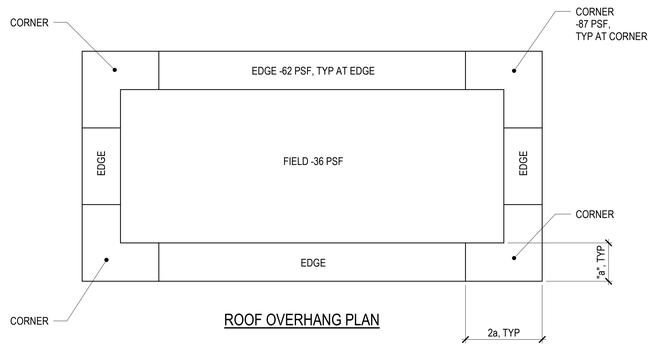
SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
1	RESIDENTIAL	30	5	5	20		
2	MECHANICAL / ELECTRICAL	15	10		5		
3	PARKING	5	5				
4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
11	FITNESS	65	5	5	5	50	ISOLATION SLAB
12	GREEN ROOF	40	10	5		25	INSULATION + LIGHT GREEN ROOF
13	ROOF	25	10			15	INSULATION + ROOFING
14	DEEP SOIL	330	10			320	SOIL DEPTH TBC

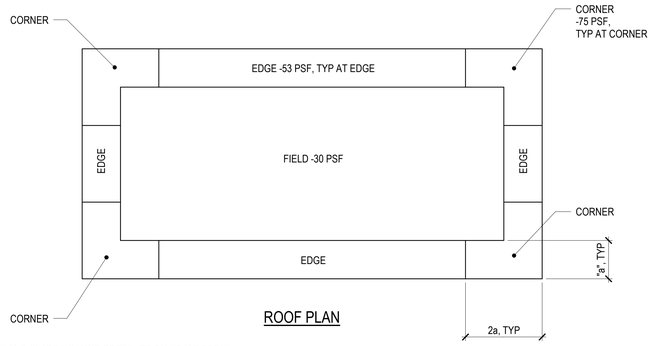
LOAD MAP NOTES:

1. LIVE LOADS MARKED (R) ARE REDUCIBLE IN ACCORDANCE WITH THE BUILDING CODE.
2. SUPERIMPOSED DEAD LOADS ARE IN ADDITION TO THE SELF-WEIGHT OF THE STRUCTURE. BUILT-UP SLABS SHOWN ON PLAN ARE CONSIDERED TO BE PART OF THE SELF-WEIGHT OF THE STRUCTURE.
3. SEE FRAMING PLANS FOR DESIGN LOAD OF SPECIFIC ITEMS SUCH AS ELEVATORS, ESCALATORS, AND MECHANICAL / ELECTRICAL EQUIPMENT.

LOAD MAP NOTES AND DESIGNATIONS



ROOF OVERHANG PLAN

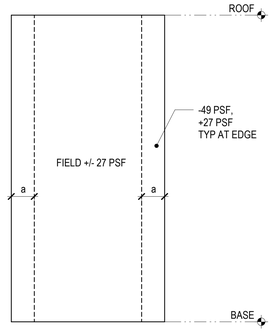


ROOF PLAN

NOTES:

1. SEE NOTES IN "COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM" DETAIL.
2. ROOF OVERHANG PLAN APPLIES TO PORTIONS OF ROOFS PROJECT HORIZONTALLY BEYOND THE WALL BELOW.

16 ROOF WIND UPLIFT PRESSURE DIAGRAM

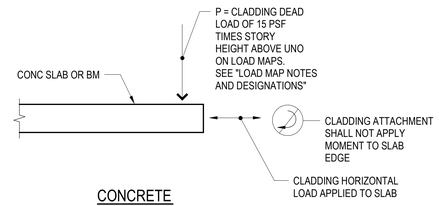


BUILDING ELEVATION

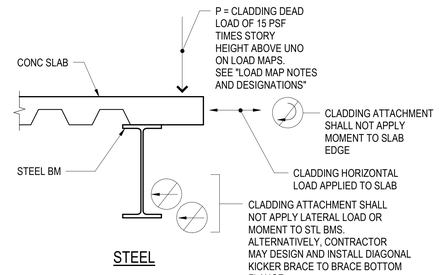
NOTES:

1. WIND LOADS FOR COMPONENTS AND CLADDING ARE DETERMINED IN ACCORDANCE WITH IBC 2018 SECTION 1609 / ASCE 7-16 SECTION 30, AND ARE STRENGTH LEVEL (Vult) PRESSURES. SCALING TO NOMINAL (Vasd) PRESSURES MAY BE ACCOMPLISHED BY MULTIPLYING THE INDICATED VALUES BY 0.60.
2. EXTERIOR COMPONENTS AND CLADDING SHALL BE DESIGNED TO ACCOMMODATE WORST CASE WIND LOADS SHOWN. ALTERNATIVELY, WIND LOADS MAY BE DETERMINED DIRECTLY FROM THE PROVISIONS OF IBC 2018 SECTION 1609 / ASCE 7-16 USING THE WIND LOAD CRITERIA NOTED IN THE "GENERAL NOTES."
3. METHOD OF APPLICATION AND MODIFICATION FACTORS APPLICABLE FOR CORNERS, OVERHANGS, ETC SHALL BE DETERMINED PER ASCE 7-16 BY THE CLADDING DESIGNER. REFER TO "GENERAL NOTES" FOR ADDITIONAL INFORMATION AFFECTING CLADDING DESIGN, AND CONNECTION TO THE STRUCTURE.
4. INWARD (POSITIVE) PRESSURE ACTS TOWARDS THE BUILDING SURFACE AND OUTWARD (NEGATIVE) PRESSURE ACTS AS SUCTION ON THE BUILDING SURFACE.
5. PRESSURES ARE CALCULATED USING THE MINIMUM EFFECTIVE WIND AREA (10 SQUARE FEET).
6. EDGE PRESSURES SHALL BE USED FOR A DISTANCE "a" FROM THE BUILDING'S CORNERS, WHERE "a" IS 10% OF THE LEAST HORIZONTAL DIMENSION, BUT NOT LESS THAN 3 FEET. "a" IS USED FOR OUTWARD PRESSURES ONLY.
7. NET PRESSURE TO ALL PARAPETS IS 104 PSF.

18 COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM



CONCRETE



STEEL

NOTES:

1. REFER TO GENERAL NOTES, "EXTERIOR CLADDING" FOR ADDITIONAL INFORMATION.
2. STRUCTURE IS DESIGNED FOR THE EQUIVALENT UNIFORM LOAD CORRESPONDING TO THE ANTICIPATED WEIGHT OF THE CLADDING SYSTEM. CLADDING ATTACHMENTS WILL APPLY CONCENTRATED LOADS TO THE STRUCTURE. CONTRACTOR SHALL SUBMIT TYPICAL CLADDING ATTACHMENT DETAILS FOR REVIEW AND COMMENT PRIOR TO PREPARATION OF DETAILED CLADDING SUBMITTAL.

20 CLADDING LOAD NOTES

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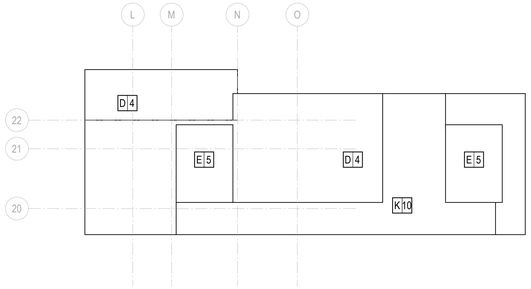
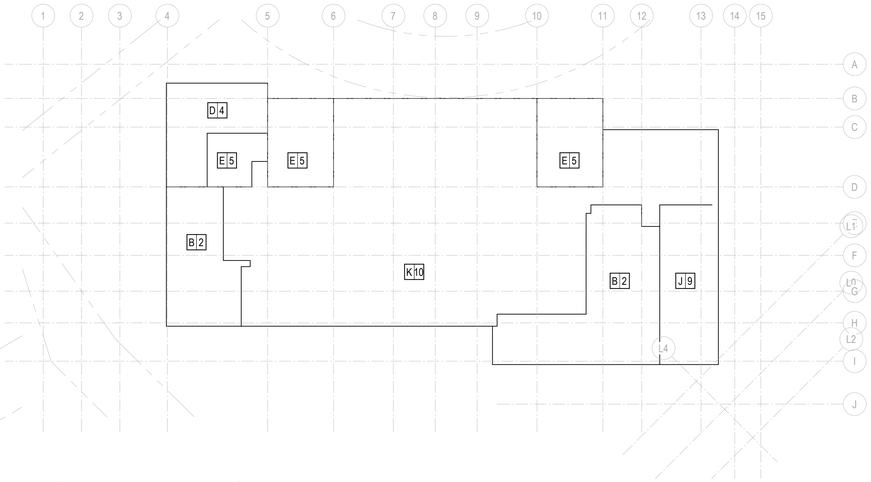
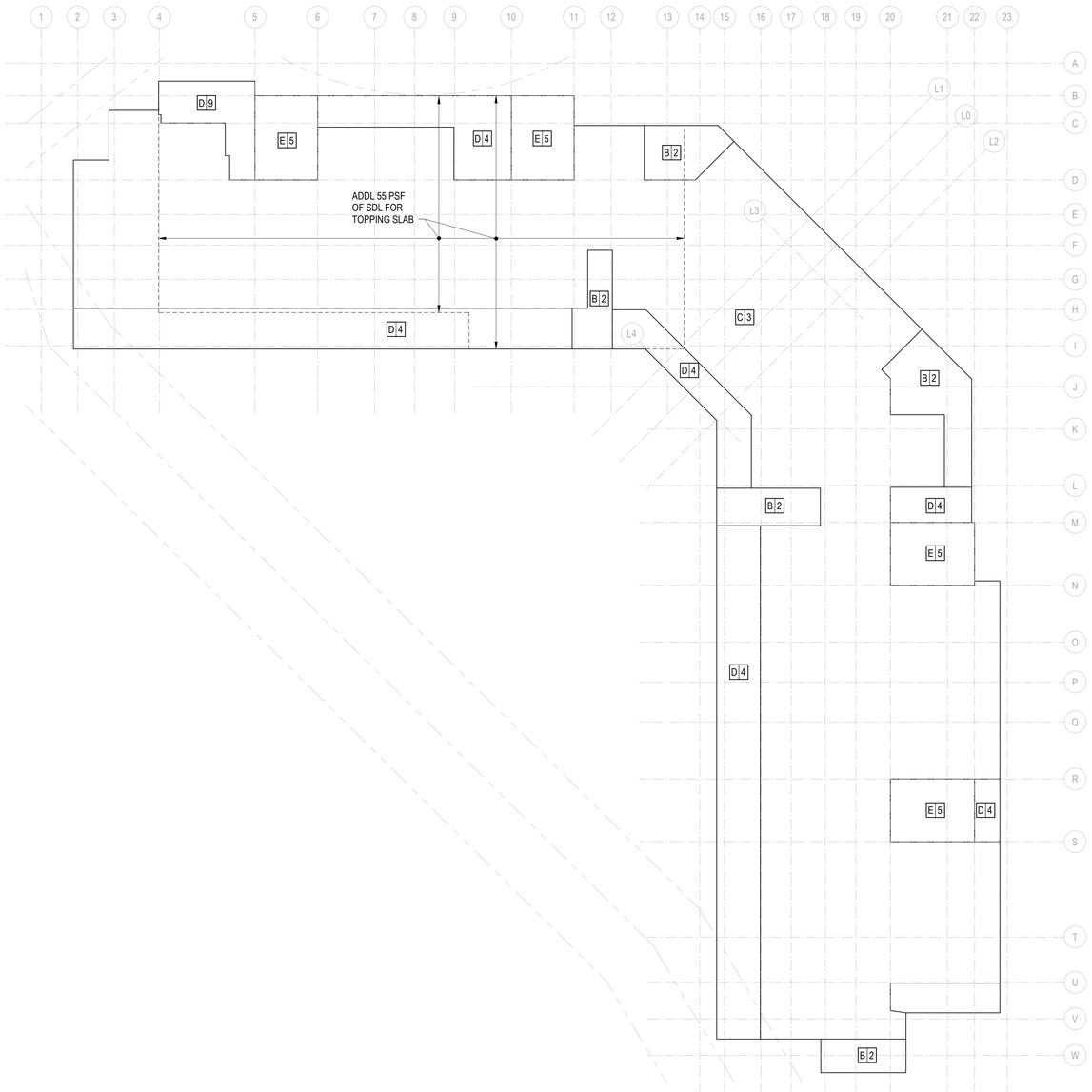




2 TOWER B - LEVEL B1 LOAD MAP

3 TOWER A - LEVEL B1 LOAD MAP



4 TOWER A & B - PARKING LEVEL 2 LOAD MAP

LOAD MAP KEY:

A 1
- NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK
- LETTER INDICATES LIVE LOAD MARK

◇ INDICATES CLADDING LOAD IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

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F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
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3. SEE FRAMING PLANS FOR DESIGN LOAD OF SPECIFIC ITEMS SUCH AS ELEVATORS, ESCALATORS, AND MECHANICAL / ELECTRICAL EQUIPMENT.

LOAD MAP NOTES AND DESIGNATIONS

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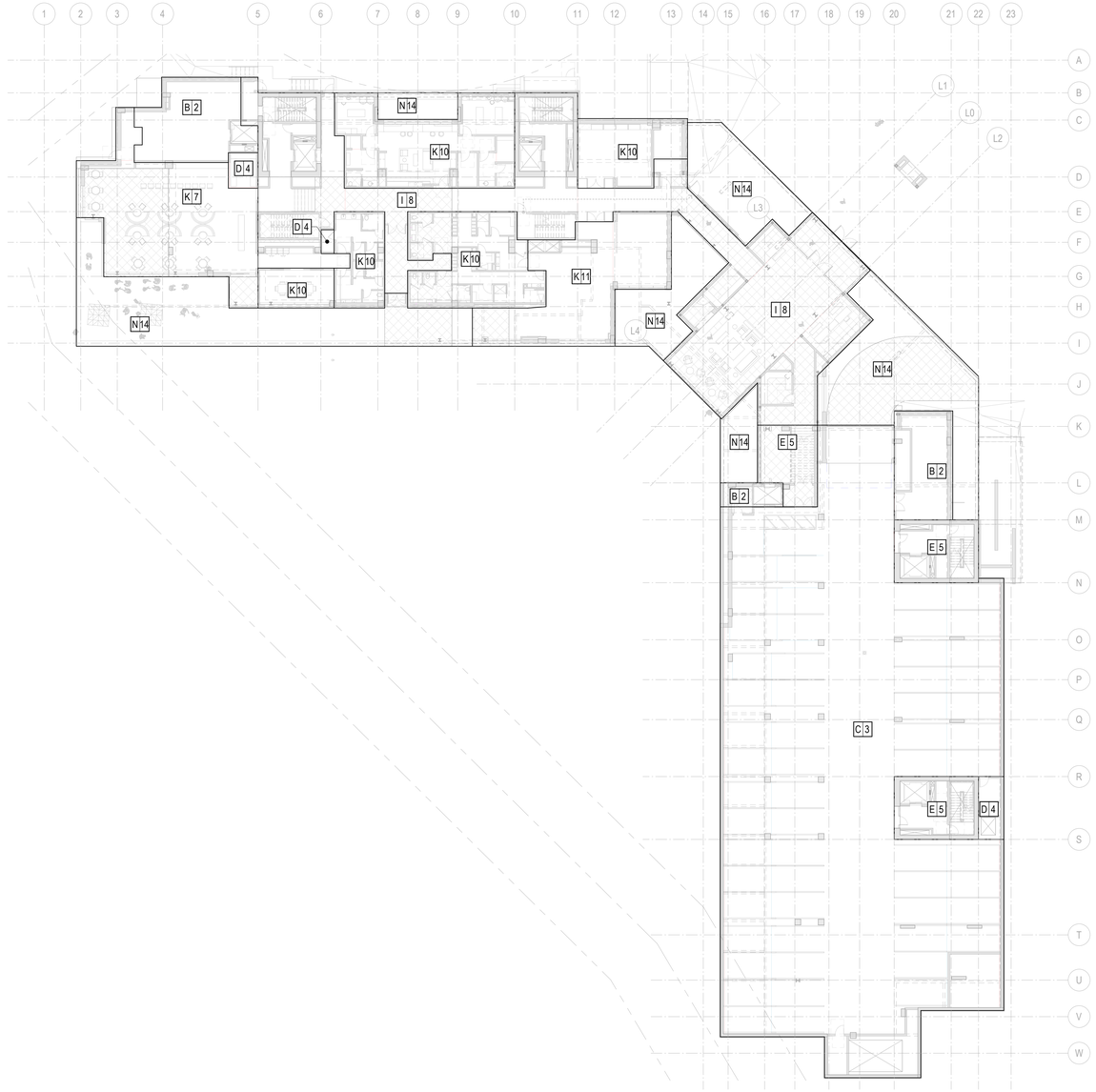

1 TOWER A - LEVEL 1 & TOWER B - LEVEL P1 LOAD MAP

LOAD MAP KEY:

A|1
- NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK
- LETTER INDICATES LIVE LOAD MARK

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F	BALCONY	60(R)
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K	AMENITY	100
N	GROUND FLOOR TERRACE	100

SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
1	RESIDENTIAL	30	5	5	20		
2	MECHANICAL / ELECTRICAL	15	10		5		
3	PARKING	5	5				
4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
11	FITNESS	65	5	5	5	50	ISOLATION SLAB
12	GREEN ROOF	40	10	5		25	INSULATION + LIGHT GREEN ROOF
13	ROOF	25	10			15	INSULATION + ROOFING
14	DEEP SOIL	330	10			320	SOIL DEPTH TBC

LOAD MAP NOTES:

1. LIVE LOADS MARKED (R) ARE REDUCIBLE IN ACCORDANCE WITH THE BUILDING CODE.
2. SUPERIMPOSED DEAD LOADS ARE IN ADDITION TO THE SELF-WEIGHT OF THE STRUCTURE. BUILT-UP SLABS SHOWN ON PLAN ARE CONSIDERED TO BE PART OF THE SELF-WEIGHT OF THE STRUCTURE.
3. SEE FRAMING PLANS FOR DESIGN LOAD OF SPECIFIC ITEMS SUCH AS ELEVATORS, ESCALATORS, AND MECHANICAL / ELECTRICAL EQUIPMENT.

LOAD MAP NOTES AND DESIGNATIONS

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LOAD MAPS

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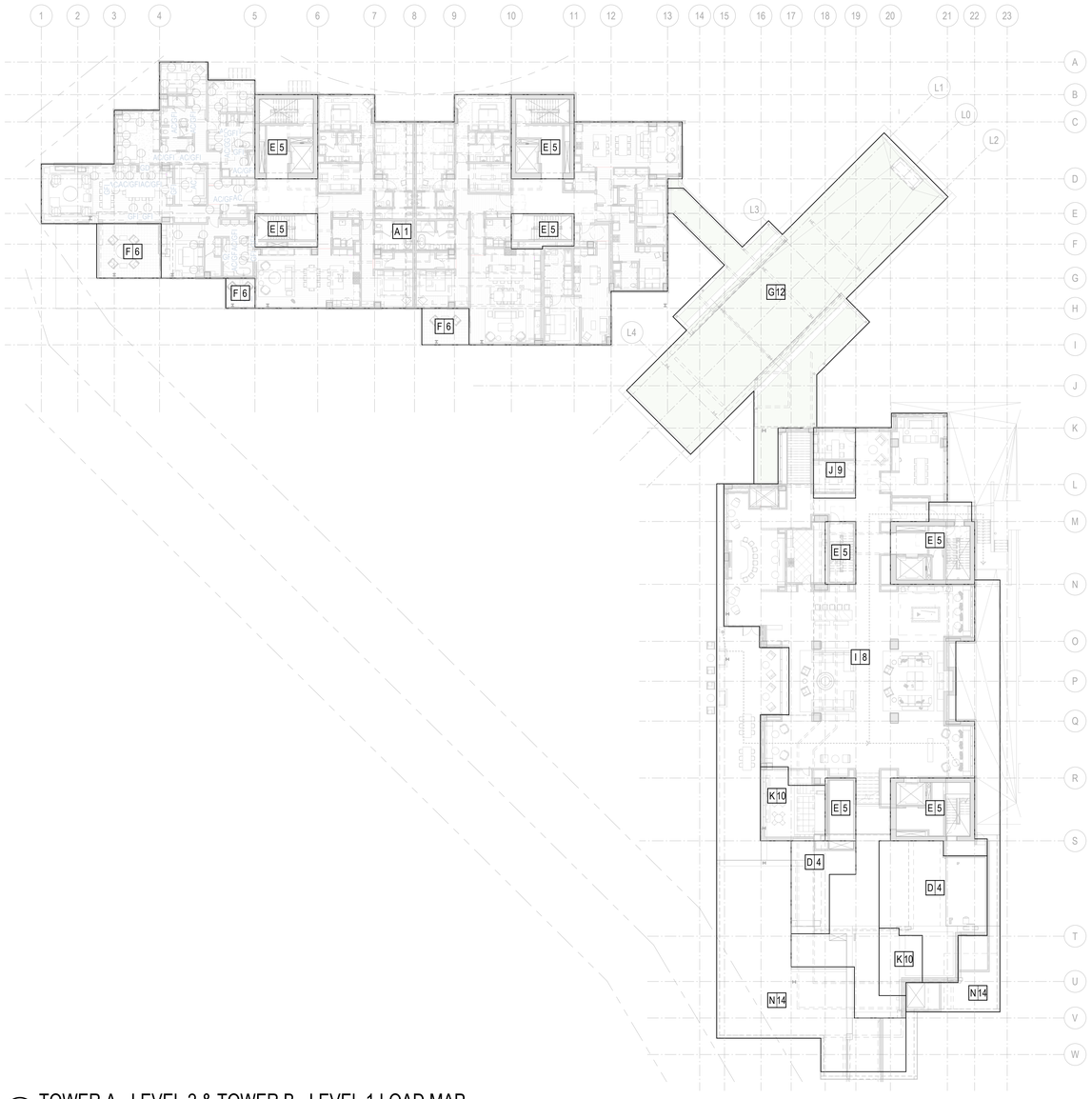
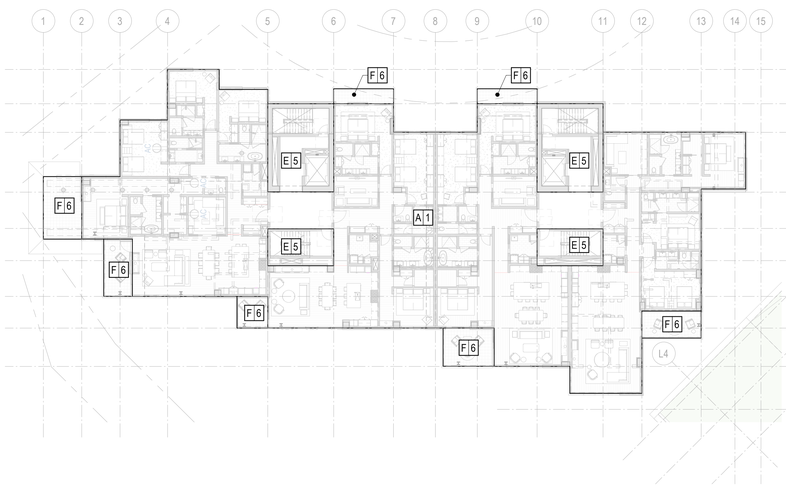


1 TOWER A - LEVEL 2 & TOWER B - LEVEL 1 LOAD MAP



2 TOWER A - LEVEL 3 LOAD MAP

3 TOWER B - LEVEL 2 LOAD MAP

LOAD MAP KEY:

A 1

NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK

LETTER INDICATES LIVE LOAD MARK

INDICATES CLADDING LOADING IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

MARK	LIVE LOAD DESIGNATIONS USE	LIVE LOAD (PSF)
A	RESIDENTIAL	40(R)
B	MECHANICAL / ELECTRICAL	125
C	PARKING	40(R) (20%)
D	LIGHT STORAGE	125
E	ASSEMBLY / CORRIDORS	100
F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
N	GROUND FLOOR TERRACE	100

SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
1	RESIDENTIAL	30	5	5	20		
2	MECHANICAL / ELECTRICAL	15	10		5		
3	PARKING	5	5				
4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
11	FITNESS	65	5	5	5	50	ISOLATION SLAB
12	GREEN ROOF	40	10	5		25	INSULATION + LIGHT GREEN ROOF
13	ROOF	25	10			15	INSULATION + ROOFING
14	DEEP SOIL	330	10			320	SOIL DEPTH TBC

LOAD MAP NOTES:

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3. SEE FRAMING PLANS FOR DESIGN LOAD OF SPECIFIC ITEMS SUCH AS ELEVATORS, ESCALATORS, AND MECHANICAL / ELECTRICAL EQUIPMENT.

LOAD MAP NOTES AND DESIGNATIONS

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LOAD MAPS

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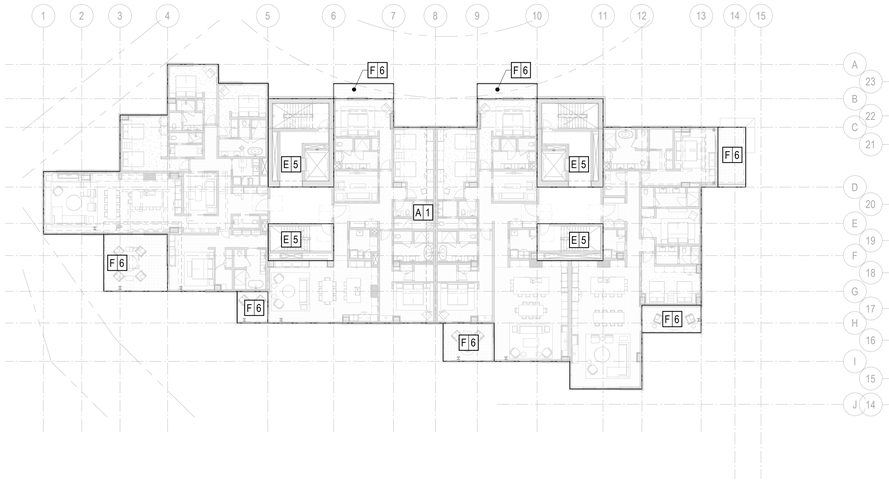
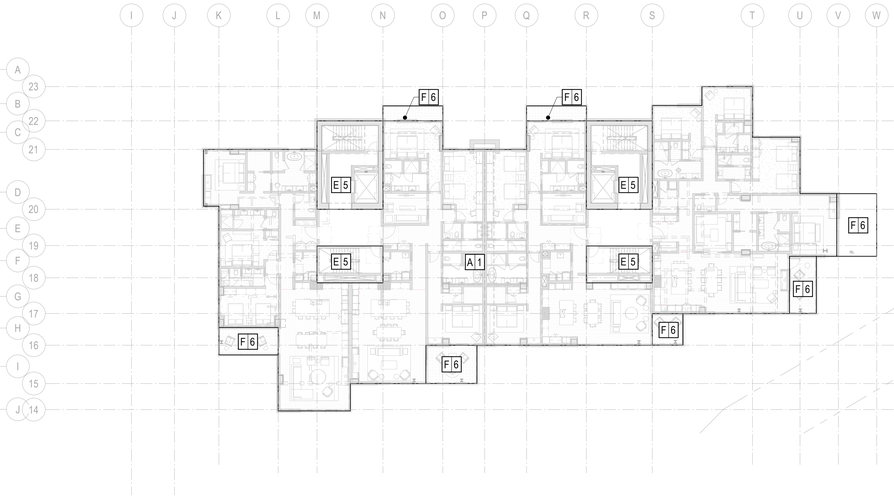


1 TOWER A - LEVEL 4 LOAD MAP



2 TOWER B - LEVEL 3 LOAD MAP

LOAD MAP KEY:

A 1
- NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK
- LETTER INDICATES LIVE LOAD MARK

◇ INDICATES CLADDING LOAD IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

LIVE LOAD (LL) DESIGNATIONS

MARK	USE	LIVE LOAD (PSF)
A	RESIDENTIAL	40(R)
B	MECHANICAL / ELECTRICAL	125
C	PARKING	40(R) (20%)
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E	ASSEMBLY / CORRIDORS	100
F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
N	GROUND FLOOR TERRACE	100

SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
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4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
11	FITNESS	65	5	5	5	50	ISOLATION SLAB
12	GREEN ROOF	40	10	5		25	INSULATION + LIGHT GREEN ROOF
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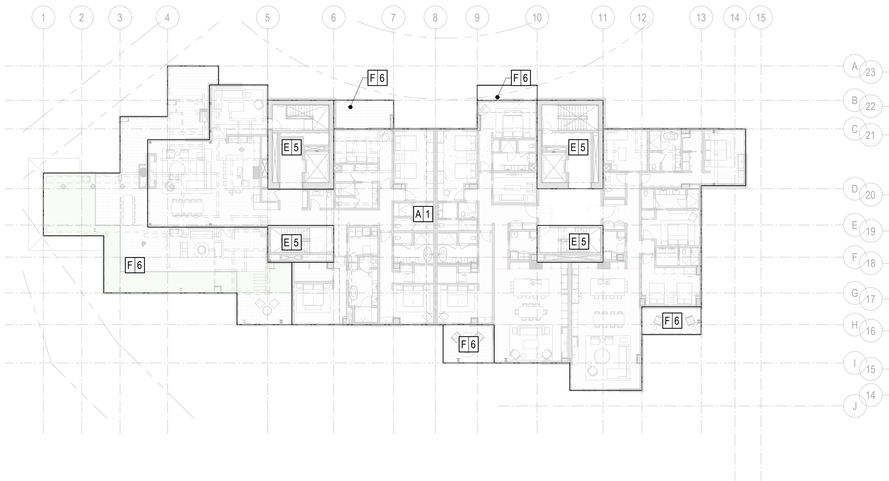
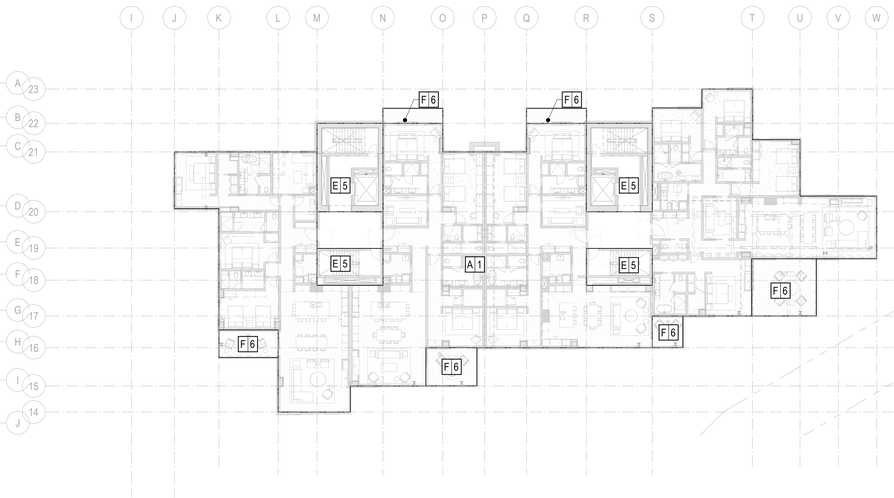
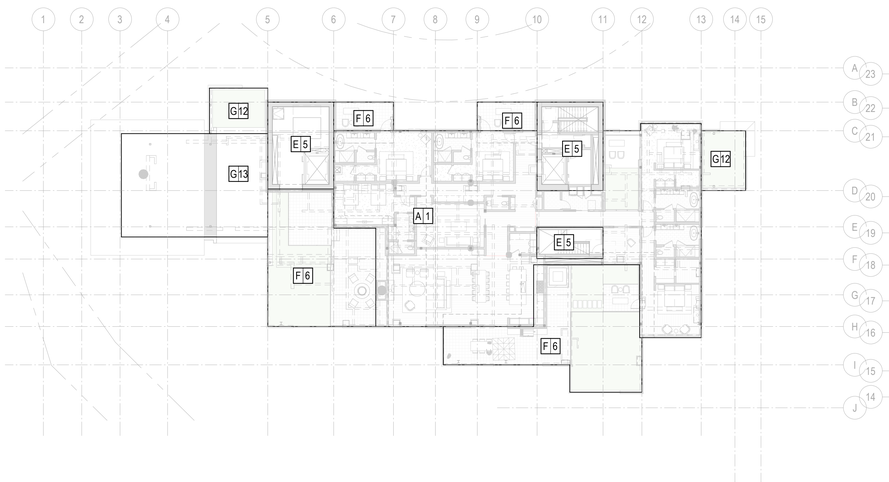
LOAD MAP NOTES AND DESIGNATIONS



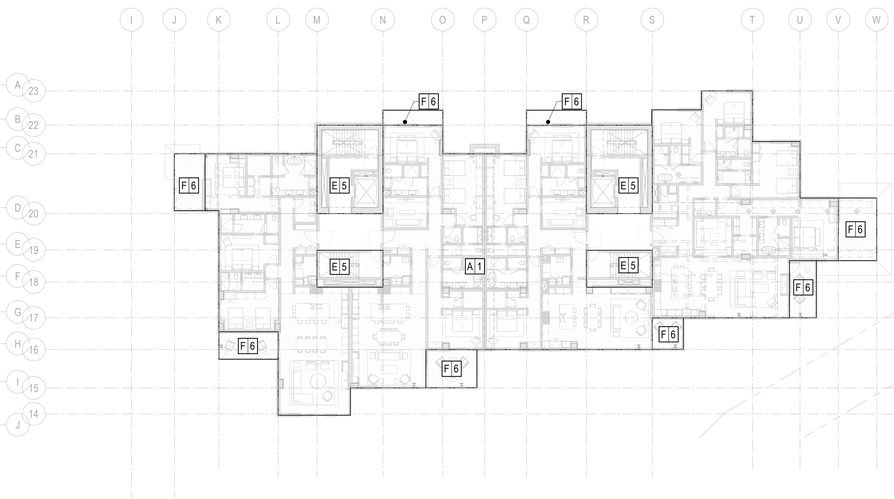
3 TOWER A - LEVEL 5 LOAD MAP



4 TOWER B - LEVEL 4 LOAD MAP



5 TOWER A - LEVEL 6 LOAD MAP



6 TOWER B - LEVEL 5 LOAD MAP

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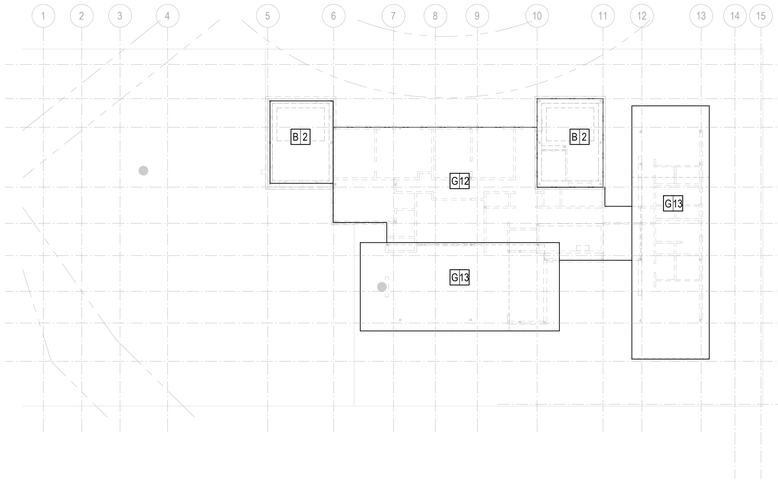
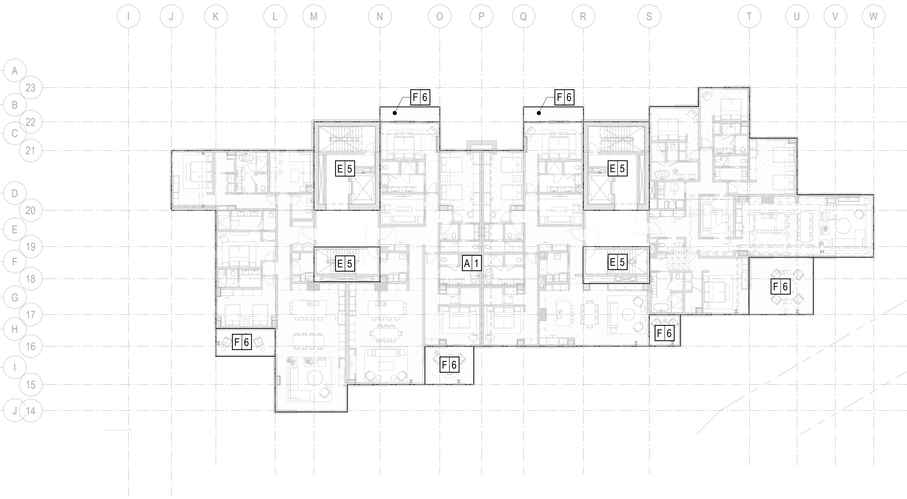


1 TOWER A - ROOF LOAD MAP



2 TOWER B - LEVEL 6 LOAD MAP

LOAD MAP KEY:

A 1
- NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK
- LETTER INDICATES LIVE LOAD MARK

◇ INDICATES CLADDING LOAD IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

LIVE LOAD (LL) DESIGNATIONS

MARK	USE	LIVE LOAD (PSF)
A	RESIDENTIAL	40(R)
B	MECHANICAL / ELECTRICAL	125
C	PARKING	40(R) (20%)
D	LIGHT STORAGE	125
E	ASSEMBLY / CORRIDORS	100
F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
N	GROUND FLOOR TERRACE	100

SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
1	RESIDENTIAL	30	5	5	20		
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3	PARKING	5	5				
4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
11	FITNESS	65	5	5	5	50	ISOLATION SLAB
12	GREEN ROOF	40	10	5		25	INSULATION + LIGHT GREEN ROOF
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14	DEEP SOIL	330	10			320	SOIL DEPTH TBC

LOAD MAP NOTES:

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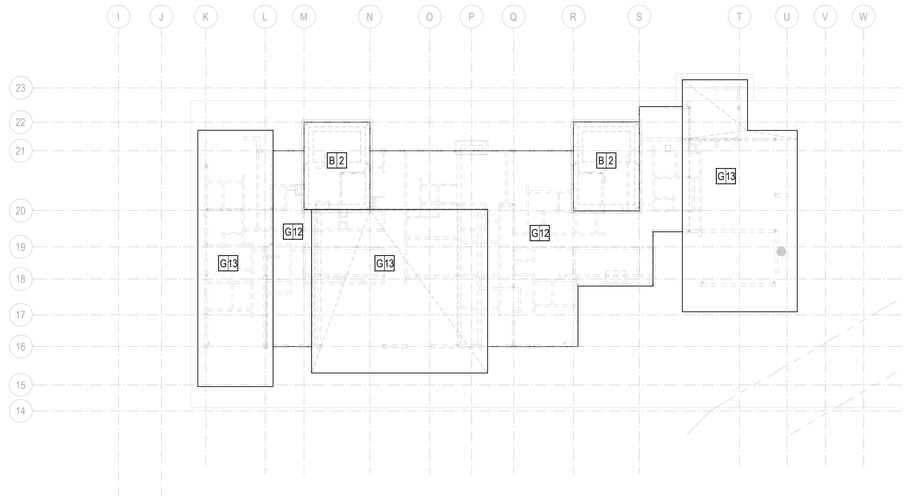
LOAD MAP NOTES AND DESIGNATIONS

4 TOWER B - LEVEL 7 LOAD MAP



6 TOWER B - ROOF LOAD MAP

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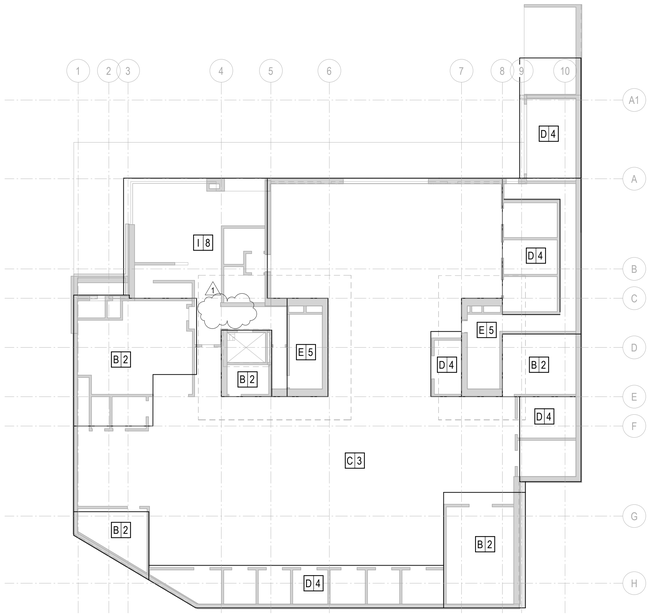
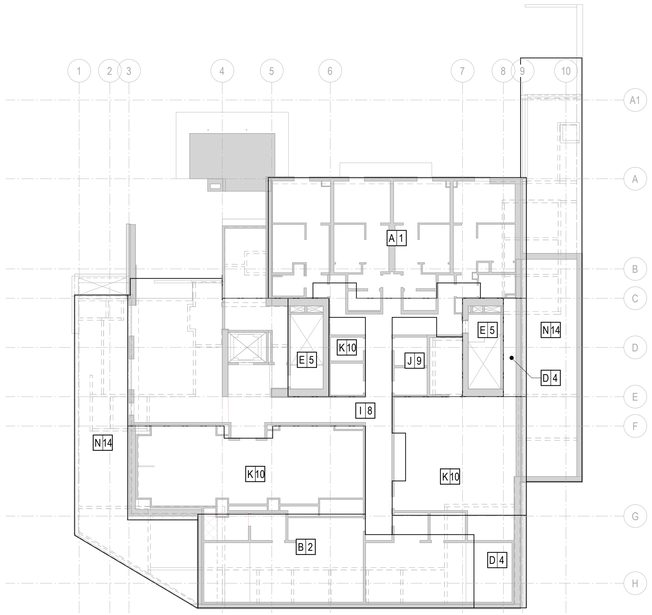
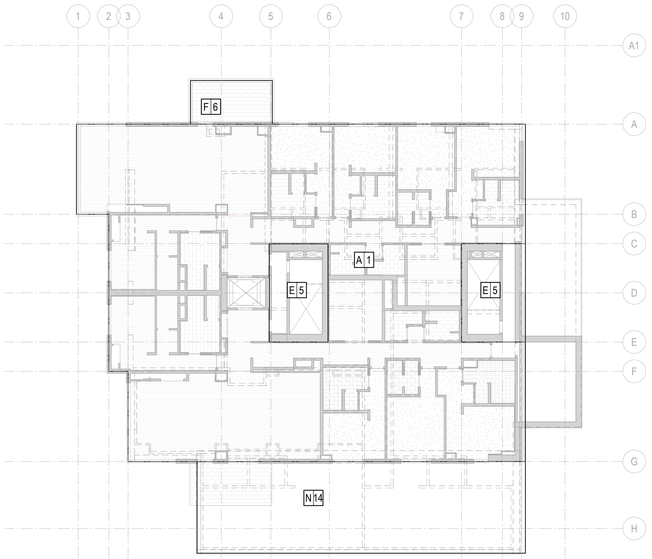


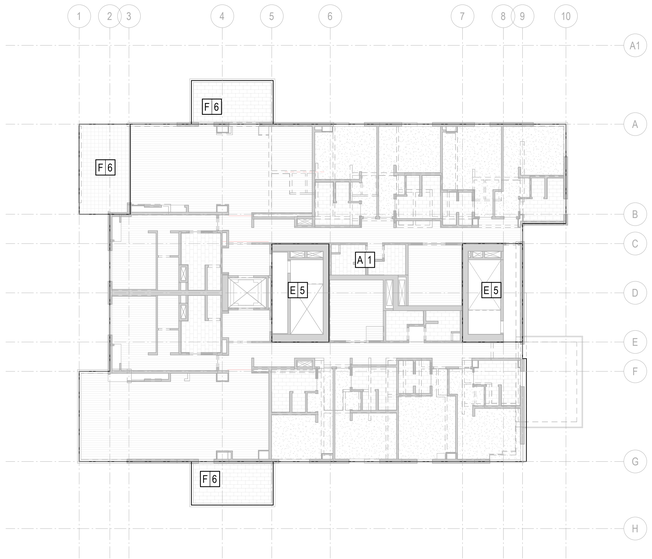
1 TOWER C PARKING LOAD MAP



2 TOWER C LEVEL 1 LOAD MAP



3 TOWER C LEVEL 2 LOAD MAP



4 TOWER C LEVEL 3 LOAD MAP

LOAD MAP KEY:

A|1
- NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK
- LETTER INDICATES LIVE LOAD MARK

◇ INDICATES CLADDING LOAD IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

LIVE LOAD (LL) DESIGNATIONS

MARK	USE	LIVE LOAD (PSF)
A	RESIDENTIAL	40(R)
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F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
N	GROUND FLOOR TERRACE	100

SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
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8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
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LOAD MAP NOTES AND DESIGNATIONS

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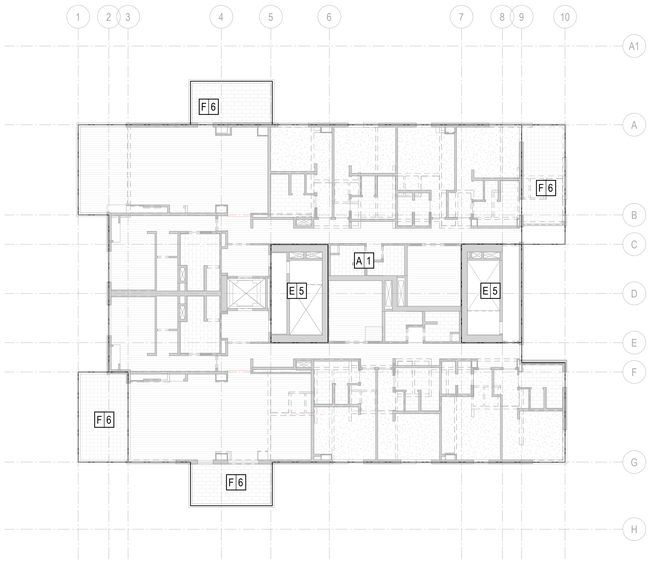
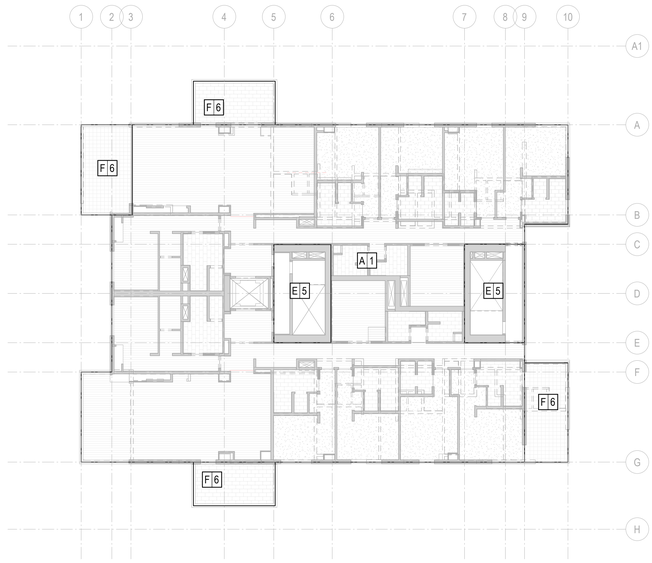
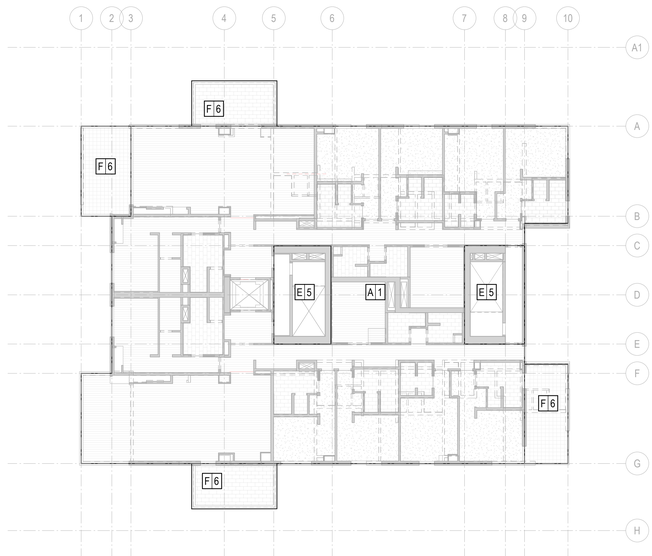


1 TOWER C LEVEL 4 LOAD MAP



2 TOWER C LEVEL 5 LOAD MAP

3 TOWER C LEVEL 6 LOAD MAP



4 TOWER C LEVEL 7 LOAD MAP

LOAD MAP KEY:

A 1
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F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
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SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

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4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
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LOAD MAP NOTES AND DESIGNATIONS

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LOAD MAPS

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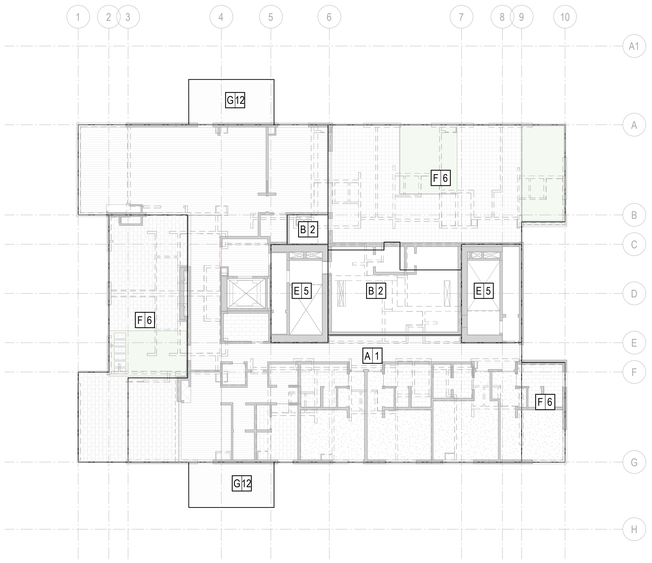
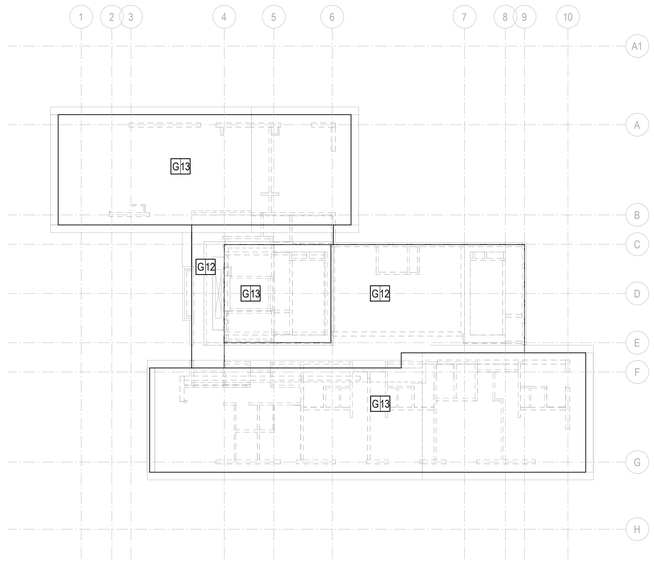


1 TOWER C LEVEL 8 LOAD MAP



2 TOWER C ROOF LOAD MAP

LOAD MAP KEY:

A 1
- NUMBER INDICATES SUPERIMPOSED DEAD LOAD MARK
- LETTER INDICATES LIVE LOAD MARK

◇ INDICATES CLADDING LOAD IN POUNDS PER SQUARE FOOT OF SURFACE AREA. SEE "CLADDING LOAD NOTES" DETAIL AT THE END OF LOAD MAPS.

LIVE LOAD (LL) DESIGNATIONS

MARK	USE	LIVE LOAD (PSF)
A	RESIDENTIAL	40(R)
B	MECHANICAL / ELECTRICAL	125
C	PARKING	40(R) (20%)
D	LIGHT STORAGE	125
E	ASSEMBLY / CORRIDORS	100
F	BALCONY	60(R)
G	ROOF	20(R)
I	GROUND LOBBY	100
J	OFFICE	60 + 15 PARTITION LOAD
K	AMENITY	100
N	GROUND FLOOR TERRACE	100

SUPERIMPOSED DEAD LOAD (SDL) DESIGNATIONS

MARK	TYPE	TOTAL SDL (PSF)	CEILING/MEP LOAD (PSF)	FLOOR FINISH LOAD (PSF)	PARTITION LOAD (PSF)	SPECIAL LOAD (PSF)	SPECIAL LOAD DESCRIPTION
1	RESIDENTIAL	30	5	5	20		
2	MECHANICAL / ELECTRICAL	15	10		5		
3	PARKING	5	5				
4	LIGHT STORAGE	15	10		5		
5	CORRIDORS	15	10		5		
6	BALCONY	75	10	15		50	INSULATION + TOPPING SLAB
7	RETAIL	60	10			50	BUILT UP SLAB
8	GROUND LOBBY	60	10	40	10		
9	OFFICE	15	10	5			
10	AMENITY	30	10	15	5		
11	FITNESS	65	5	5	5	50	ISOLATION SLAB
12	GREEN ROOF	40	10	5		25	INSULATION + LIGHT GREEN ROOF
13	ROOF	25	10			15	INSULATION + ROOFING
14	DEEP SOIL	330	10			320	SOIL DEPTH TBC

LOAD MAP NOTES:

1. LIVE LOADS MARKED (R) ARE REDUCIBLE IN ACCORDANCE WITH THE BUILDING CODE.
2. SUPERIMPOSED DEAD LOADS ARE IN ADDITION TO THE SELF-WEIGHT OF THE STRUCTURE.
3. SEE FRAMING PLANS FOR DESIGN LOAD OF SPECIFIC ITEMS SUCH AS ELEVATORS, ESCALATORS, AND MECHANICAL / ELECTRICAL EQUIPMENT.

LOAD MAP NOTES AND DESIGNATIONS

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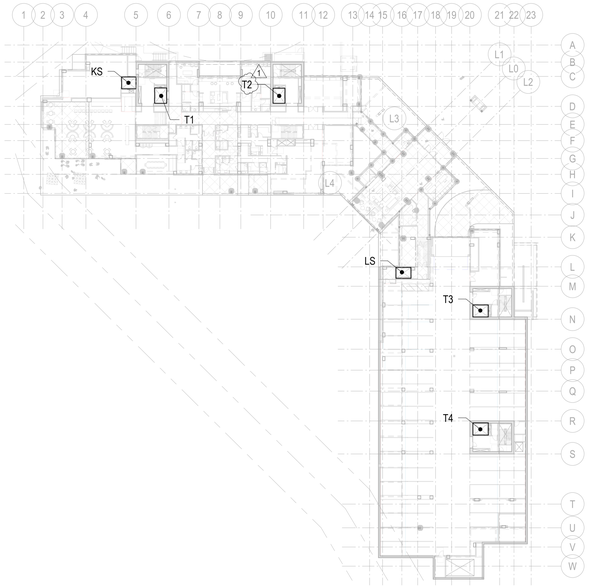
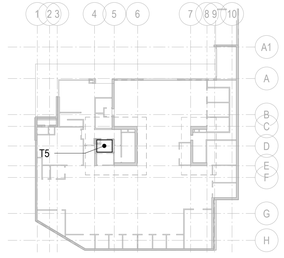
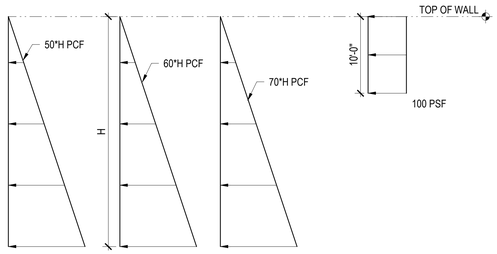
LOAD MAPS

S1.08







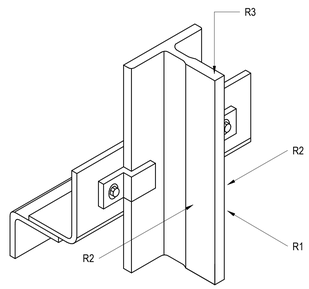
SOIL LOADS

LIVE LOADS

NOTES:

1. H = HEIGHT OF BASEMENT WALL, (FT)
2. DESIGN PRESSURES ARE BASED ON A "DRAINED" CONDITION PER THE GENERAL NOTES.
3. THE PRESSURE DIAGRAM IS APPROPRIATE FOR PERMANENT BASEMENT WALLS CONSTRUCTED IN FRONT OF TEMPORARY SHORING WALLS WITH TIEBACK OR SOIL NAIL ANCHORS. IF ADDITIONAL SURCHARGE LOADING (SUCH AS FROM SOIL STOCKPILES, EXCAVATORS, DUMP TRUCKS, CRANES, OR CONCRETE TRUCKS) IS ANTICIPATED, THE STRUCTURAL AND GEOTECHNICAL ENGINEERS SHALL BE NOTIFIED.

TOWER C - ELEVATOR KEY PLAN



ELEVATOR RAIL NORMAL REACTIONS		LS	KS	T1-4	T5
CAR	R1 (KIPS)	0.7	1.4	1.3	1.3
	R2 (KIPS)	0.4	1.9	0.7	0.7
	R3 (KIPS)	SEE ELEVATOR PIT PLANS			
CAR	R1 (KIPS)	0.8	1.1	1.1	1.1
	R2 (KIPS)	0.4	0.6	0.6	0.6
	R3 (KIPS)	SEE ELEVATOR PIT PLANS			

ELEVATOR RAIL NORMAL REACTIONS		LS	KS	T1-4	T5
CAR	R1 (KIPS)	0.7	1.1	1.1	1.1
	R2 (KIPS)	0.4	0.5	0.5	0.5
	R3 (KIPS)	SEE ELEVATOR PIT PLANS			
CAR	R1 (KIPS)	0.8	1.1	1.1	1.1
	R2 (KIPS)	0.4	0.6	0.6	0.6
	R3 (KIPS)	SEE ELEVATOR PIT PLANS			

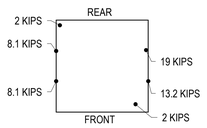
NOTES:

1.NOTES:
1. ELEVATOR RAIL SUPPORT BEAMS AND POST HAVE BEEN DESIGNED TO MEET DEFLECTION LIMITS OF 1/4" FOR NORMAL LOADS AND 1/4" FOR SEISMIC LOADS.

2. MAXIMUM ASSUMED UNSUPPORTED ELEVATOR RAIL SPAN IS 10'-6" FOR ELEVATORS "KS" and T1 THROUGH T5. MAXIMUM ASSUMED UNSUPPORTED ELEVATOR RAIL SPAN IS 14'-0" FOR ELEVATOR "LS". PROVIDE ELEVATOR RAIL SUPPORT POST PER THE TYPICAL DETAIL WHERE REQUIRED BY THE ELEVATOR SUPPLIER. COORDINATE QUANTITY AND LOCATION WITH ELEVATOR SUPPLIER.

3. "CWT" = COUNTERWEIGHT.

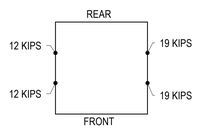
16 ELEVATOR RAIL REACTIONS



OVER HEAD
(ABOVE TOWER B LEVEL 1)

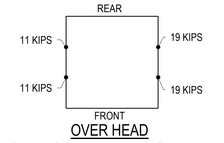
REACTION POINT	REACTION TYPE	REACTION (KIPS)
A	CAR BUFFER	29.1
B	CAR BUFFER	26.4
C	CAR SAFETY	27.4
D	DRIVE MACHINE	23.4
E	CAR RAIL	25
F	CWT RAIL	17

14 ELEVATOR RAIL REACTIONS



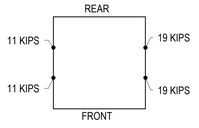
OVER HEAD
(ABOVE TOWER A L1)

REACTION POINT	REACTION TYPE	REACTION (KIPS)
A	CAR BUFFER	52.3
B	CAR BUFFER	47.8
C	CAR SAFETY	31.2
D	DRIVE MACHINE	31
E	CAR RAIL	22
F	CWT RAIL	10.3



OVER HEAD
(T1 - TOWER A6; T2 - TOWER A ROOF; T3 & T4 - TOWER B ROOF)

REACTION POINT	REACTION TYPE	REACTION (KIPS)
A	CAR BUFFER	53.3
B	CAR BUFFER	48.8
C	CAR SAFETY	32.2
D	DRIVE MACHINE	32
E	CAR RAIL	23
F	CWT RAIL	11



OVER HEAD
(TOWER C ROOF)

REACTION POINT	REACTION TYPE	REACTION (KIPS)
A	CAR BUFFER	53.3
B	CAR BUFFER	48.8
C	CAR SAFETY	32.2
D	DRIVE MACHINE	32
E	CAR RAIL	23
F	CWT RAIL	11

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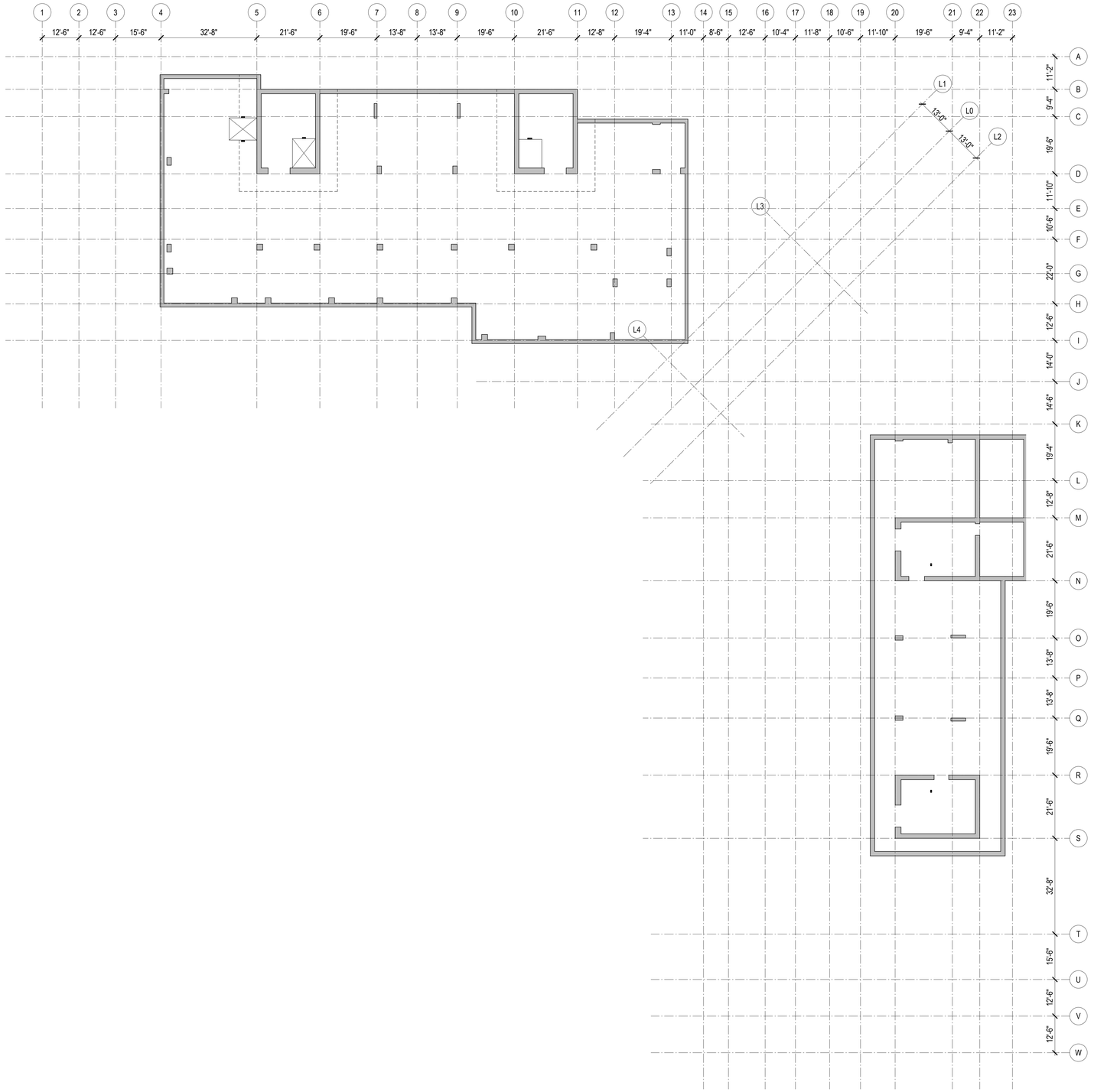
TOWER C FOUNDATION



1 TOWER A & B - B1 COMPOSITE FRAMING PLAN
1/16" = 1'-0"

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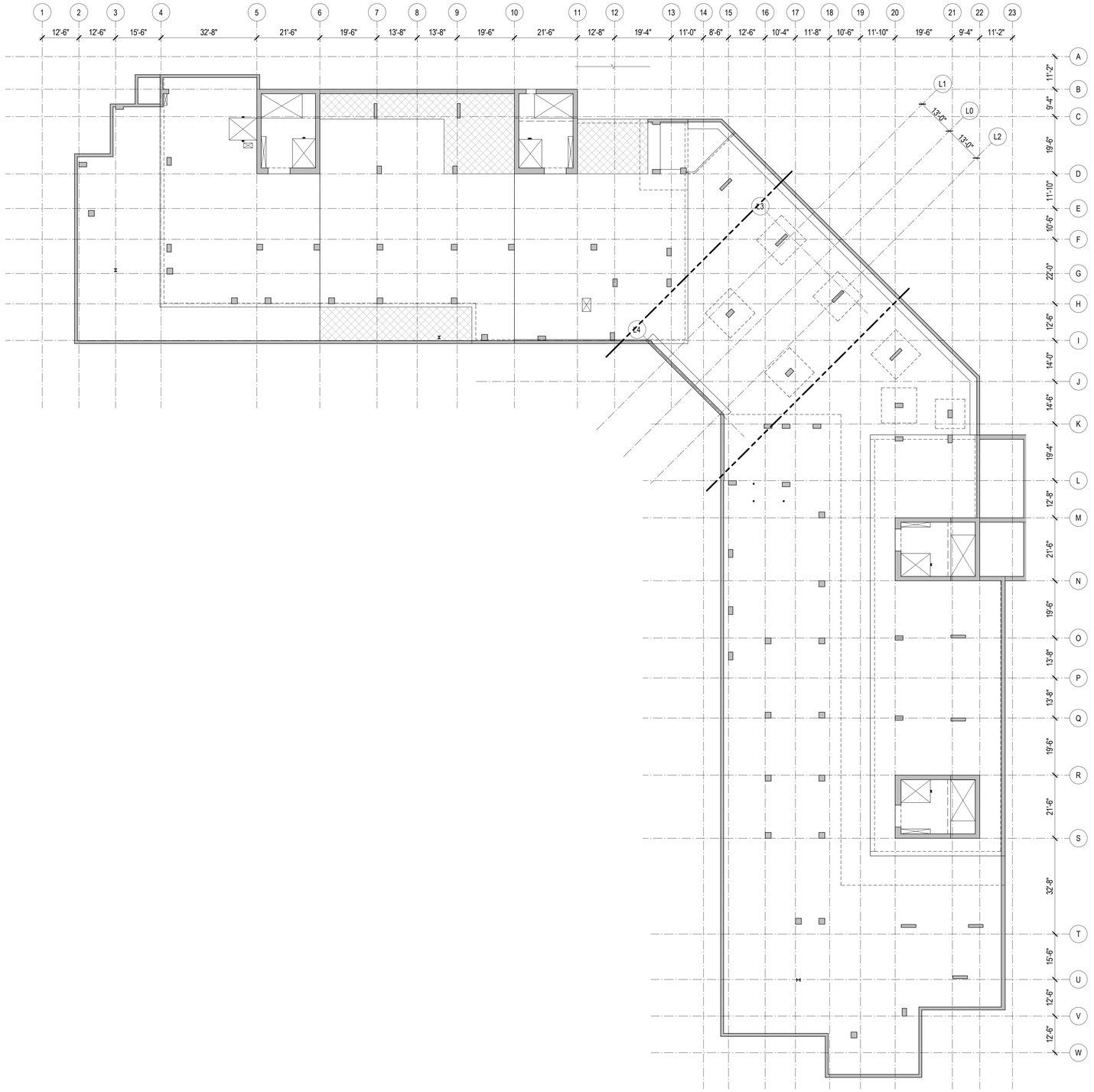
TOWER A & B
LEVEL B1
COMPOSITE
FRAMING PLAN

S2.01



1 TOWER A & B - PARKING LEVEL 2 COMPOSITE FRAMING PLAN
1/16" = 1'-0"

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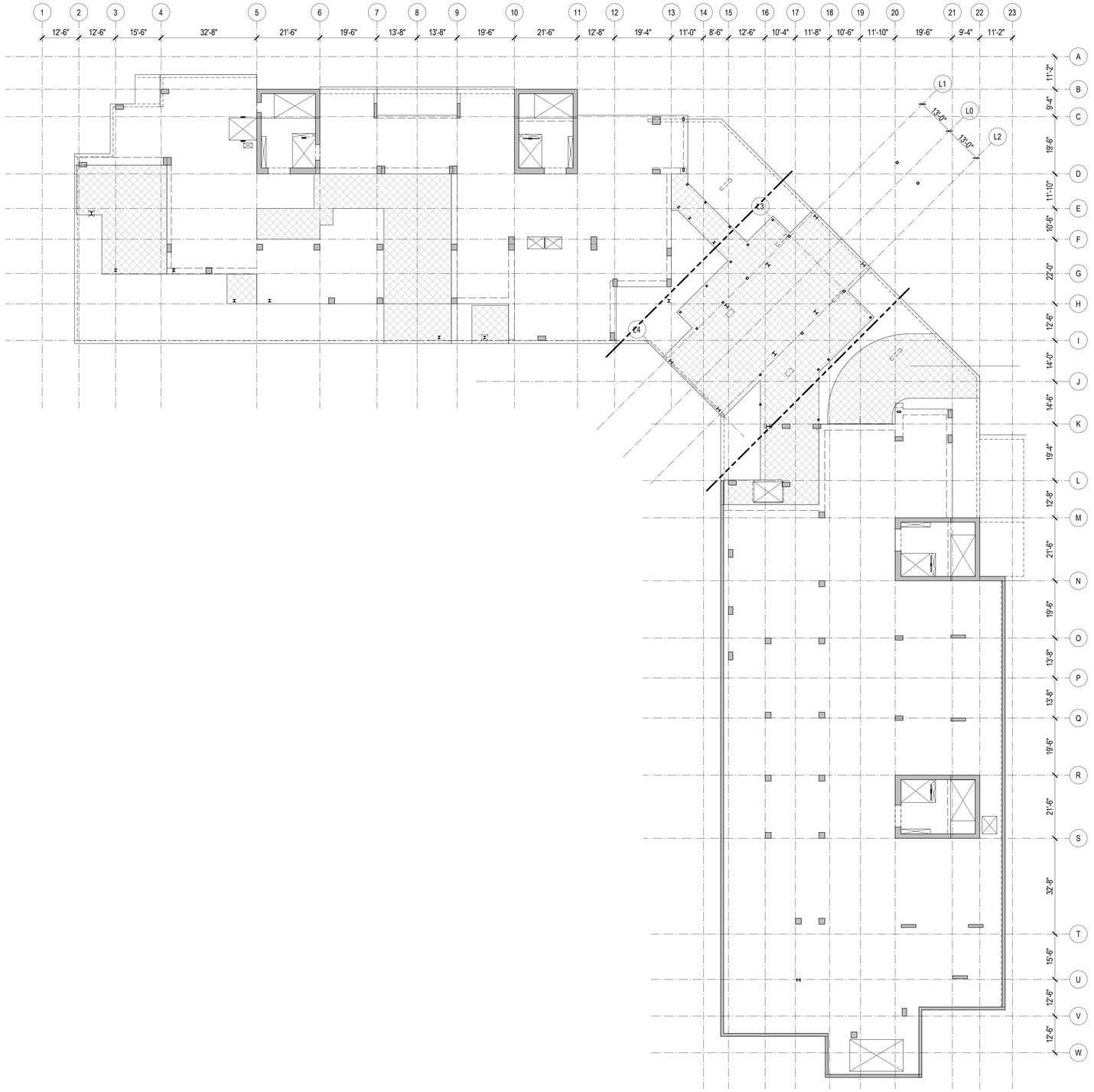
TOWER A & B
LEVEL P2
COMPOSITE
FRAMING PLAN

S2.02



1 TOWER A LEVEL 1 & TOWER B PARKING LEVEL 1 COMPOSITE FRAMING PLAN
1/16" = 1'-0"

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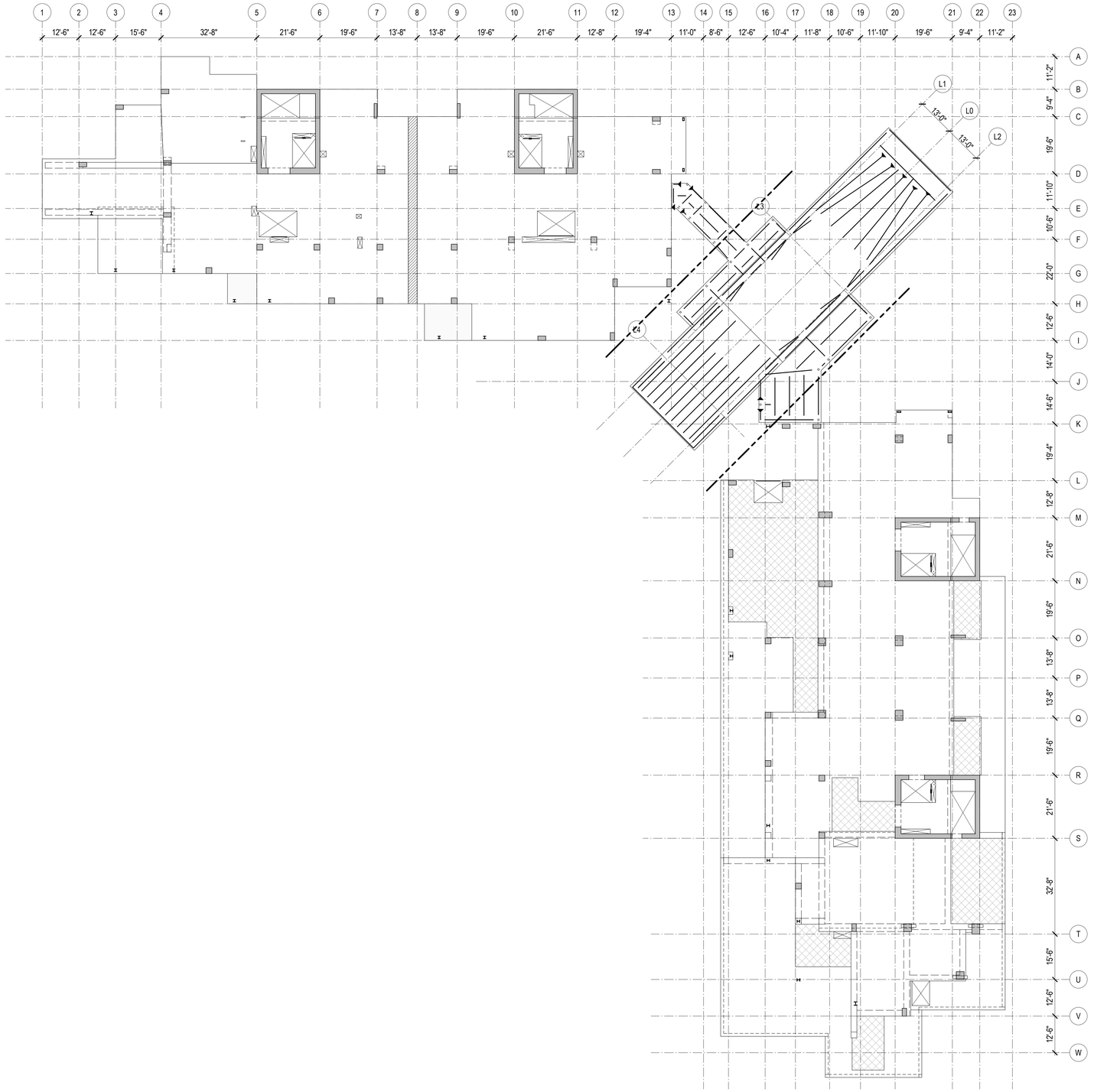
TOWER A LEVEL 1 & TOWER B LEVEL P1 COMPOSITE FRAMING PLAN

S2.11



1 TOWER A LEVEL 2 & TOWER B LEVEL 1 COMPOSITE FRAMING PLAN
1/16" = 1'-0"

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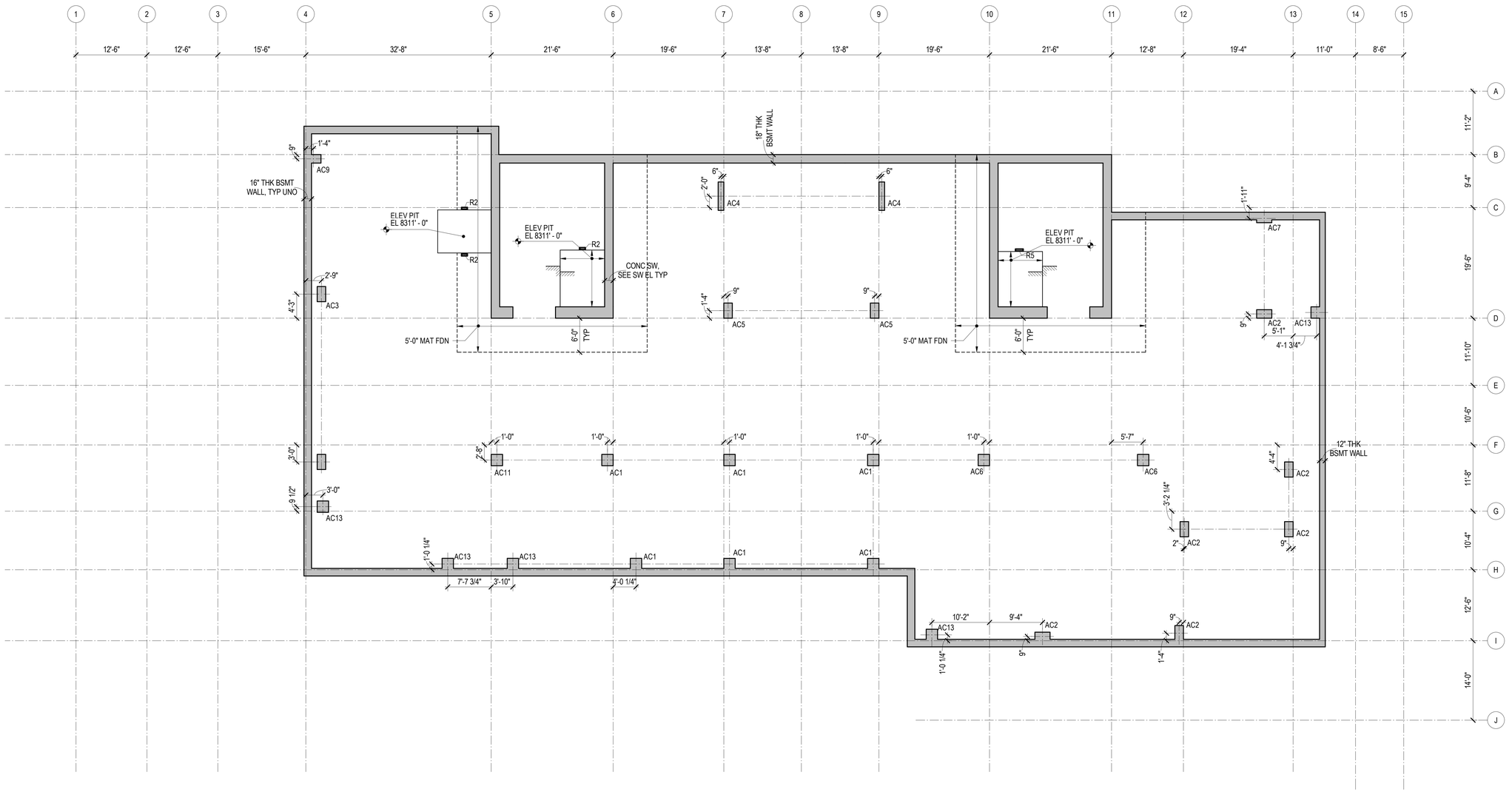
TOWER A LEVEL 2 & TOWER B LEVEL 1 COMPOSITE FRAMING PLAN

S2.12



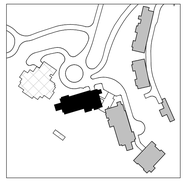
1 TOWER A - LEVEL B1 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE ELEVATION IS 8314' - 7". TOP OF MAT IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL SLAB EDGES, OPENINGS, SLOPES, AND DEPRESSIONS NOT DEFINED ON THE STRUCTURAL PLANS.
2. MAT FOUNDATION IS 3'-0" THICK UNLESS NOTED OTHERWISE. UPON REACHING THE MAT FOUNDATION SUBGRADE ELEVATION, SOIL CONDITIONS SHALL BE EVALUATED AND APPROVED BY THE GEOTECHNICAL ENGINEER OF RECORD.
3. SHEAR WALL OPENINGS, WALL ENDS, AND WALL LOCATIONS ARE DIMENSIONED RELATIVE TO GRID LINES ON THE SHEAR WALL ELEVATION.
4. BASEMENT WALLS SHALL BE RESTRAINED AT EACH FLOOR BY THE STRUCTURAL SLAB AND AT THE BOTTOM OF THE MAT, AND SHALL HAVE REACHED DESIGN STRENGTH PRIOR TO PLACING BACKFILL AND/OR DE-TENSIONING TIE-BACK ANCHORS.
5. BASEMENT WALLS ARE DESIGNED FOR A FULLY DRAINED CONDITION IN THE RETAINED SOIL.
6. BASEMENT WALL REINFORCEMENT IS SHOWN ON THE BASEMENT WALL ELEVATIONS.
7. THE STRUCTURAL SLAB IS A 10-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
8. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
9. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
10. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS PRIOR TO CASTING FOUNDATIONS.
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. PROVIDE DOCK LEVELER, ELEVATOR PITS, ESCALATOR PITS, SUMP PITS, M/E/P VAULTS, TRENCH AND AREA DRAINS, AND CONCRETE ENCASEMENTS/EMBEDMENTS/INSERTS/ETC. AS REQUIRED. REINFORCE PER TYPICAL DETAILS.
12. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR SIDEWALKS, PAVING, AND SITE DETAILS AT BUILDING EXTERIOR UNLESS NOTED OTHERWISE.
13. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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TOWER A LEVEL B1 FRAMING PLAN

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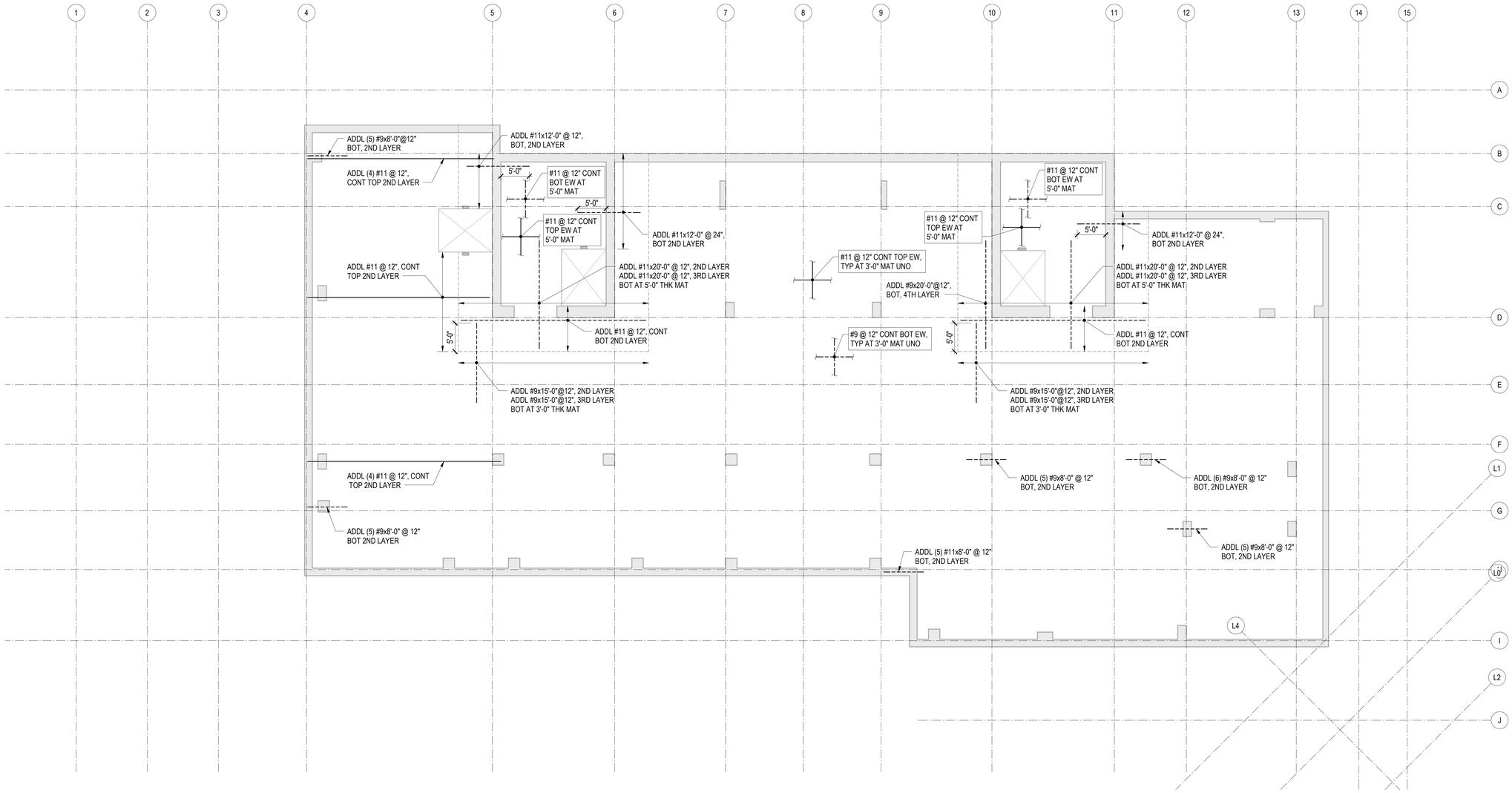
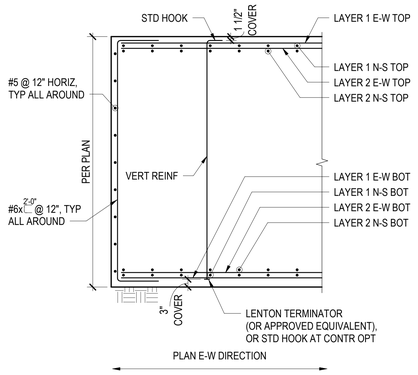


1 TOWER A - LEVEL B1 - LONGITUDINAL REINFORCEMENT PLAN

1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.



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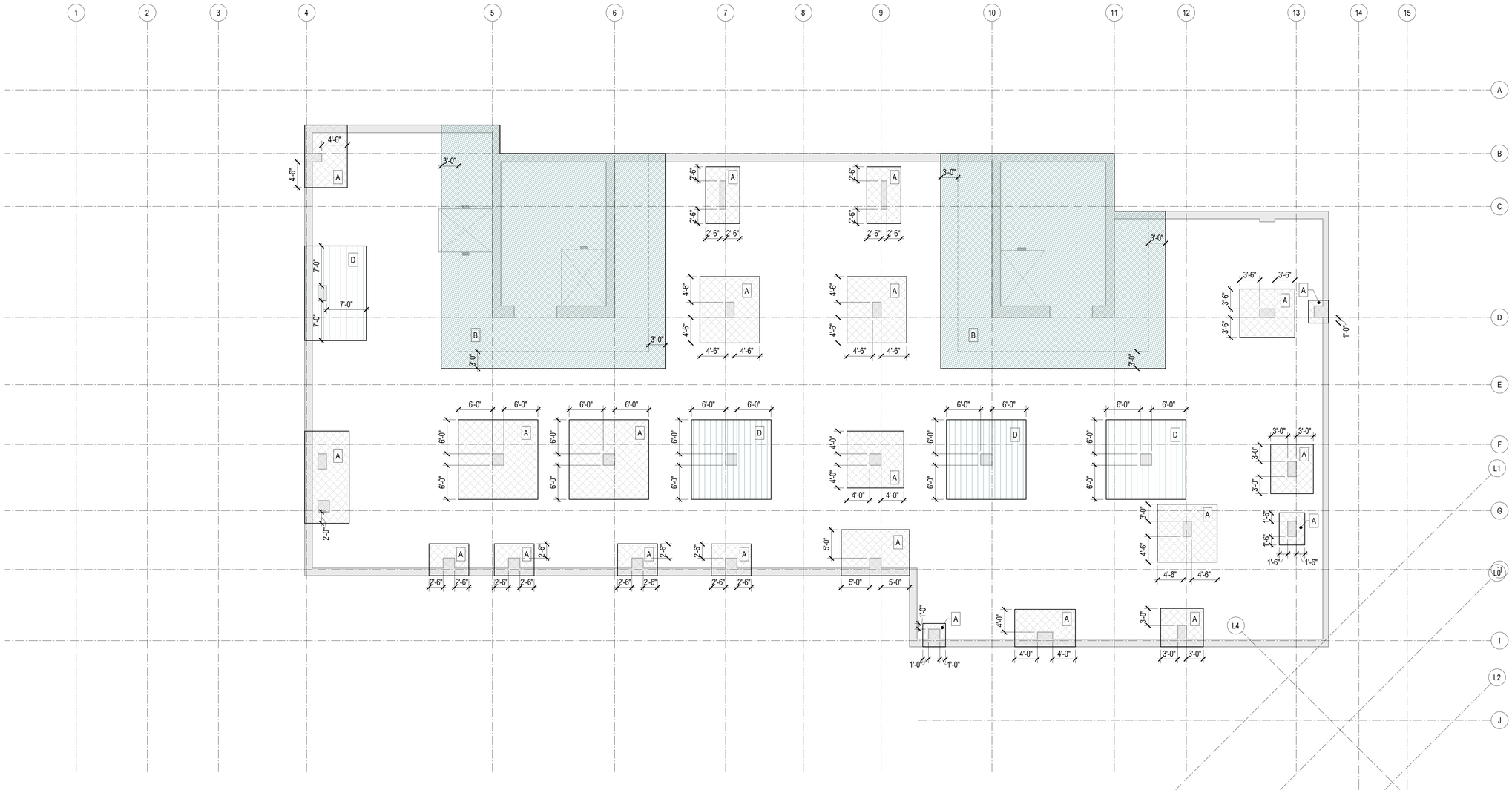
TOWER A LEVEL B1 LONGITUDINAL REINFORCING PLAN

S2.A.01.B



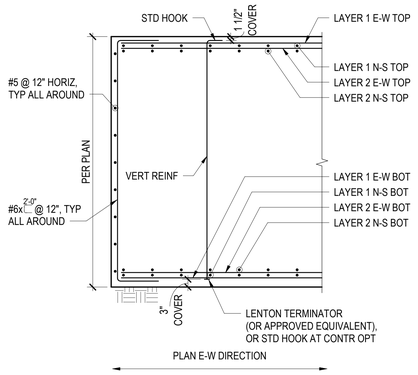
1 TOWER A - LEVEL B1 - SHEAR REINFORCEMENT PLAN

1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.

FOUNDATION VERTICAL REINFORCING SCHEDULE		
TYPE	REINFORCING	REMARKS
A	#7 @ 24" EACH WAY	1
B	#8 @ 24" EACH WAY	
C	#9 @ 24" EACH WAY	
D	#7 @ 24" EACH WAY	



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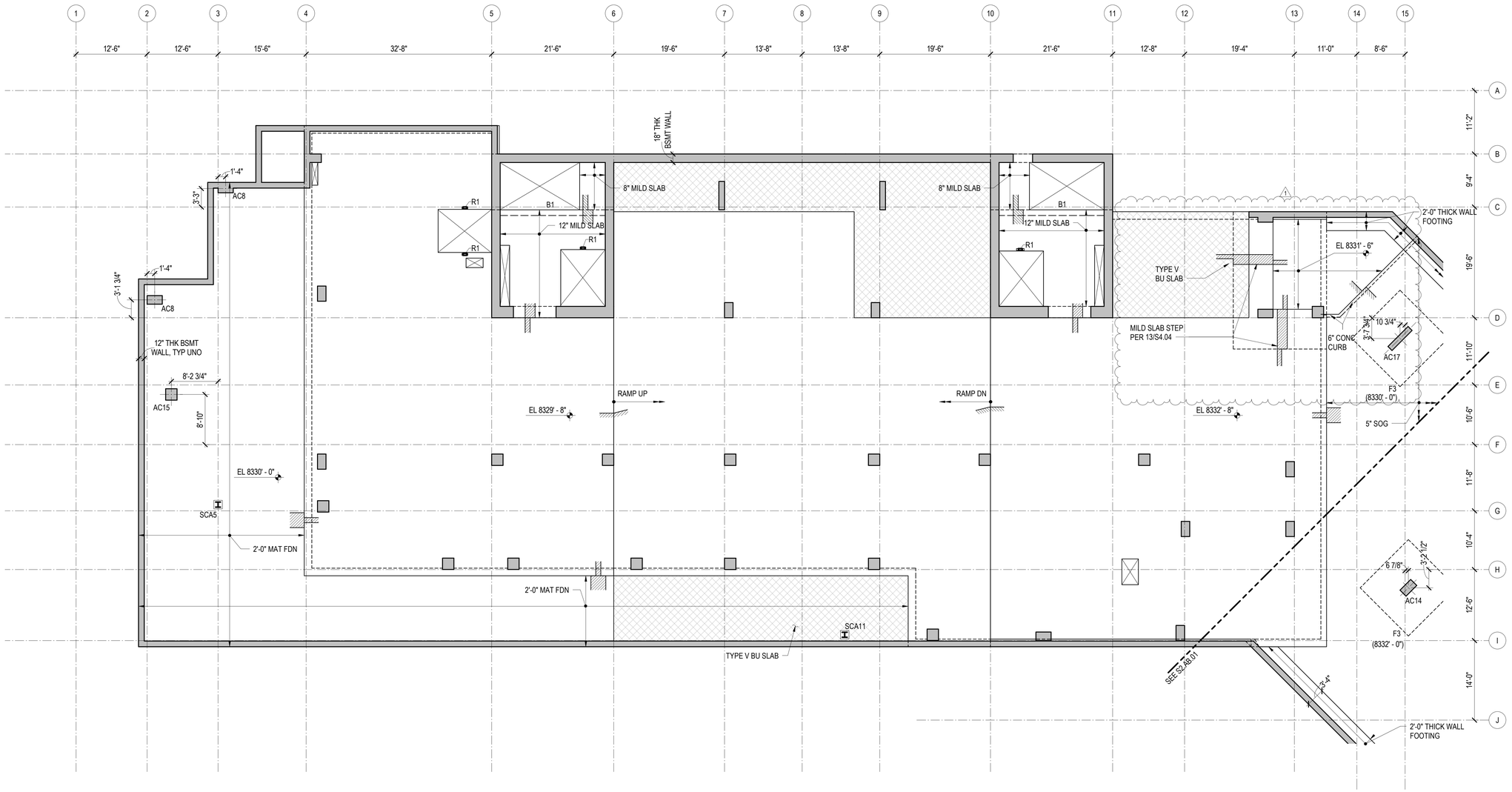
TOWER A LEVEL B1 SHEAR REINFORCING PLAN

S2.A.01.V



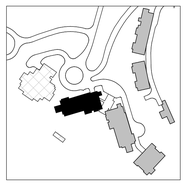
1 TOWER A - PARKING LEVEL 2 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8333' - 0". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. MAT FOUNDATION IS 2'-0" THICK UNLESS NOTED OTHERWISE. UPON REACHING THE MAT FOUNDATION SUBGRADE ELEVATION, SOIL CONDITIONS SHALL BE EVALUATED AND APPROVED BY THE GEOTECHNICAL ENGINEER OF RECORD.
3. THE STRUCTURAL SLAB IS A 10-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. [ ] INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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date 7/15/2022

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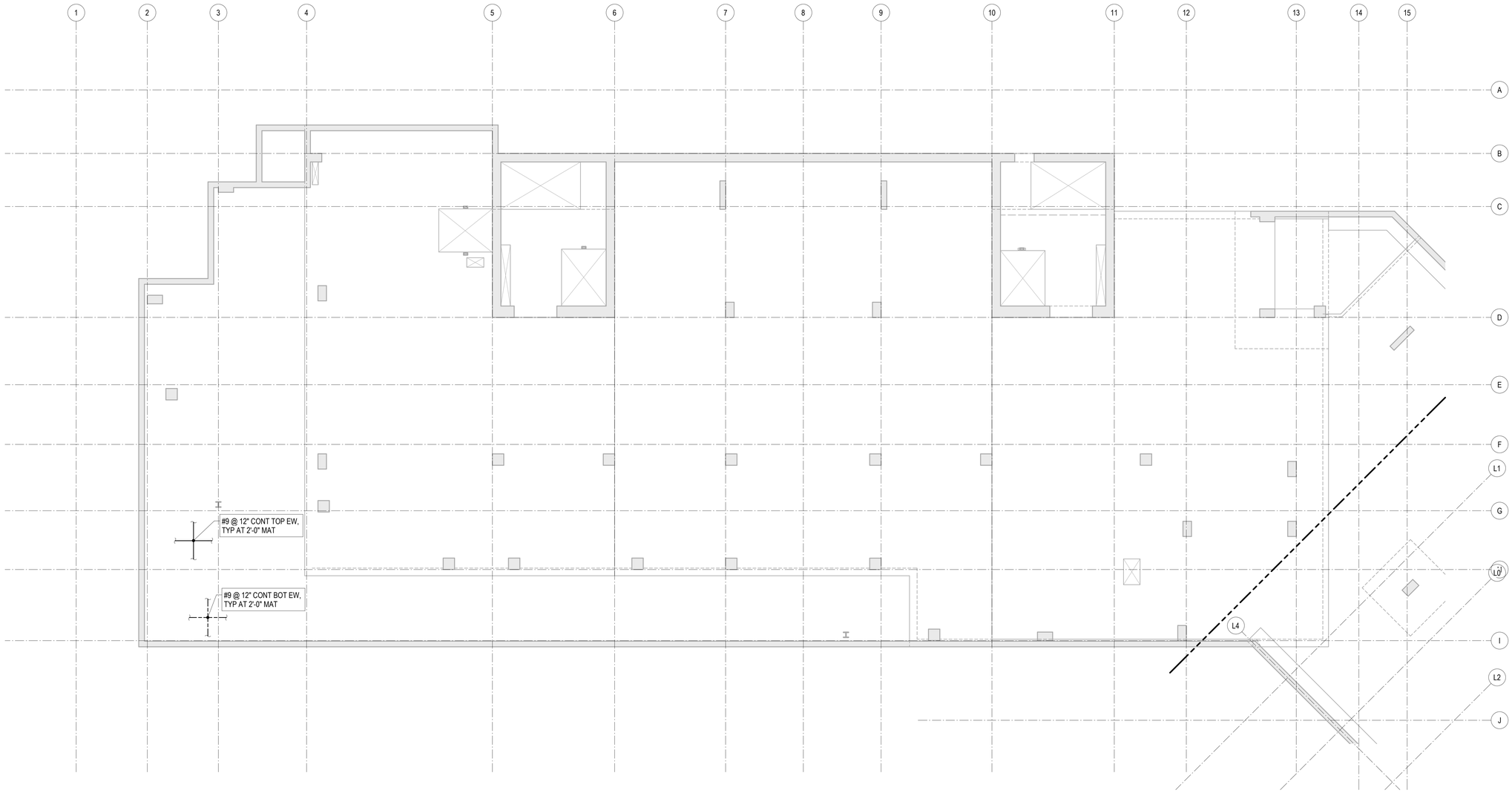
TOWER A LEVEL P2 FRAMING PLAN

S2.A.02

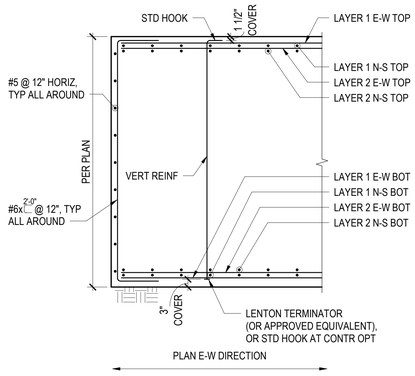


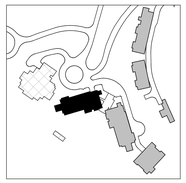
1 TOWER A - PARKING LEVEL 2 - REINFORCEMENT PLAN

1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.





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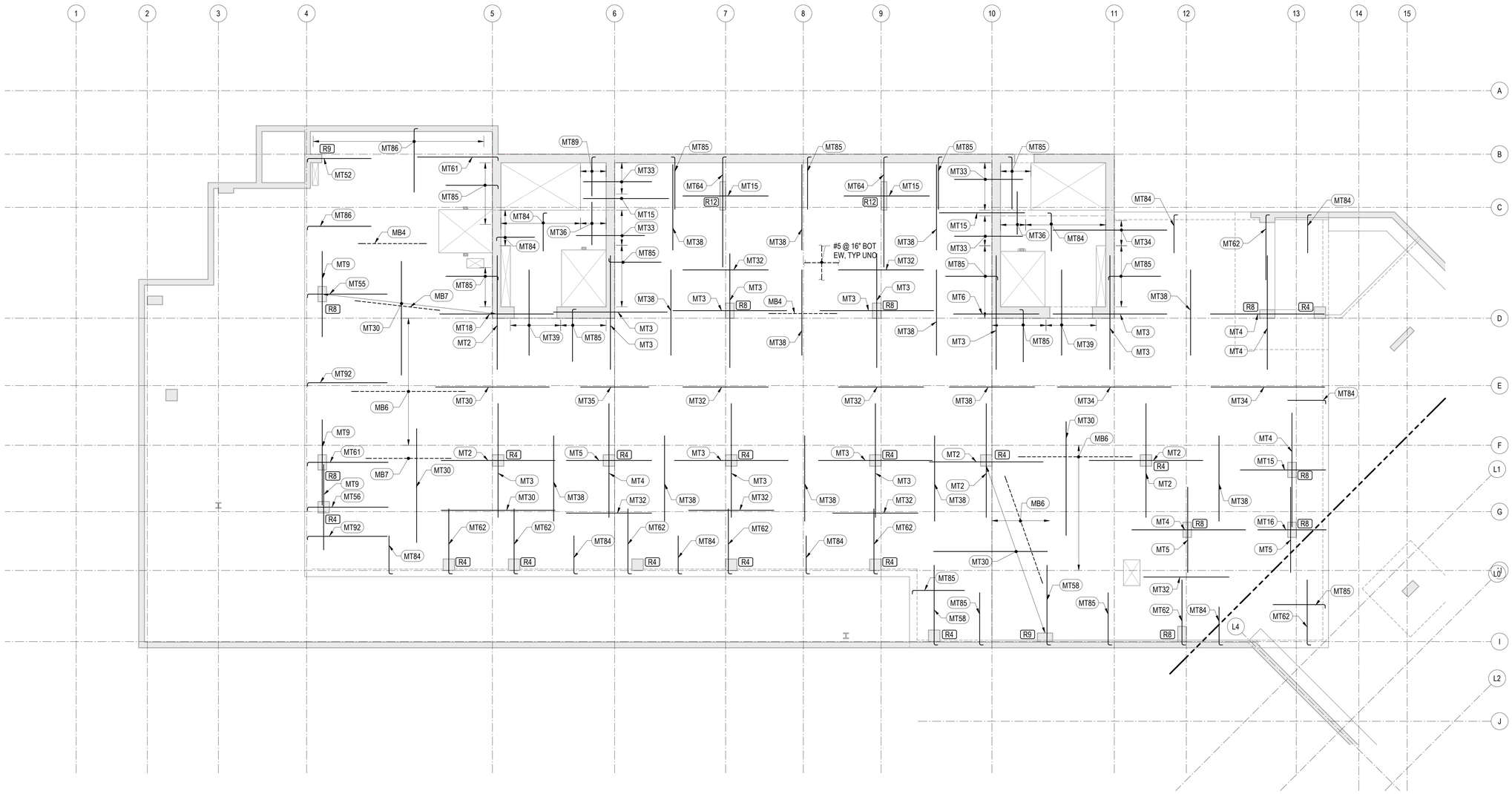
TOWER A LEVEL P2 MAT LONGITUDINAL REINFORCING PLAN

S2.A.02.B



1 TOWER A - PARKING LEVEL 2 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT3	(11) #7x20'-0" @ 12"	STAGGER 4'-0"
MT4	(11) #6x20'-0" @ 12"	STAGGER 4'-0"
MT5	(13) #5x15'-0" @ 10"	STAGGER 4'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x15'-0" @ 9"	STAGGER 3'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT11	(11) #5x12'-0" @ 12"	STAGGER 2'-0"
MT12	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT13	(21) #8x20'-0" @ 6"	STAGGER 5'-0"
MT14	(21) #7x20'-0" @ 6"	STAGGER 5'-0"
MT15	(11) #5x15'-0" @ 12"	STAGGER 3'-0"

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT16	(11) #4x12'-0" @ 12"	STAGGER 2'-0"
MT17	(11) #4x15'-0" @ 12"	STAGGER 3'-0"
MT18	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT31	#5x20'-0" @ 10"	STAGGER 2'-0"
MT32	#5x15'-0" @ 12"	STAGGER 2'-0"
MT33	#5x12'-0" @ 12"	STAGGER 2'-0"
MT34	#5x20'-0" @ 12"	STAGGER 4'-0"
MT35	#5x12'-0" @ 12"	STAGGER 1'-0"
MT36	#5x7'-6" @ 12"	STAGGER 0'-0"
MT37	#4x12'-0" @ 12"	STAGGER 1'-0"
MT38	#4x15'-0" @ 12"	STAGGER 1'-0"
MT39	#5x15'-0" @ 8"	STAGGER 2'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

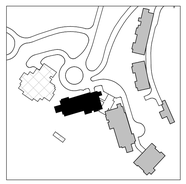
MARK	REINFORCING	REMARKS
MT51	(11) #5x6'-8" @ 12"	HOOK AT END
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14-2" @ 12"	HOOK AT END
MT55	(16) #6x14'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT57	(6) #6x9'-0" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT59	(11) #5x29'-3" @ 12"	HOOK AT END
MT60	(16) #7x10'-10" @ 8"	HOOK AT END
MT61	(11) #5x14'-2" @ 12"	HOOK AT END
MT62	(11) #4x11'-5" @ 12"	HOOK AT END
MT63	(11) #4x14'-5" @ 12"	HOOK AT END
MT64	(11) #4x19'-4" @ 12"	HOOK AT END
MT65	(11) #4x6'-10" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT87	#6x11'-0" @ 12"	HOOK AT END
MT88	#4x14'-5" @ 12"	HOOK AT END
MT89	#4x6'-10" @ 12"	HOOK AT END
MT90	#4 @ 12"	HOOK BOTH ENDS
MT91	#4x9'-5" @ 12"	HOOK AT END,
MT92	#6x14'-0" @ 12"	HOOK AT END
MT93	#5x19'-3" @ 10"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB1	#5x20'-0" @ 12"	STAGGER 2'-0"
MB2	#5x12'-0" @ 24"	STAGGER 2'-0"
MB4	(3) #5x12'-0" @ 14"	STAGGER 2'-0"
MB5	#5x20'-0" @ 18"	STAGGER 2'-0"
MB6	#5x20'-0" @ 16"	STAGGER 2'-0"
MB7	(6) #5x15'-0" @ 16"	STAGGER 2'-0"
MB8	(11) #5x20'-0" @ 12"	STAGGER 3'-0"
MB9	(3) #5x15'-0" @ 24"	STAGGER 2'-0"
MB15	(6) #5x30'-0" @ 24"	STAGGER 3'-0"



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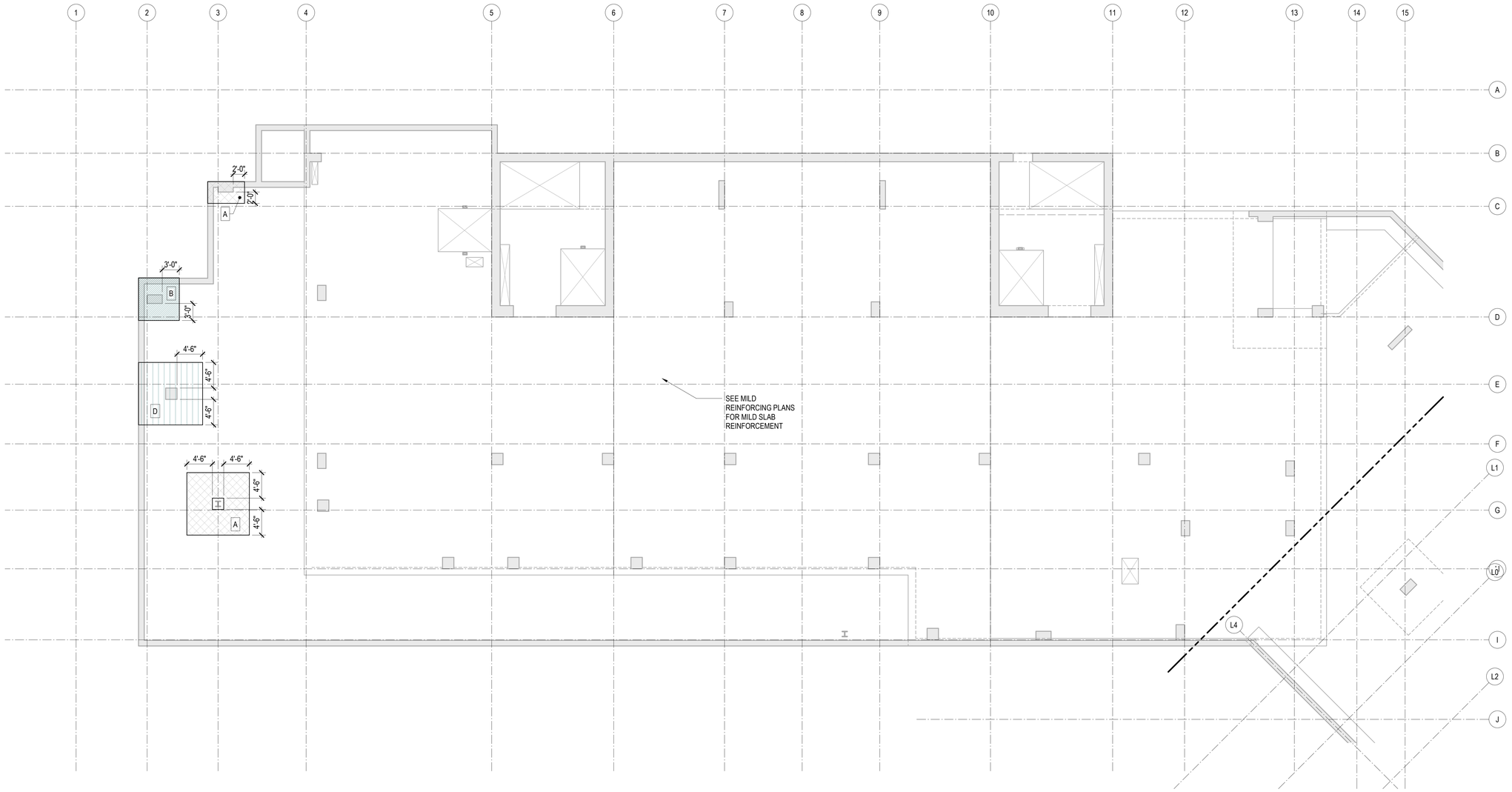
TOWER A LEVEL P2
REINFORCING
PLAN

S2.A.02.R



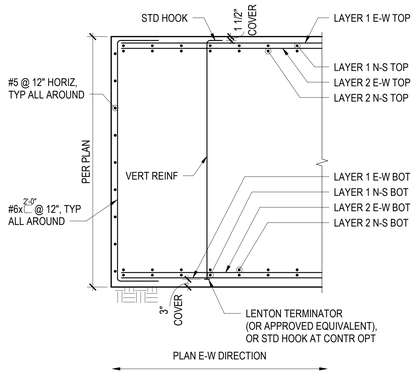
1 TOWER A - PARKING LEVEL 2 - MAT SHEAR REINFORCEMENT PLAN

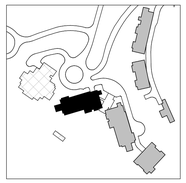
1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.

FOUNDATION VERTICAL REINFORCING SCHEDULE		
TYPE	REINFORCING	REMARKS
A	#7 @ 24" EACH WAY	
B	#8 @ 24" EACH WAY	
C	#9 @ 24" EACH WAY	
D	#7 @ 24" EACH WAY	





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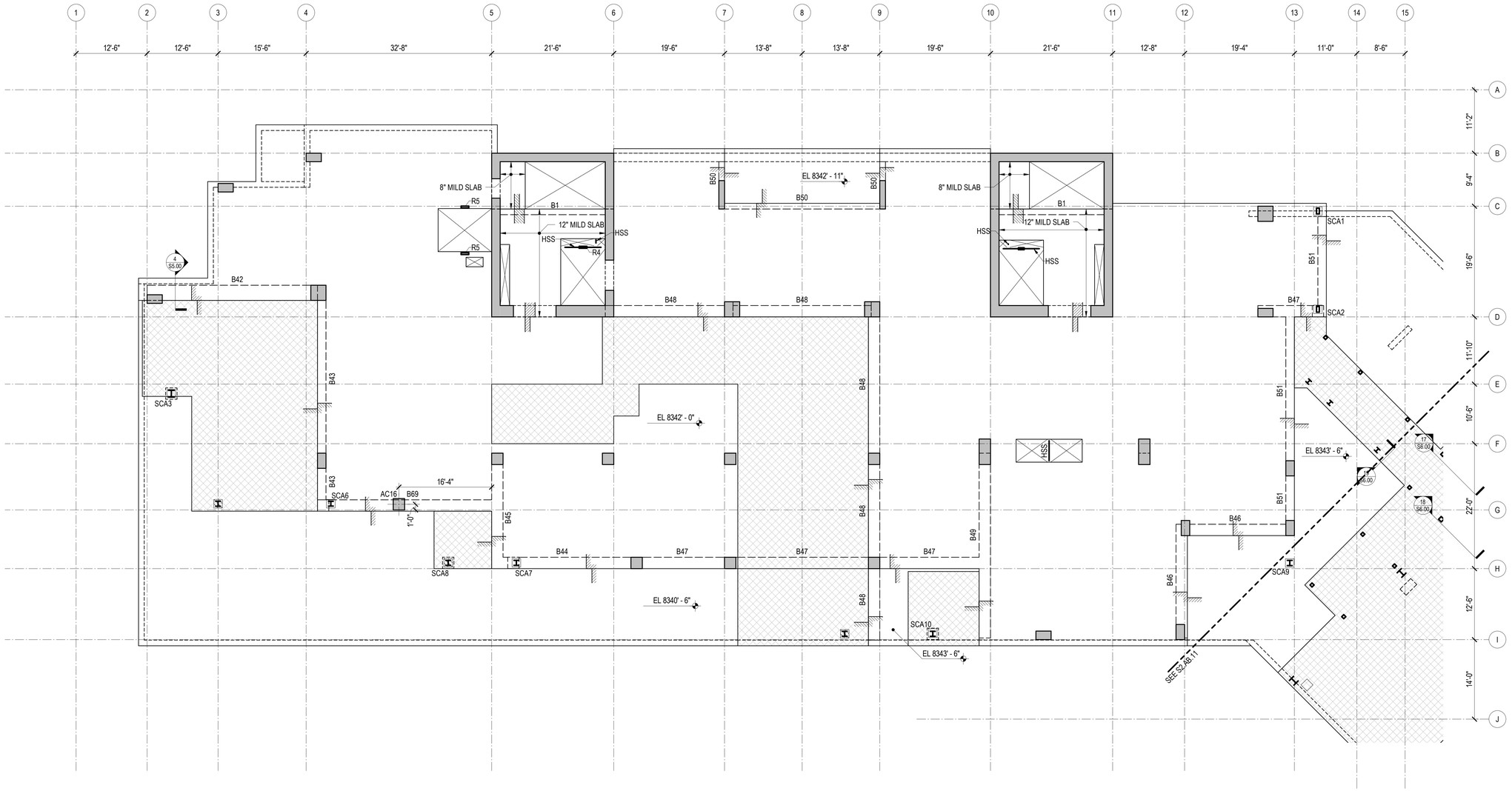
TOWER A LEVEL P2
MAT SHEAR
REINFORCING
PLAN

S2.A.02.V



1 TOWER A - LEVEL 1 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
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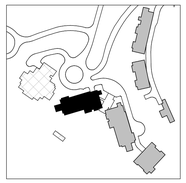
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8345' - 0". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 14-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. [...] INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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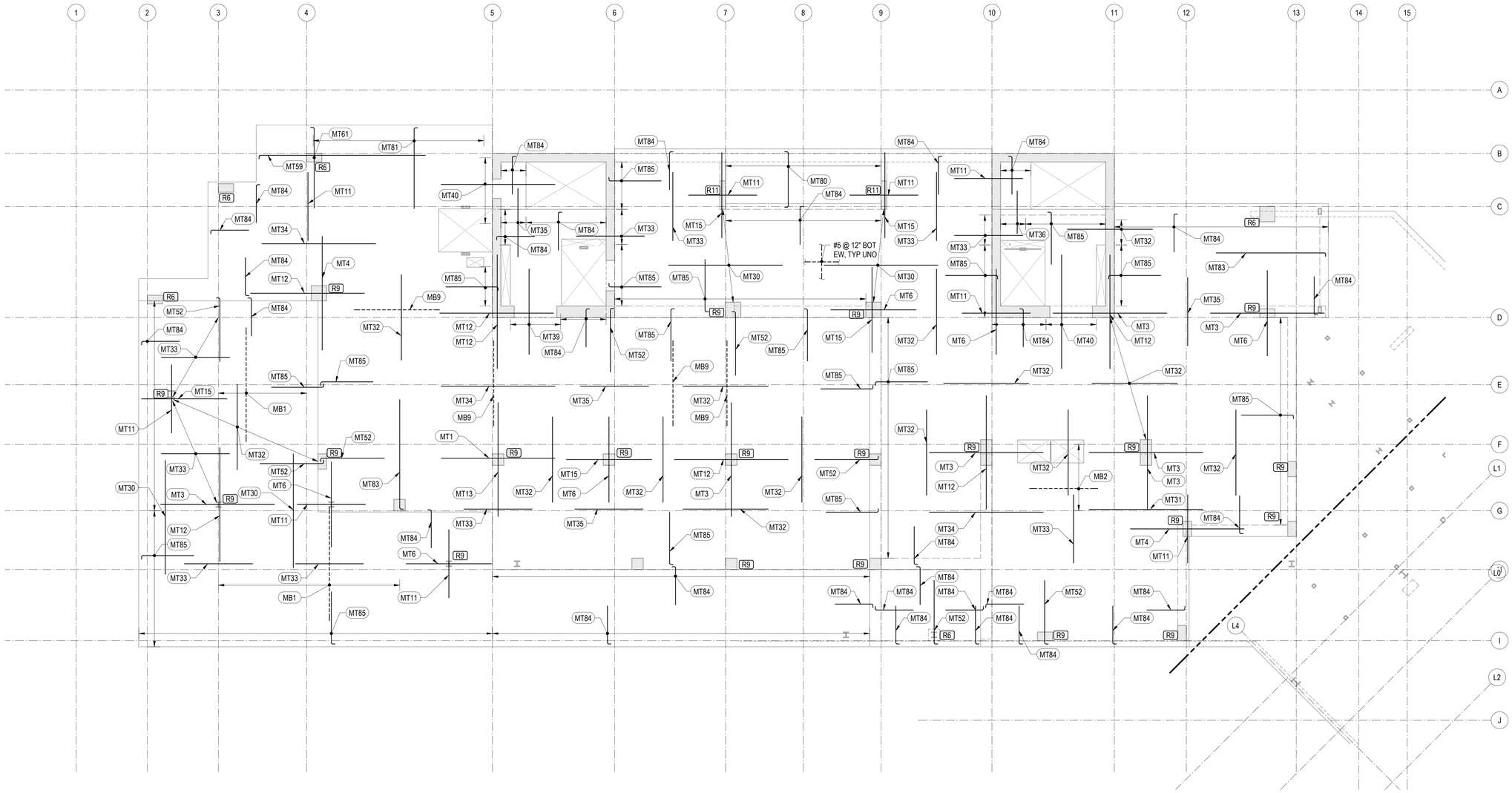
TOWER A LEVEL 1 FRAMING PLAN

S2.A.11



1 TOWER A - LEVEL 1 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT3	(11) #7x20'-0" @ 12"	STAGGER 4'-0"
MT4	(11) #6x20'-0" @ 12"	STAGGER 4'-0"
MT5	(13) #5x15'-0" @ 10"	STAGGER 4'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x15'-0" @ 9"	STAGGER 3'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT11	(11) #5x12'-0" @ 12"	STAGGER 2'-0"
MT12	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT13	(21) #8x20'-0" @ 6"	STAGGER 5'-0"
MT14	(21) #7x20'-0" @ 6"	STAGGER 5'-0"
MT15	(11) #5x15'-0" @ 12"	STAGGER 3'-0"

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT16	(11) #4x12'-0" @ 12"	STAGGER 2'-0"
MT17	(11) #4x15'-0" @ 12"	STAGGER 3'-0"
MT18	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT31	#5x20'-0" @ 10"	STAGGER 2'-0"
MT32	#5x15'-0" @ 12"	STAGGER 2'-0"
MT33	#5x12'-0" @ 12"	STAGGER 2'-0"
MT34	#5x20'-0" @ 12"	STAGGER 4'-0"
MT35	#5x12'-0" @ 12"	STAGGER 1'-0"
MT36	#5x7'-6" @ 12"	STAGGER 0'-0"
MT37	#4x12'-0" @ 12"	STAGGER 1'-0"
MT38	#4x15'-0" @ 12"	STAGGER 1'-0"
MT39	#5x15'-0" @ 8"	STAGGER 2'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

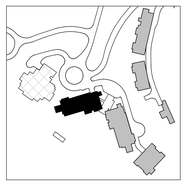
MARK	REINFORCING	REMARKS
MT51	(11) #5x6'-8" @ 12"	HOOK AT END
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14-2" @ 12"	HOOK AT END
MT55	(16) #6x14'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT57	(6) #6x9'-0" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT59	(11) #5x29'-3" @ 12"	HOOK AT END
MT60	(16) #7x10'-10" @ 8"	HOOK AT END
MT61	(11) #5x14'-2" @ 12"	HOOK AT END
MT62	(11) #4x11'-5" @ 12"	HOOK AT END
MT63	(11) #4x14'-5" @ 12"	HOOK AT END
MT64	(11) #4x19'-4" @ 12"	HOOK AT END
MT65	(11) #4x6'-10" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT87	#6x11'-0" @ 12"	HOOK AT END
MT88	#4x14'-5" @ 12"	HOOK AT END
MT89	#4x6'-10" @ 12"	HOOK AT END
MT90	#4 @ 12"	HOOK BOTH ENDS
MT91	#4x9'-5" @ 12"	HOOK AT END,
MT92	#6x14'-0" @ 12"	HOOK AT END
MT93	#5x19'-3" @ 10"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB1	#5x20'-0" @ 12"	STAGGER 2'-0"
MB2	#5x12'-0" @ 24"	STAGGER 2'-0"
MB4	(3) #5x12'-0" @ 14"	STAGGER 2'-0"
MB5	#5x20'-0" @ 18"	STAGGER 2'-0"
MB6	#5x20'-0" @ 16"	STAGGER 2'-0"
MB7	(6) #5x15'-0" @ 16"	STAGGER 2'-0"
MB8	(11) #5x20'-0" @ 12"	STAGGER 3'-0"
MB9	(3) #5x15'-0" @ 24"	STAGGER 2'-0"
MB15	(6) #5x30'-0" @ 24"	STAGGER 3'-0"



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job no. 20052
date 7/15/2022

revisions:

no.	date	by

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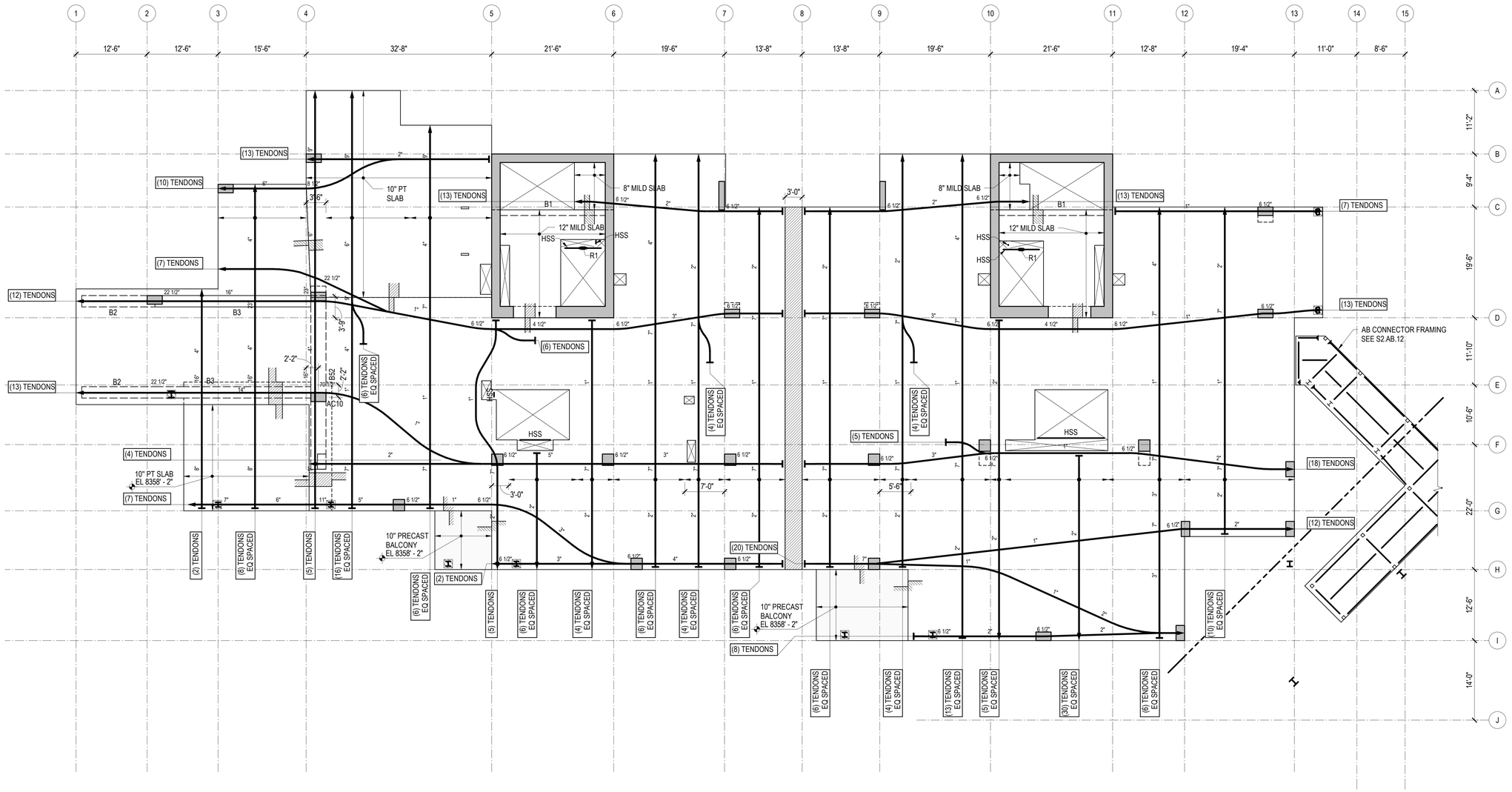
TOWER A LEVEL 1 REINFORCING PLAN

S2.A.11.R



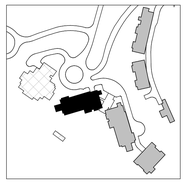
1 TOWER A - LEVEL 2 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8359' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8358' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.



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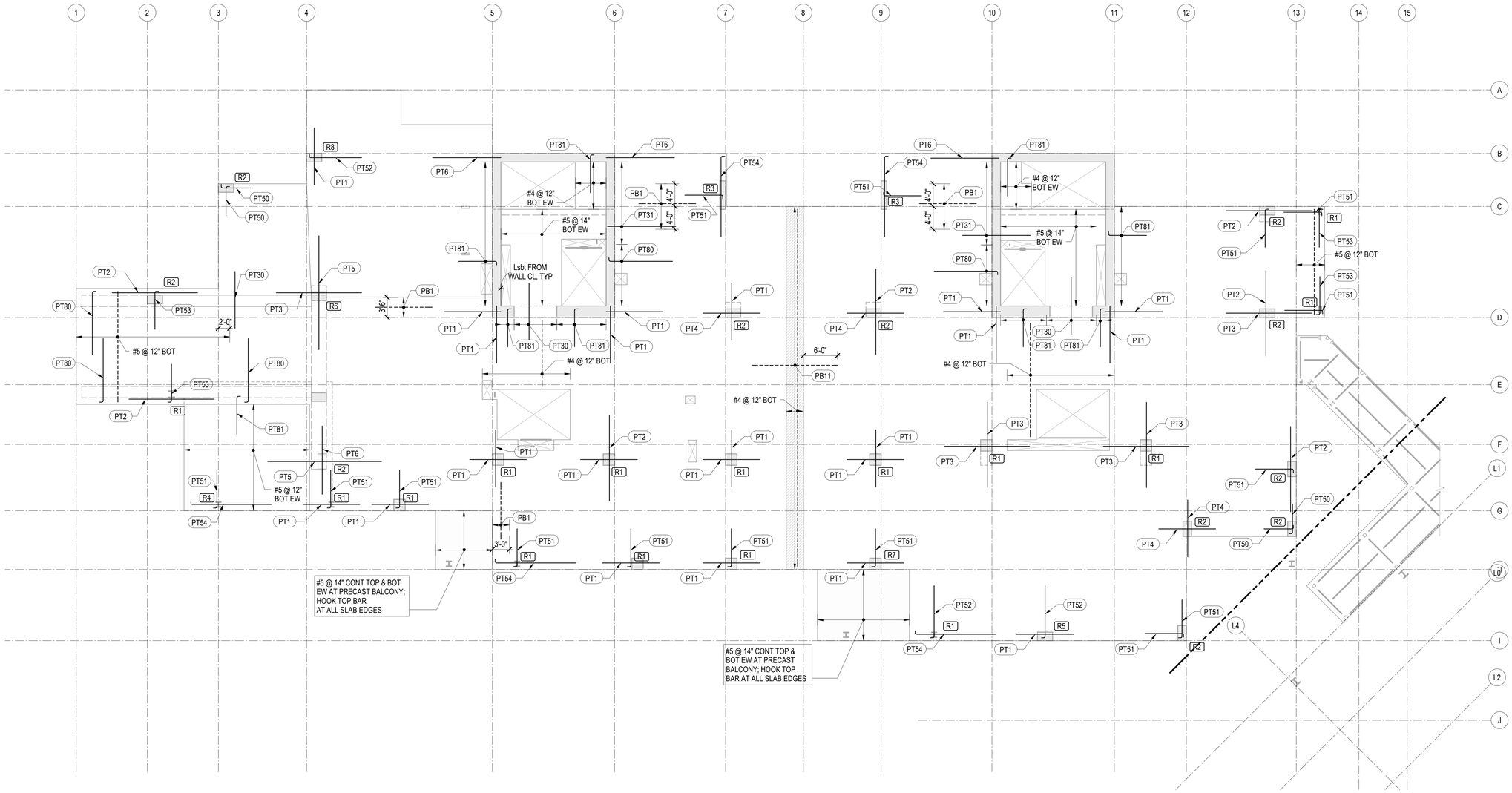
TOWER A LEVEL 2 FRAMING PLAN

S2.A.12



1 TOWER A - LEVEL 2 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05

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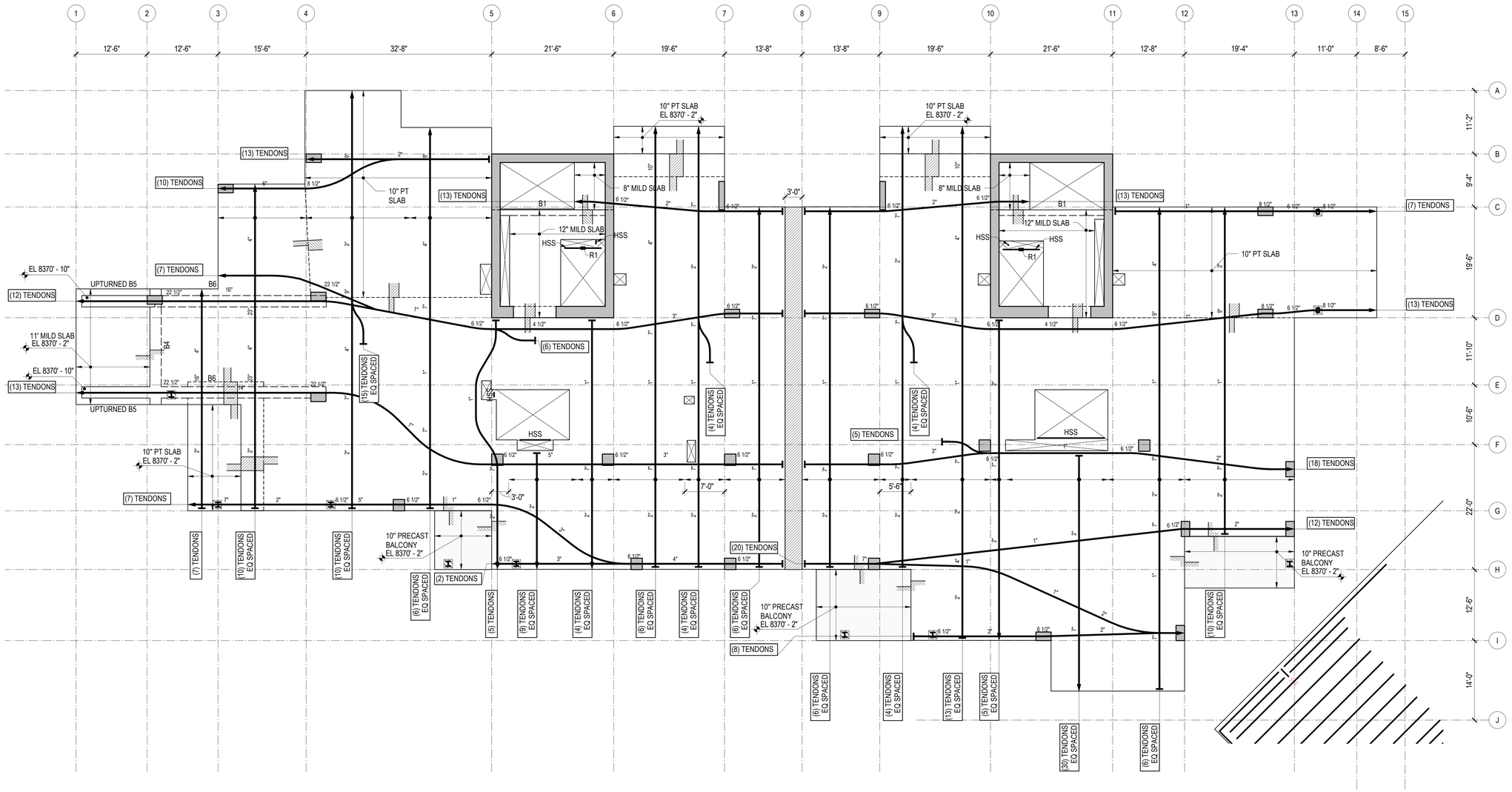
TOWER A LEVEL 2 REINFORCING PLAN

S2.A.12.R



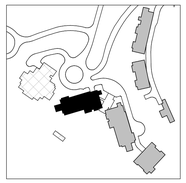
1 TOWER A - LEVEL 3 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8371' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8370' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.



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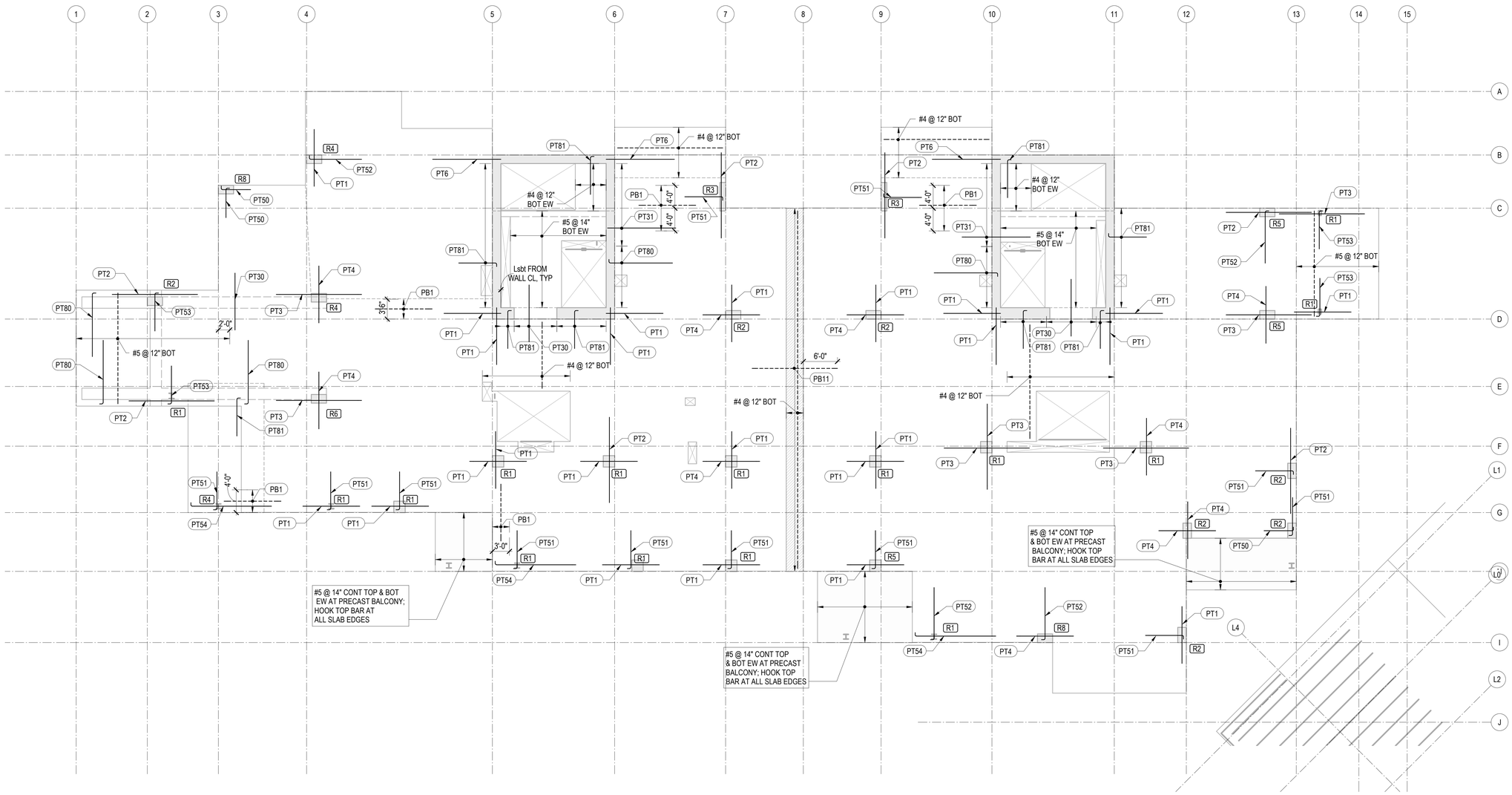
TOWER A LEVEL 3 FRAMING PLAN

S2.A.13



1 TOWER A - LEVEL 3 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

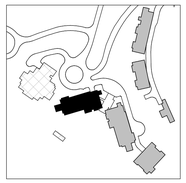
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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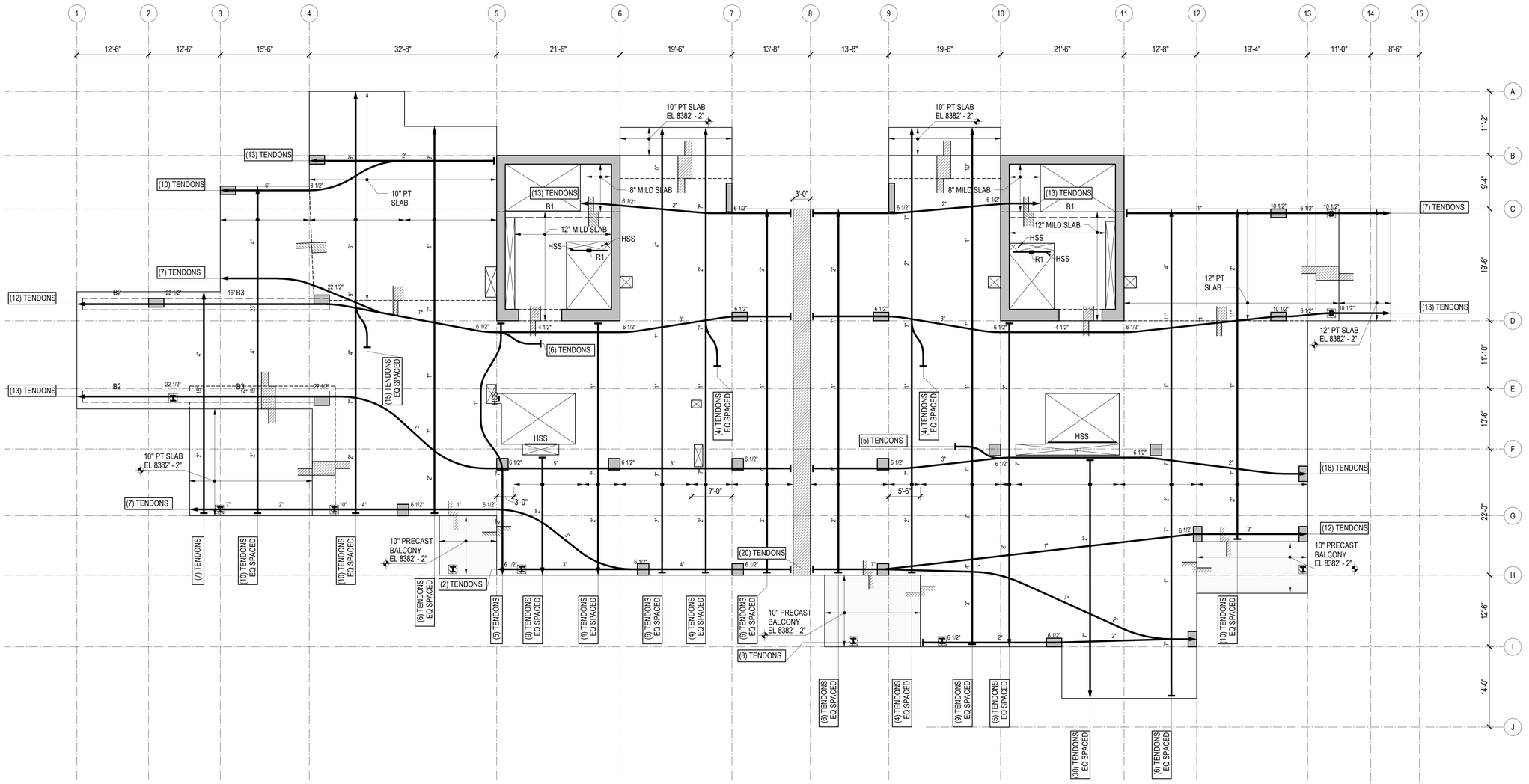
TOWER A LEVEL 3 REINFORCING PLAN

S2.A.13.R



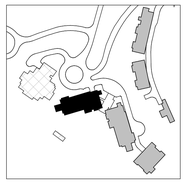
1 TOWER A - LEVEL 4 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8383' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8382' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.
10. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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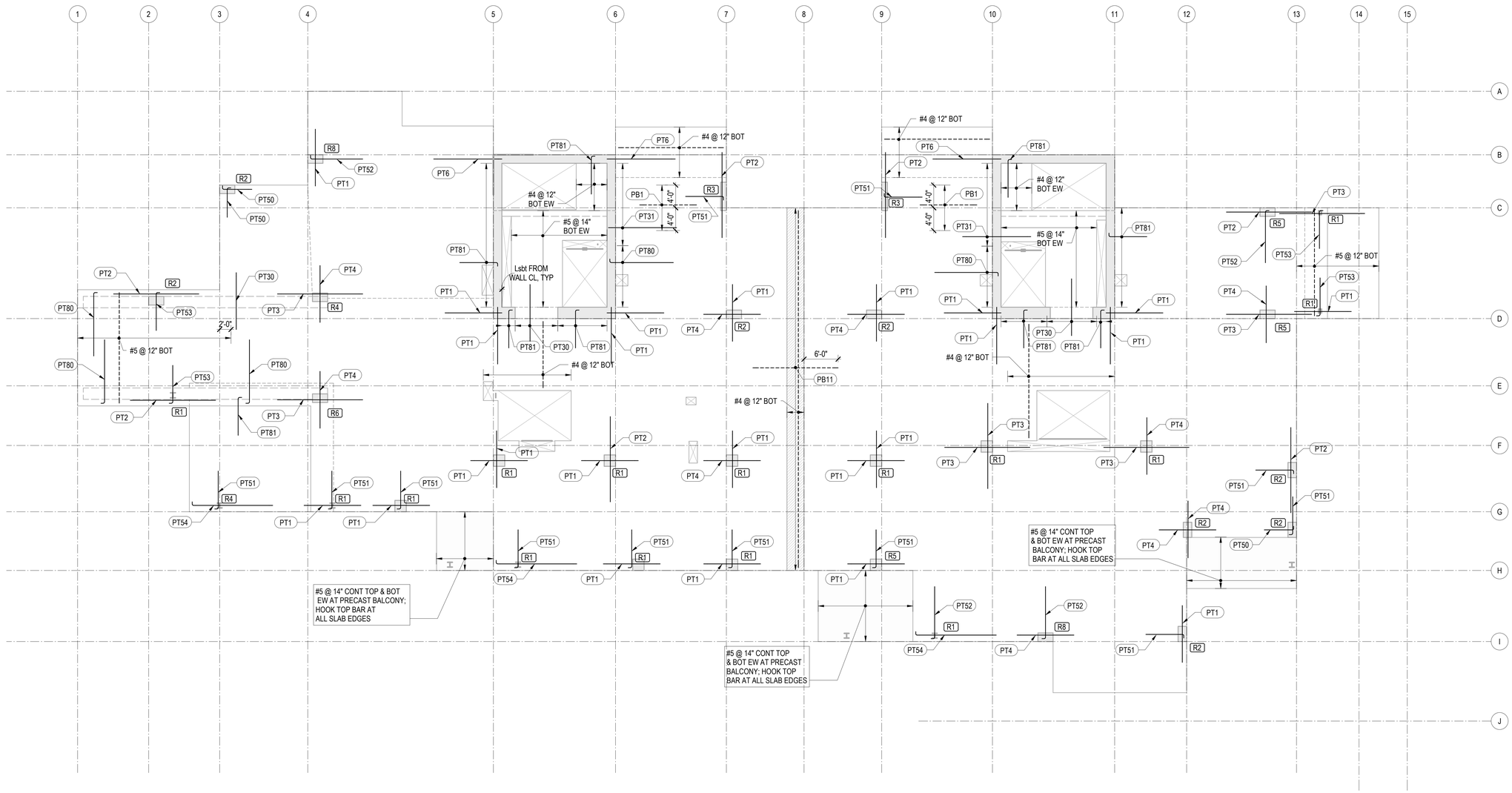
TOWER A LEVEL 4 FRAMING PLAN

S2.A.14



1 TOWER A - LEVEL 4 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

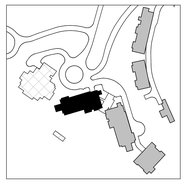
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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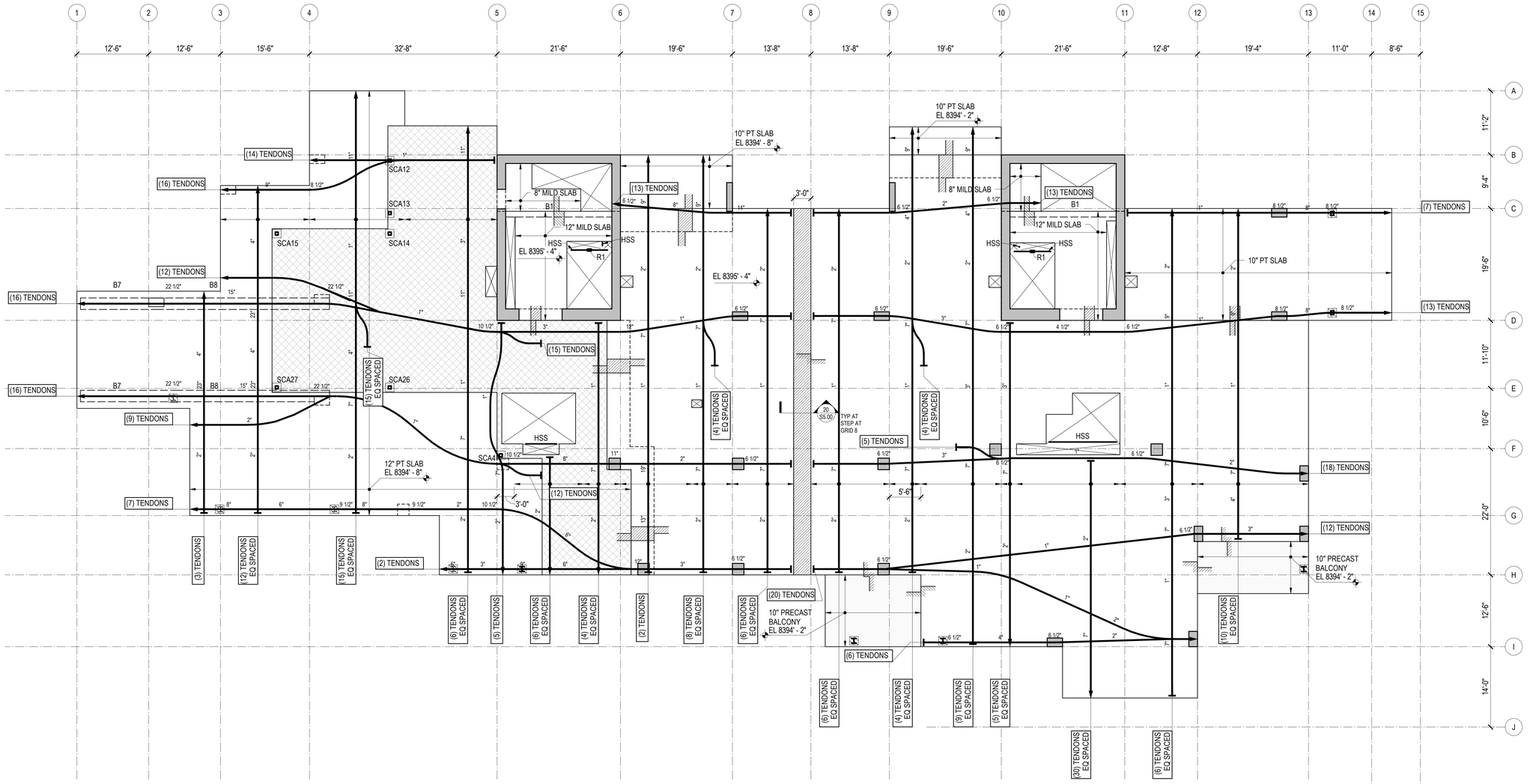
TOWER A LEVEL 4 REINFORCING PLAN

S2.A.14.R



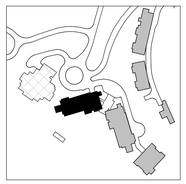
1 TOWER A - LEVEL 5 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8395' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8394' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.
10. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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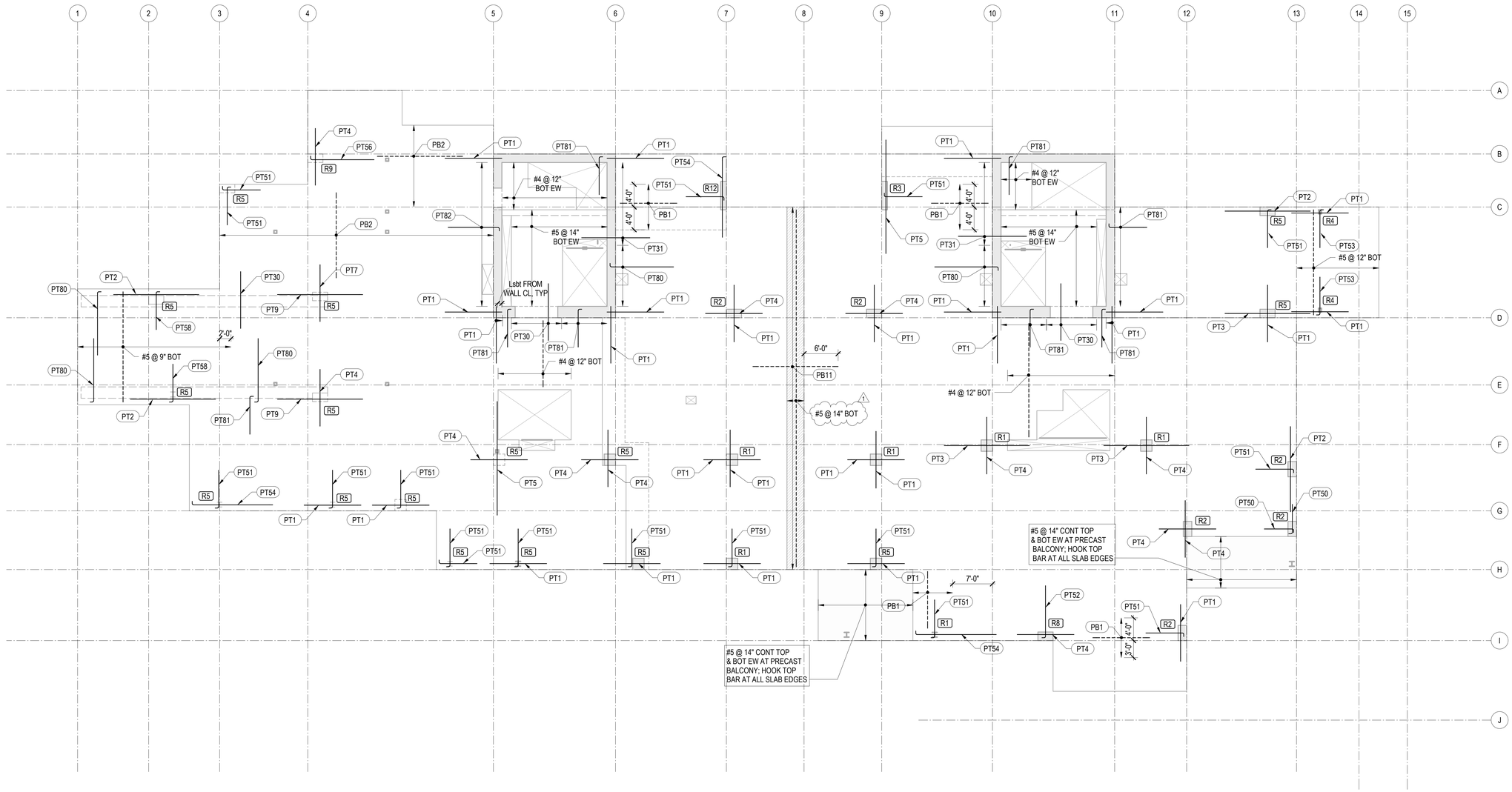
TOWER A LEVEL 5 FRAMING PLAN

S2.A.15



1 TOWER A - LEVEL 5 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
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8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

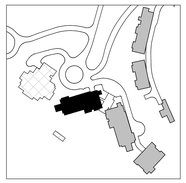
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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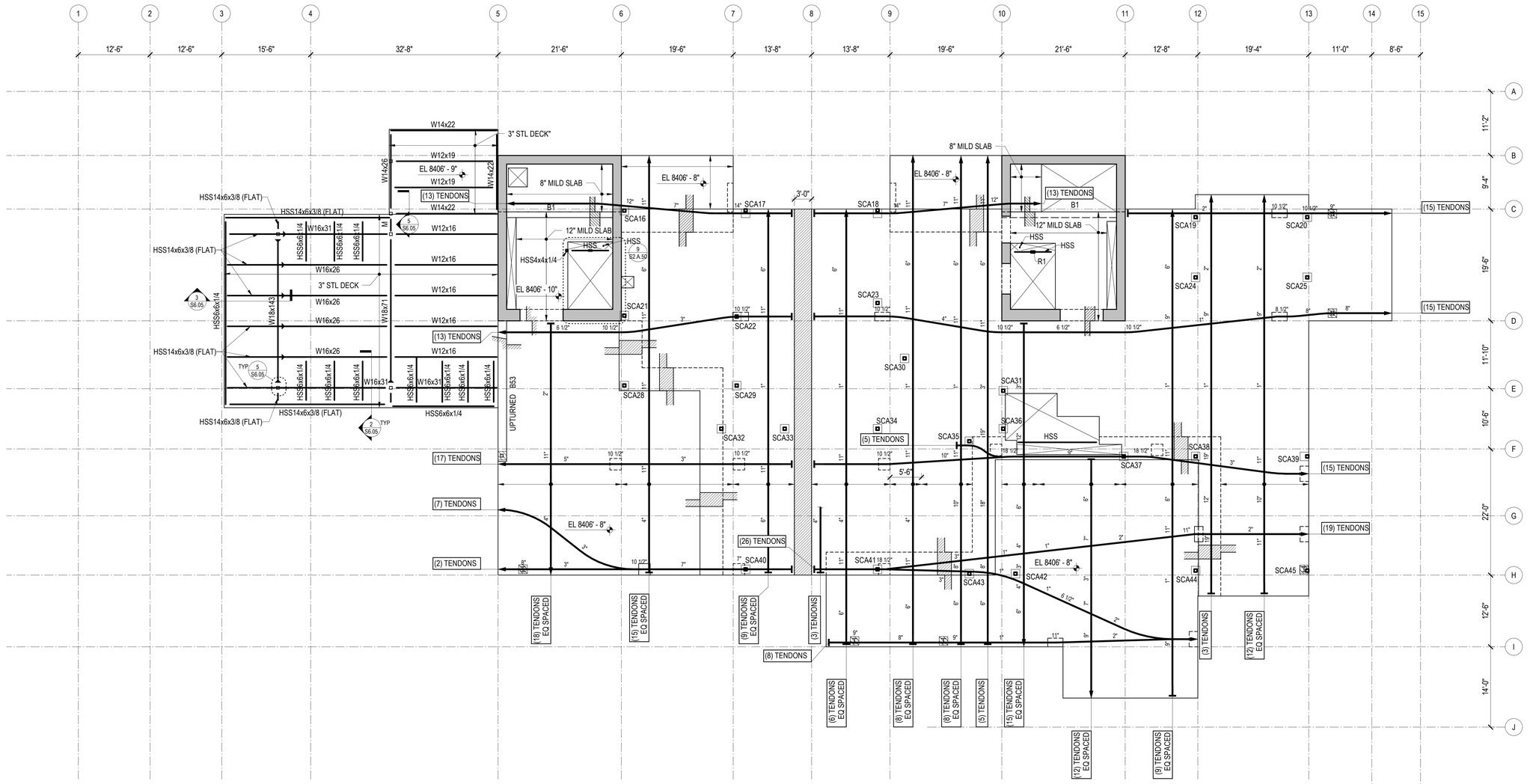
TOWER A LEVEL 5 REINFORCING PLAN

S2.A.15.R



1 TOWER A - LEVEL 6 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8407' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8407' - 4" , UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS A 12-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
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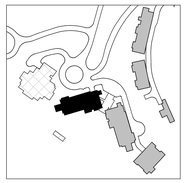
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.
10. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.
11. ROOF DECK IS MINIMUM 3 INCH X 20 GAUGE STEEL DECK. DECKING IS TO BE INSTALLED IN MINIMUM TWO SPAN CONDITION WHERE POSSIBLE. DECKING CONNECTIONS ARE TO BE AS FOLLOWS:

    SIDE LAP SEAM CONNECTIONS ARE 1-1/2 INCH TOP SEAM WELDS AT 18 INCHES MAXIMUM.

    CONNECTION TO SUPPORTING FRAMING SHALL BE A 24/4 WELD PATTERN WITH 3/4 INCH PUDDLE WELDS.

    CONNECTION TO FRAMING BELOW AND IN LINE WITH FLUTE SHALL BE 3/4 INCH PUDDLE WELDS AT 18 INCHES MAXIMUM.

    BUTTON PUNCHING OF SIDE LAP CONNECTIONS IS NOT PERMITTED.



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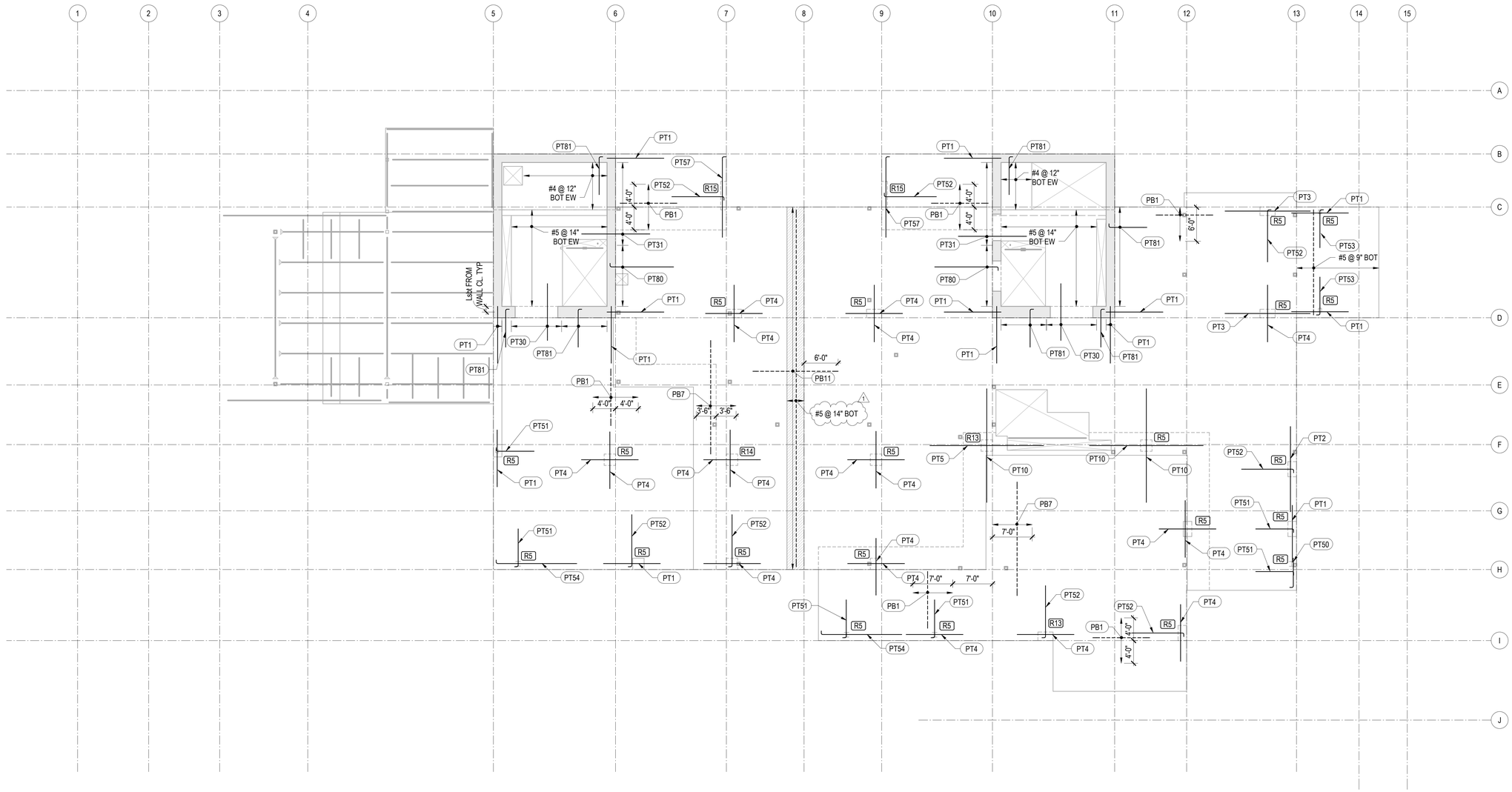
TOWER A LEVEL 6 FRAMING PLAN

S2.A.16



1 TOWER A - LEVEL 6 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
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7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

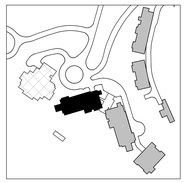
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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project manager
drawn by
Author
checked by JB
job no. 20052
date 7/15/2022

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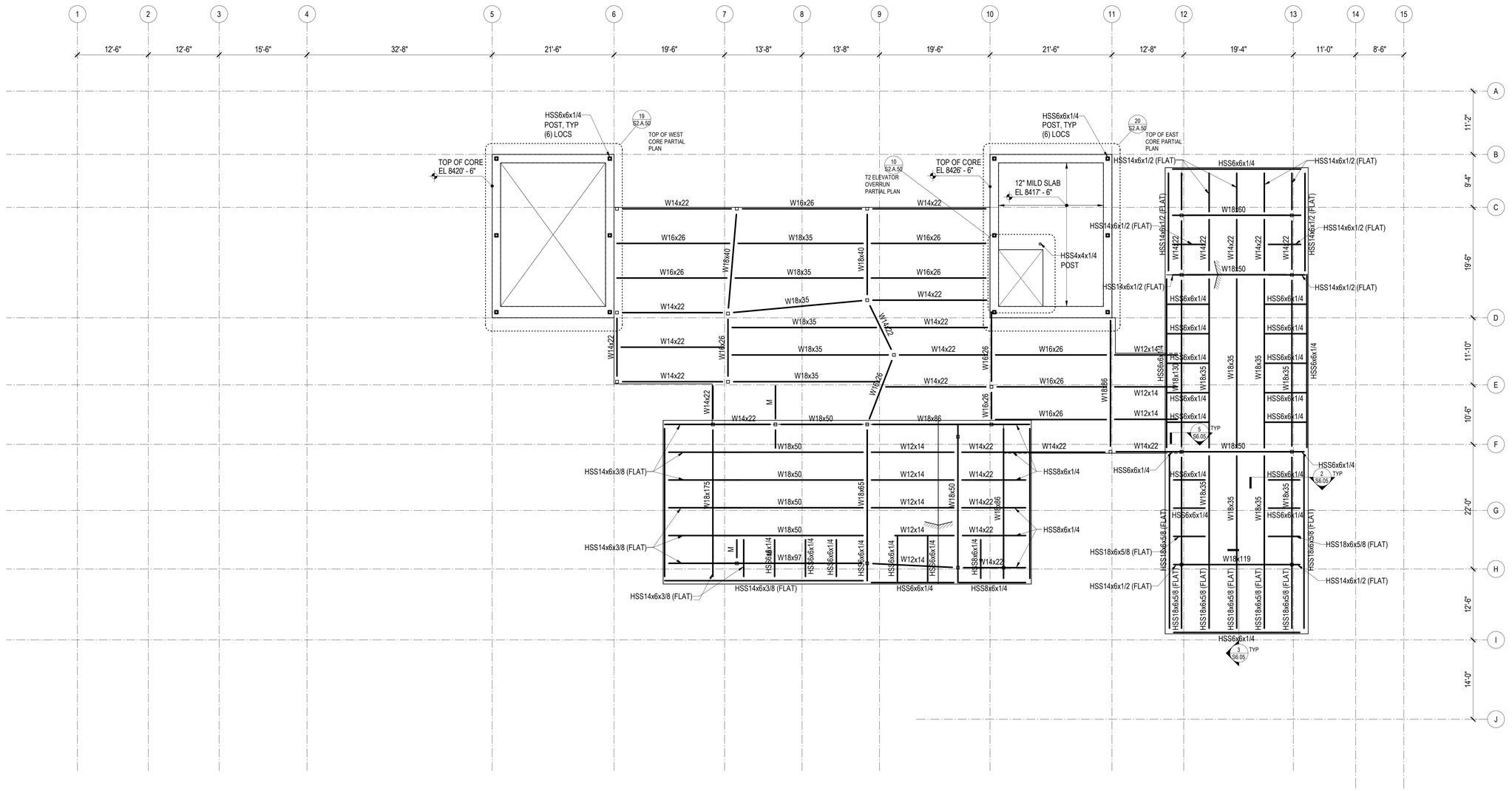
TOWER A LEVEL 6 REINFORCING PLAN

S2.A.16.R



1 TOWER A - ROOF FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

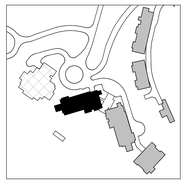
1. REFERENCE FLOOR ELEVATION IS 8419' - 6". TOP OF SLAB ON STEEL DECK IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. ROOF DECK IS MINIMUM 3 INCH X 20 GAUGE STEEL DECK. DECKING IS TO BE INSTALLED IN MINIMUM TWO SPAN CONDITION WHERE POSSIBLE. DECKING CONNECTIONS ARE TO BE AS FOLLOWS:

   SIDE LAP SEAM CONNECTIONS ARE 1-1/2 INCH TOP SEAM WELDS AT 18 INCHES MAXIMUM.

   CONNECTION TO SUPPORTING FRAMING SHALL BE A 24/4 WELD PATTERN WITH 3/4 INCH PUDDLE WELDS.

   CONNECTION TO FRAMING BELOW AND IN LINE WITH FLUTE SHALL BE 3/4 INCH PUDDLE WELDS AT 18 INCHES MAXIMUM.

   BUTTON PUNCHING OF SIDE LAP CONNECTIONS IS NOT PERMITTED.
3. REFERENCE TOP OF STEEL IS AT THE BOTTOM OF SLAB ON STEEL DECK UNLESS NOTED OTHERWISE.
4. STEEL SLOPES UNIFORMLY BETWEEN GIVEN TOP OF STEEL ELEVATIONS. WHERE BEAMS OR BEAMS AND COLUMNS INTERSECT, MATCH TOP OF STEEL UNLESS NOTED OTHERWISE.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING SLABS AND WALLS.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE 'TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE,' 'TYPICAL ROOF DECK OPENINGS,' AND 'TYPICAL COMPOSITE DECK OPENINGS' DETAILS FOR OPENING PLACEMENT CRITERIA AND REINFORCING OR FRAMING REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.



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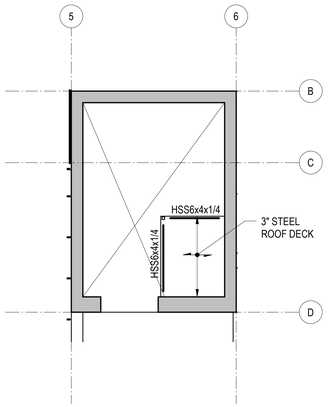
TOWER A ROOF FRAMING PLAN

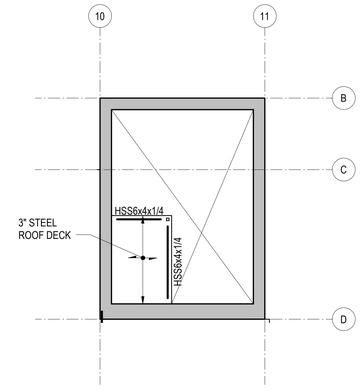
S2.A.17



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8410'-7". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

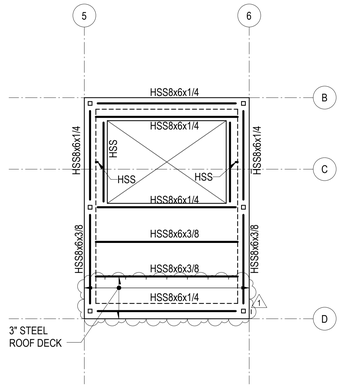
9 PARTIAL PLAN - T1 ELEVATOR OVERRUN
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8421'-3". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

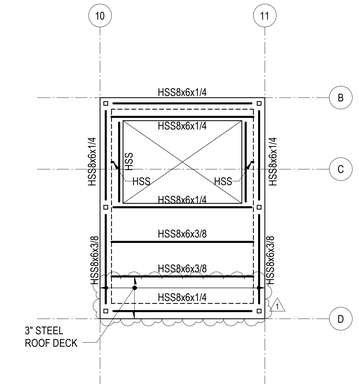
10 PARTIAL PLAN - T2 ELEVATOR OVERRUN
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8423'-3". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

19 PARTIAL PLAN - TOP OF WEST CORE
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8429'-9". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

20 PARTIAL PLAN - TOP OF EAST CORE
1/8" = 1'-0"

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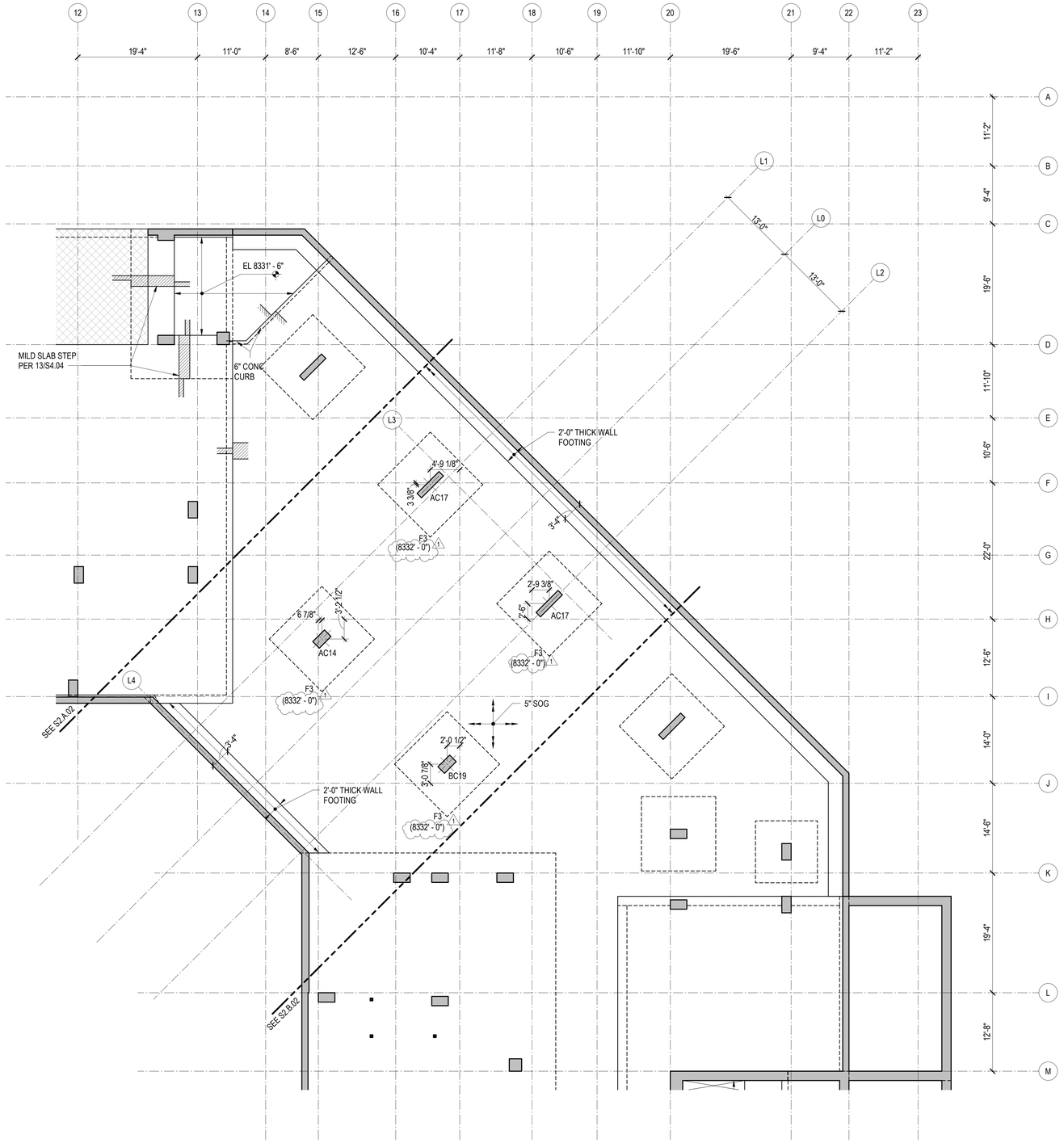
TOWER A PARTIAL PLANS

S2.A.50



1 TOWER AB - PARKING LEVEL 2 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

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S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8333' - 0". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. MAT FOUNDATION IS 3'-0" THICK UNLESS NOTED OTHERWISE. UPON REACHING THE MAT FOUNDATION SUBGRADE ELEVATION, SOIL CONDITIONS SHALL BE EVALUATED AND APPROVED BY THE GEOTECHNICAL ENGINEER OF RECORD.
3. THE STRUCTURAL SLAB IS A 10-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.

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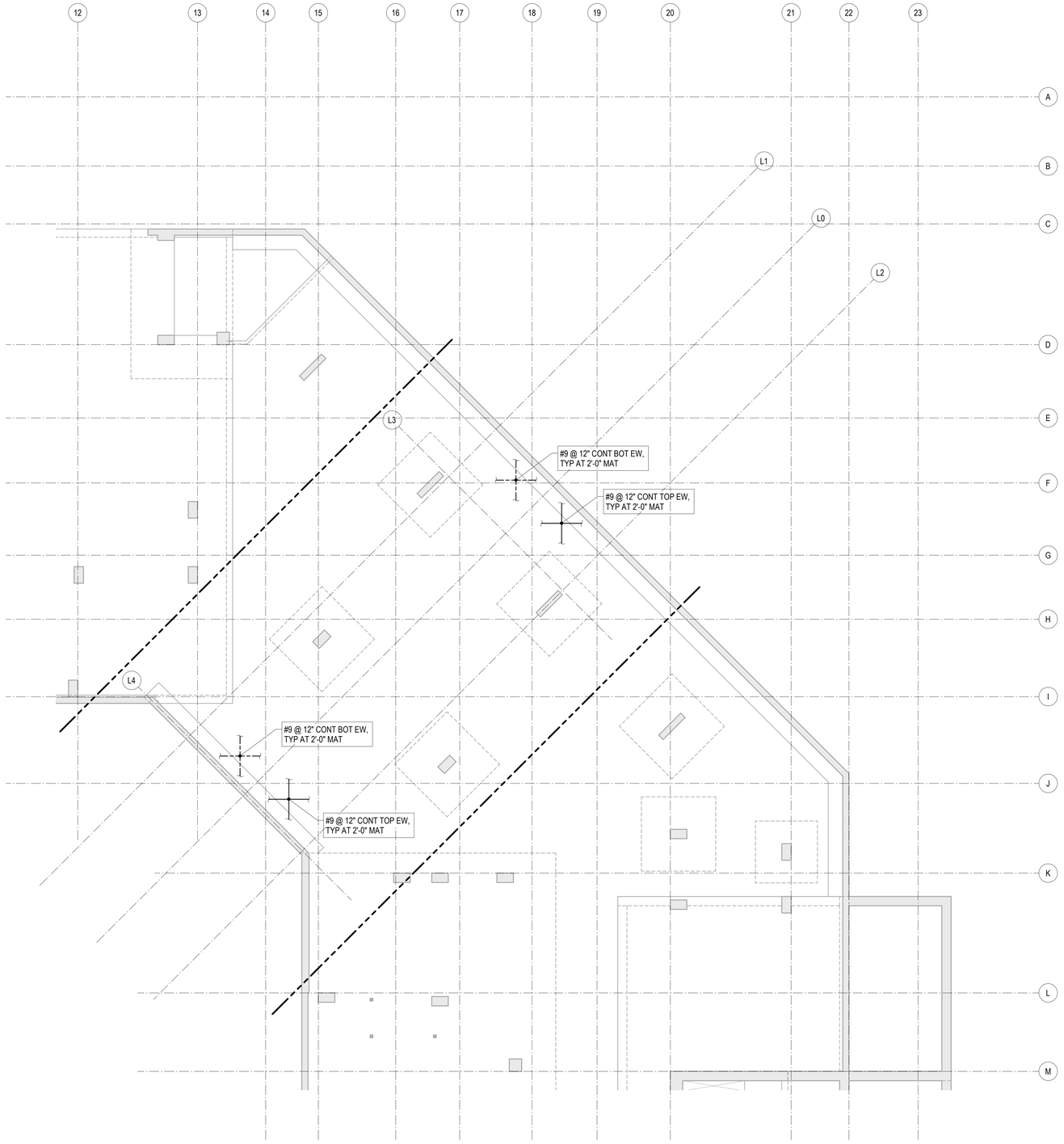
TOWER A & B
PARKING LEVEL 2
FRAMING PLAN

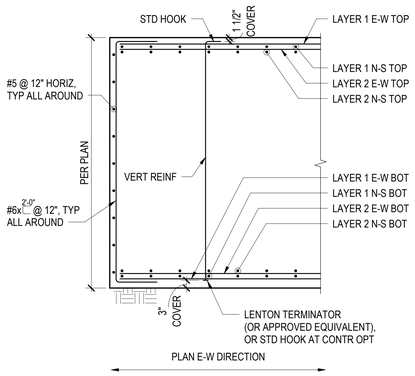
S2.AB.01



1 TOWER AB - PARKING LEVEL 2 LONGITUDINAL REINFORCING PLAN
1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.



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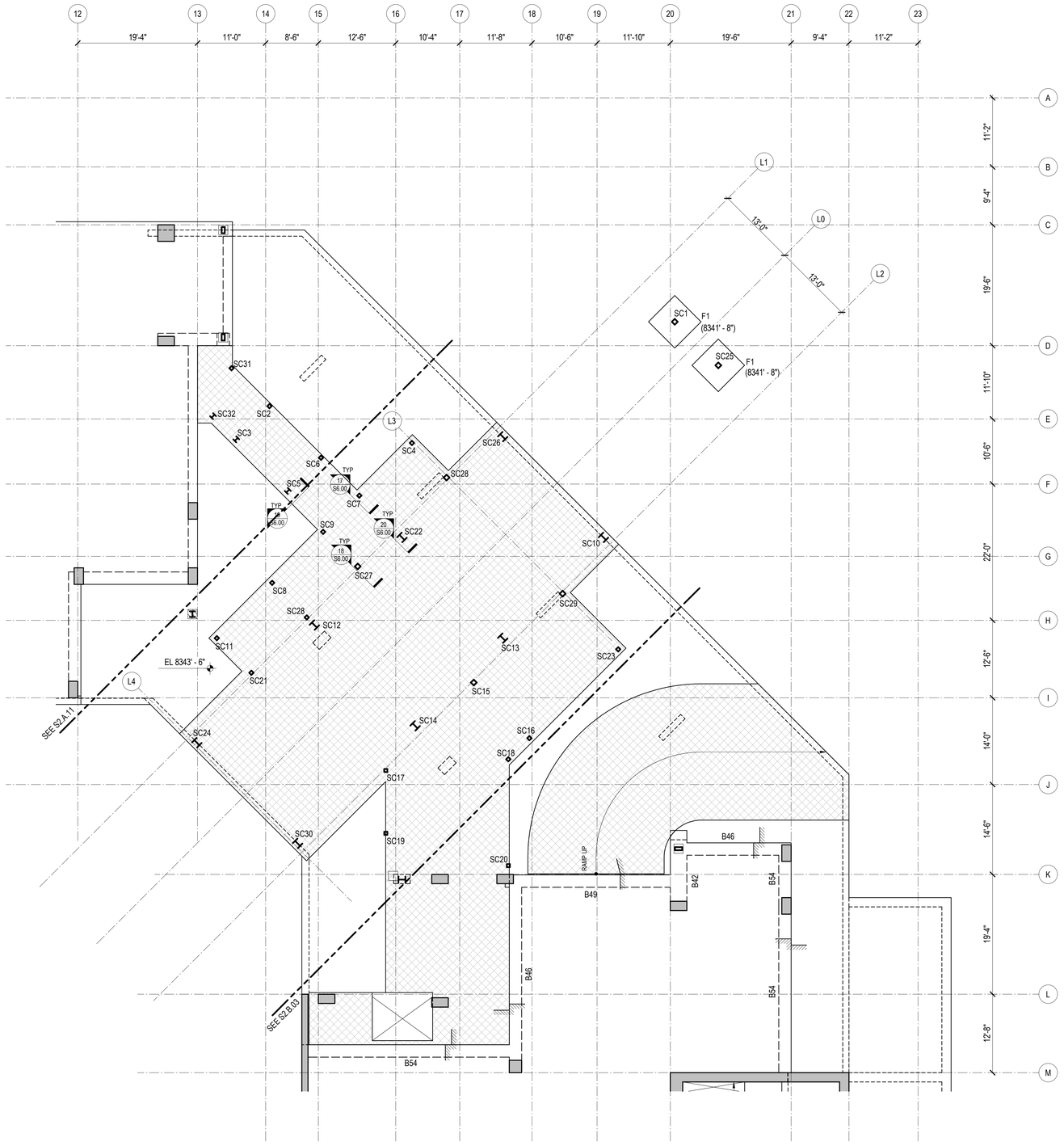
TOWER A & B
PARKING LEVEL 2
LONGITUDINAL
REINFORCING
PLAN

S2.AB.01.B



1 TOWER A & B - LEVEL 1 LOBBY FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
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S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8343'-6". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 14-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENT IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. [ ] INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.
9. ( ) INDICATES TOP OF FOOTING/ MAT FOUNDATION ELEVATION. ALL FOOTINGS SHALL BE PLACED AT TOP COMPACTED STRUCTURAL FILL IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
10. "FX" INDICATES FOOTING MARK. SEE FOOTING SCHEDULE FOR SIZE AND REINFORCEMENT.

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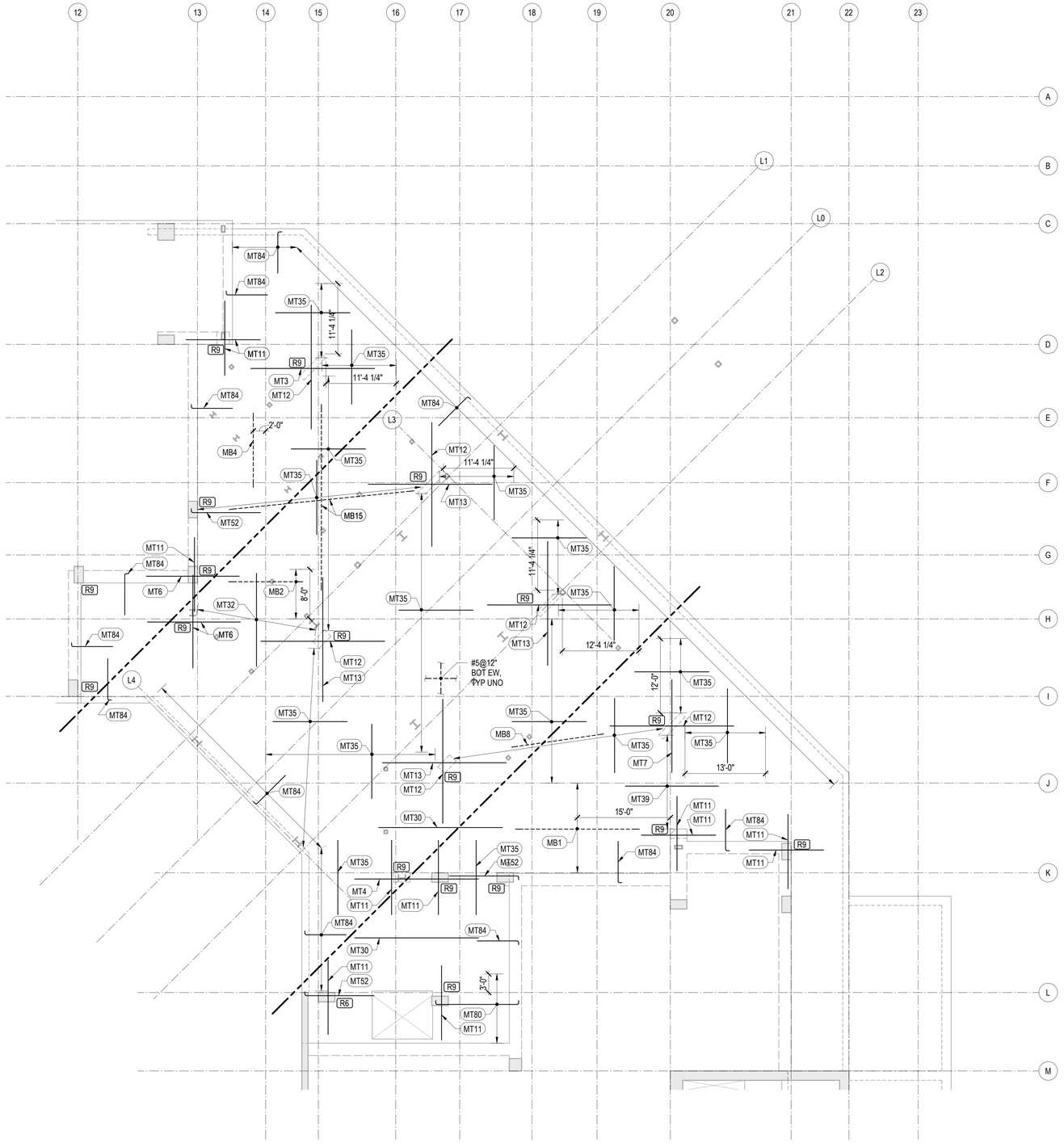
AB CONNECTOR
LEVEL 1 FRAMING PLAN

S2.AB.11



1 TOWER A & B - LEVEL 1 LOBBY FRAMING PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT3	(11) #7x20'-0" @ 12"	STAGGER 4'-0"
MT4	(11) #6x20'-0" @ 12"	STAGGER 4'-0"
MT5	(13) #5x15'-0" @ 10"	STAGGER 4'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x15'-0" @ 9"	STAGGER 3'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT11	(11) #5x12'-0" @ 12"	STAGGER 2'-0"
MT12	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT13	(21) #8x20'-0" @ 6"	STAGGER 5'-0"
MT14	(21) #7x20'-0" @ 6"	STAGGER 5'-0"
MT15	(11) #5x15'-0" @ 12"	STAGGER 3'-0"

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT16	(11) #4x12'-0" @ 12"	STAGGER 2'-0"
MT17	(11) #4x15'-0" @ 12"	STAGGER 3'-0"
MT18	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT31	#5x20'-0" @ 10"	STAGGER 2'-0"
MT32	#5x15'-0" @ 12"	STAGGER 2'-0"
MT33	#5x12'-0" @ 12"	STAGGER 2'-0"
MT34	#5x20'-0" @ 12"	STAGGER 4'-0"
MT35	#5x12'-0" @ 12"	STAGGER 1'-0"
MT36	#5x7'-6" @ 12"	STAGGER 0'-0"
MT37	#4x12'-0" @ 12"	STAGGER 1'-0"
MT38	#4x15'-0" @ 12"	STAGGER 1'-0"
MT39	#5x15'-0" @ 8"	STAGGER 2'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT51	(11) #5x6'-8" @ 12"	HOOK AT END
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14-2" @ 12"	HOOK AT END
MT55	(16) #6x14'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT57	(6) #6x9'-0" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT59	(11) #5x29'-3" @ 12"	HOOK AT END
MT60	(16) #7x10'-10" @ 8"	HOOK AT END
MT61	(11) #5x14'-2" @ 12"	HOOK AT END
MT62	(11) #4x11'-5" @ 12"	HOOK AT END
MT63	(11) #4x14'-5" @ 12"	HOOK AT END
MT64	(11) #4x19'-4" @ 12"	HOOK AT END
MT65	(11) #4x6'-10" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT87	#6x11'-0" @ 12"	HOOK AT END
MT88	#4x14'-5" @ 12"	HOOK AT END
MT89	#4x6'-10" @ 12"	HOOK AT END
MT90	#4 @ 12"	HOOK BOTH ENDS
MT91	#4x9'-5" @ 12"	HOOK AT END,
MT92	#6x14'-0" @ 12"	HOOK AT END
MT93	#5x19'-3" @ 10"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB1	#5x20'-0" @ 12"	STAGGER 2'-0"
MB2	#5x12'-0" @ 24"	STAGGER 2'-0"
MB4	(3) #5x12'-0" @ 14"	STAGGER 2'-0"
MB5	#5x20'-0" @ 18"	STAGGER 2'-0"
MB6	#5x20'-0" @ 16"	STAGGER 2'-0"
MB7	(6) #5x15'-0" @ 16"	STAGGER 2'-0"
MB8	(11) #5x20'-0" @ 12"	STAGGER 3'-0"
MB9	(3) #5x15'-0" @ 24"	STAGGER 2'-0"
MB15	(6) #5x30'-0" @ 24"	STAGGER 3'-0"

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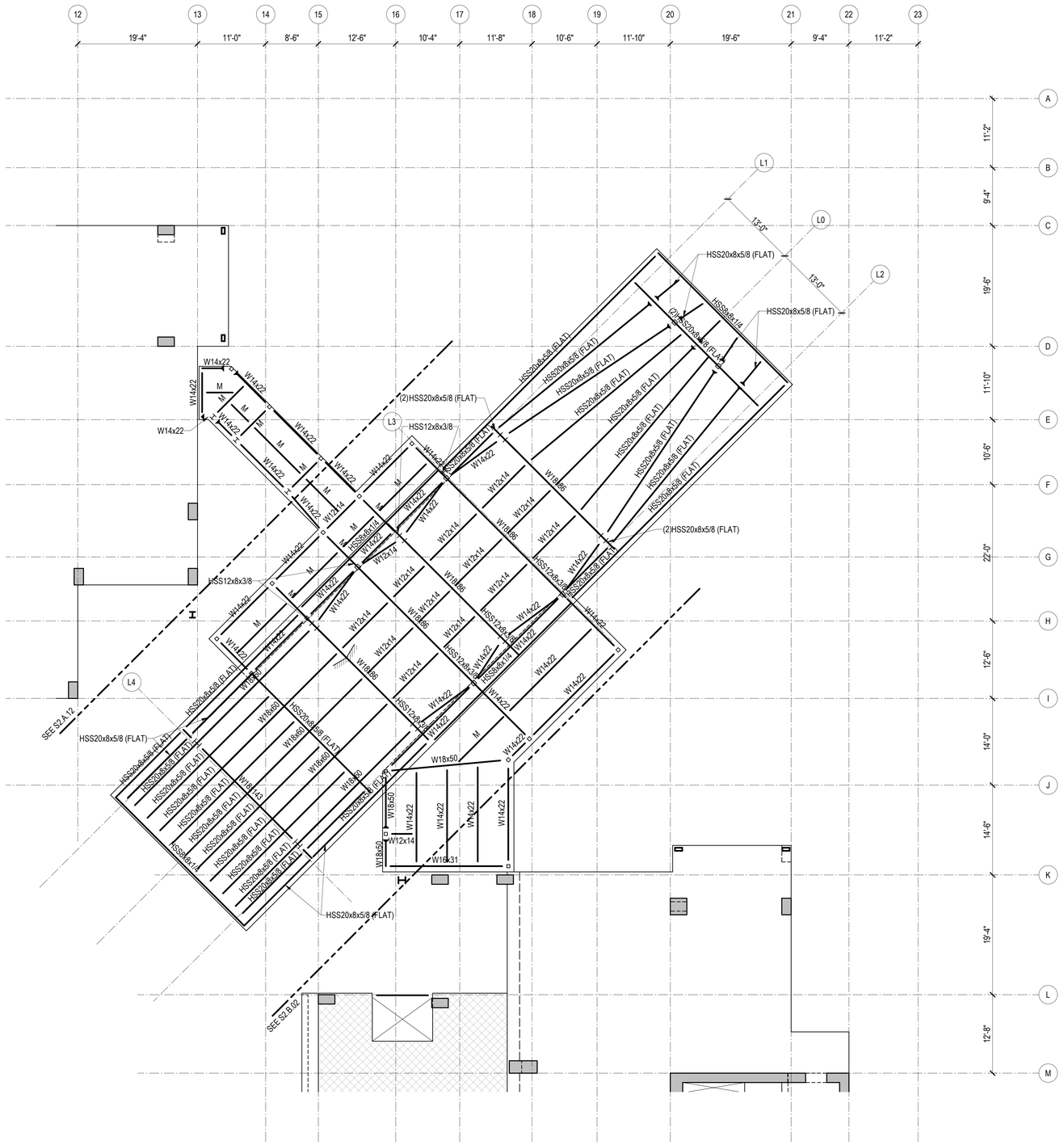
AB CONNECTOR
LEVEL 1
REINFORCING
PLAN

S2.AB.11.R



1 TOWER A LEVEL 2 & TOWER B LEVEL 1 COMPOSITE FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8357'-4 1/4". TOP OF SLAB ON STEEL DECK IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.

2. ROOF DECK IS MINIMUM 3 INCH X 20 GAUGE STEEL DECK. DECKING IS TO BE INSTALLED IN MINIMUM TWO SPAN CONDITION WHERE POSSIBLE. DECKING CONNECTIONS ARE TO BE AS FOLLOWS:

   SIDE LAP SEAM CONNECTIONS ARE 1-1/2 INCH TOP SEAM WELDS AT 18 INCHES MAXIMUM.

   CONNECTION TO SUPPORTING FRAMING SHALL BE A 24/4 WELD PATTERN WITH 3/4 INCH PUDDLE WELDS.

   CONNECTION TO FRAMING BELOW AND IN LINE WITH FLUTE SHALL BE 3/4 INCH PUDDLE WELDS AT 18 INCHES MAXIMUM.

   BUTTON PUNCHING OF SIDE LAP CONNECTIONS IS NOT PERMITTED.

3. REFERENCE TOP OF STEEL IS AT THE BOTTOM OF SLAB ON STEEL DECK UNLESS NOTED OTHERWISE.

4. STEEL SLOPES UNIFORMLY BETWEEN GIVEN TOP OF STEEL ELEVATIONS. WHERE BEAMS OR BEAMS AND COLUMNS INTERSECT, MATCH TOP OF STEEL UNLESS NOTED OTHERWISE.

5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING SLABS AND WALLS.

6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE 'TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE,' 'TYPICAL ROOF DECK OPENINGS,' AND 'TYPICAL COMPOSITE DECK OPENINGS' DETAILS FOR OPENING PLACEMENT CRITERIA AND REINFORCING OR FRAMING REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.

7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

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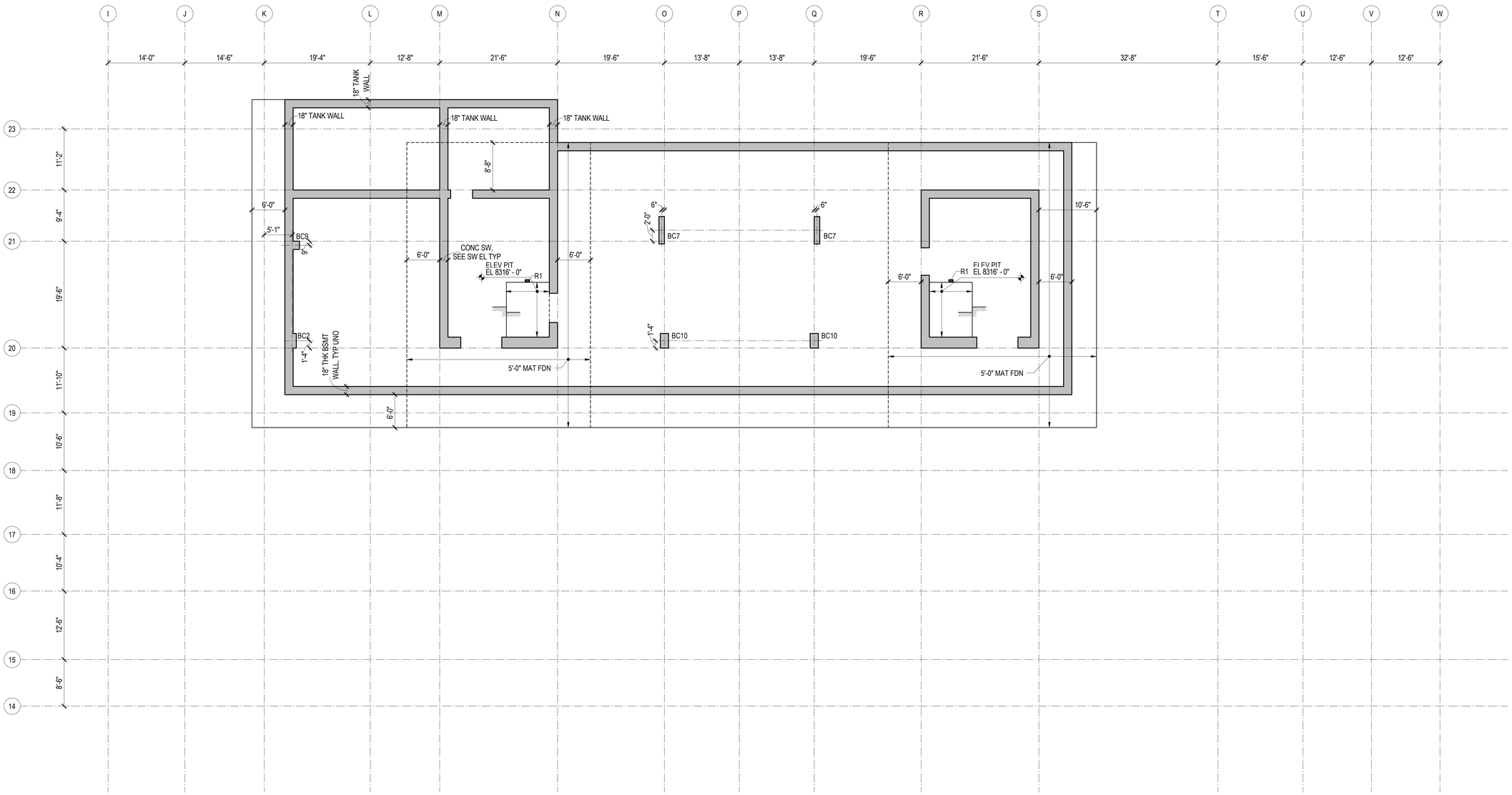
AB CONNECTOR
ROOF LEVEL
FRAMING PLAN

S2.AB.12



1 TOWER B - LEVEL B1 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE ELEVATION IS 8319' - 8". TOP OF MAT IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL SLAB EDGES, OPENINGS, SLOPES, AND DEPRESSIONS NOT DEFINED ON THE STRUCTURAL PLANS.
2. MAT FOUNDATION IS 3'-0" THICK UNLESS NOTED OTHERWISE. UPON REACHING THE MAT FOUNDATION SUBGRADE ELEVATION, SOIL CONDITIONS SHALL BE EVALUATED AND APPROVED BY THE GEOTECHNICAL ENGINEER OF RECORD.
3. SHEAR WALL OPENINGS, WALL ENDS, AND WALL LOCATIONS ARE DIMENSIONED RELATIVE TO GRID LINES ON THE SHEAR WALL ELEVATION.
4. BASEMENT WALLS SHALL BE RESTRAINED AT EACH FLOOR BY THE STRUCTURAL SLAB AND AT THE BOTTOM OF THE MAT, AND SHALL HAVE REACHED DESIGN STRENGTH PRIOR TO PLACING BACKFILL AND/OR DE-TENSIONING TIE-BACK ANCHORS.
5. BASEMENT WALLS ARE DESIGNED FOR A FULLY DRAINED CONDITION IN THE RETAINED SOIL.
6. BASEMENT WALL REINFORCEMENT IS SHOWN ON THE BASEMENT WALL ELEVATIONS.
7. THE STRUCTURAL SLAB IS A 10-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
8. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
9. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
10. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS PRIOR TO CASTING FOUNDATIONS.
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. PROVIDE DOCK LEVELER, ELEVATOR PITS, ESCALATOR PITS, SUMP PITS, M/E/P VAULTS, TRENCH AND AREA DRAINS, AND CONCRETE ENCASEMENTS/EMBEDMENTS/INSERTS/ETC. AS REQUIRED. REINFORCE PER TYPICAL DETAILS.
12. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR SIDEWALKS, PAVING, AND SITE DETAILS AT BUILDING EXTERIOR UNLESS NOTED OTHERWISE.
13. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.

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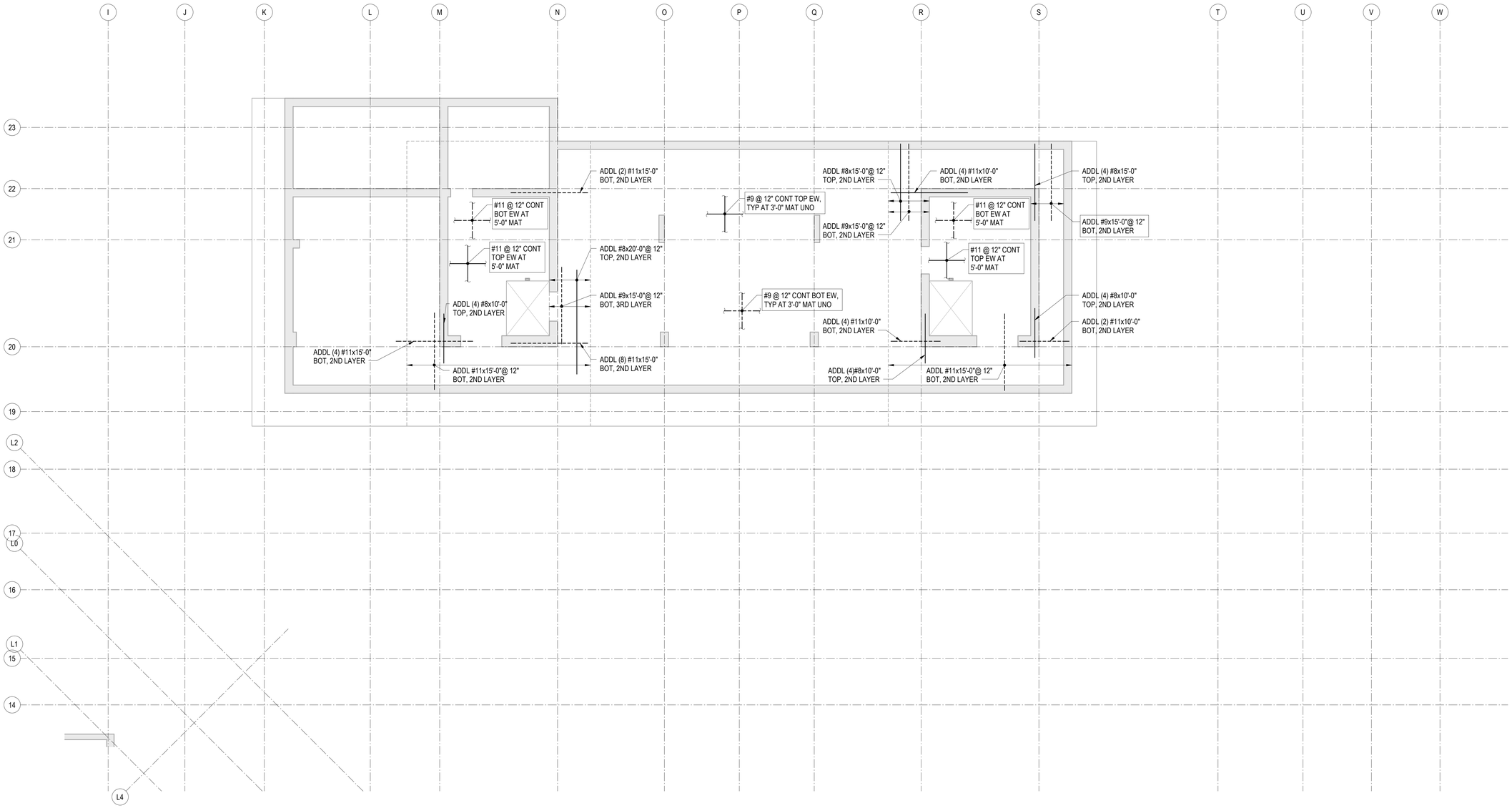
TOWER B LEVEL B1 FRAMING PLAN

S2.B.01



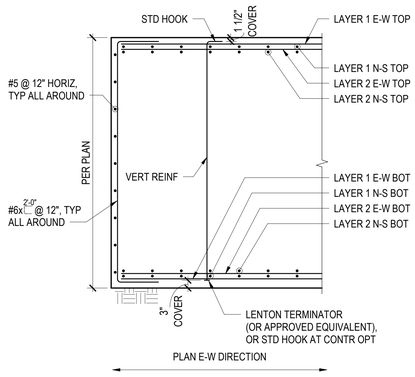
1 TOWER B - LEVEL B1 - LONGITUDINAL REINFORCEMENT PLAN

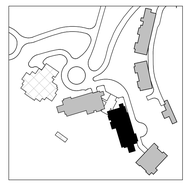
1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.

FOUNDATION VERTICAL REINFORCING SCHEDULE		
TYPE	REINFORCING	REMARKS
A	#7 @ 24" EACH WAY	
B	#8 @ 24" EACH WAY	
C	#9 @ 24" EACH WAY	
D	#7 @ 24" EACH WAY	





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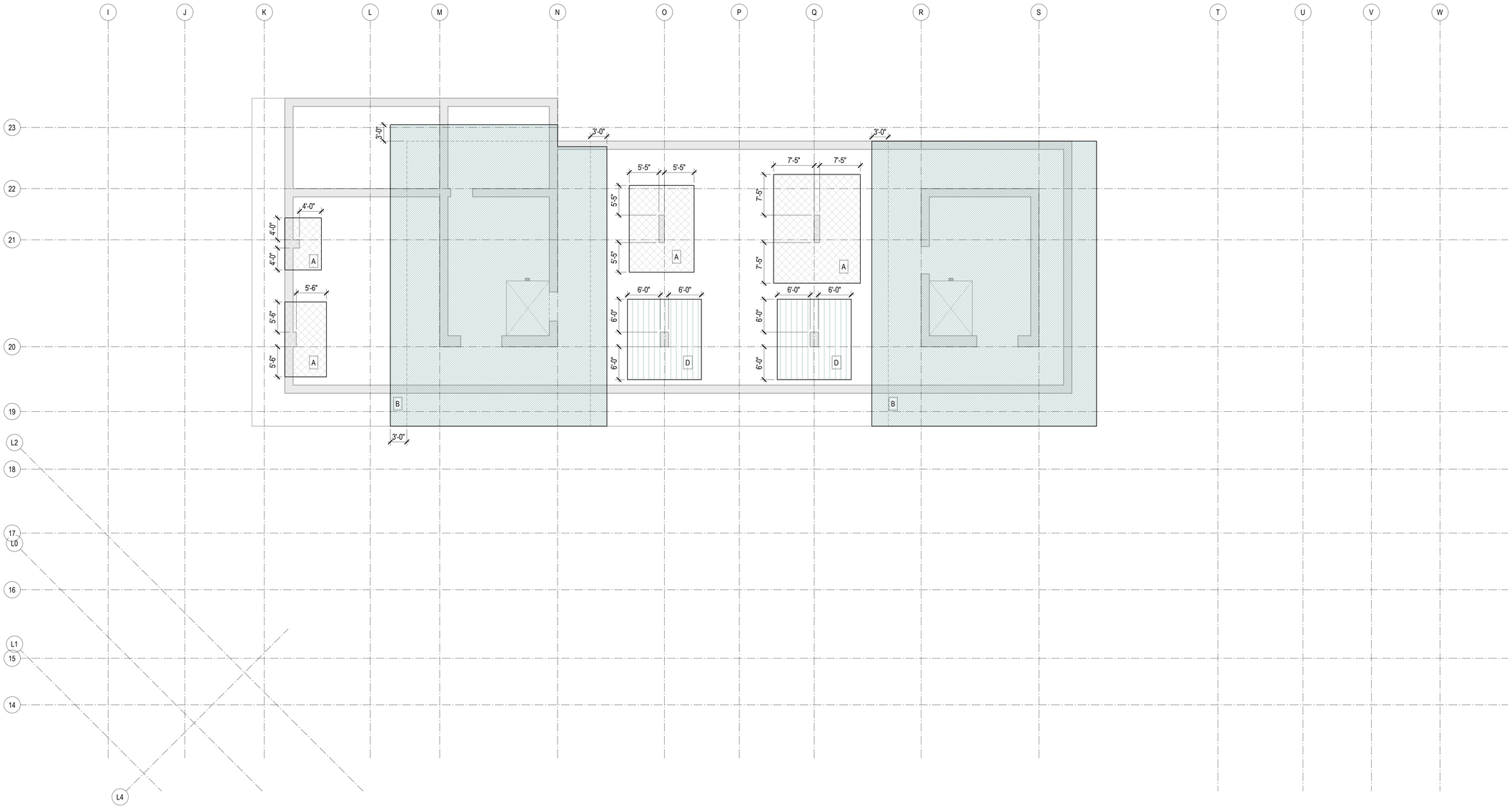
TOWER B LEVEL B1 LONGITUDINAL REINFORCING PLAN

S2.B.01.B



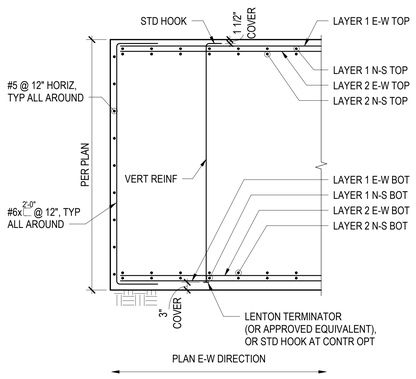
1 TOWER B - LEVEL B1 - SHEAR REINFORCEMENT PLAN

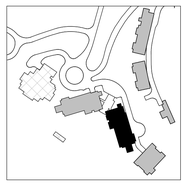
1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.

FOUNDATION VERTICAL REINFORCING SCHEDULE		
TYPE	REINFORCING	REMARKS
A	#7 @ 24" EACH WAY	
B	#8 @ 24" EACH WAY	
C	#9 @ 24" EACH WAY	
D	#7 @ 24" EACH WAY	





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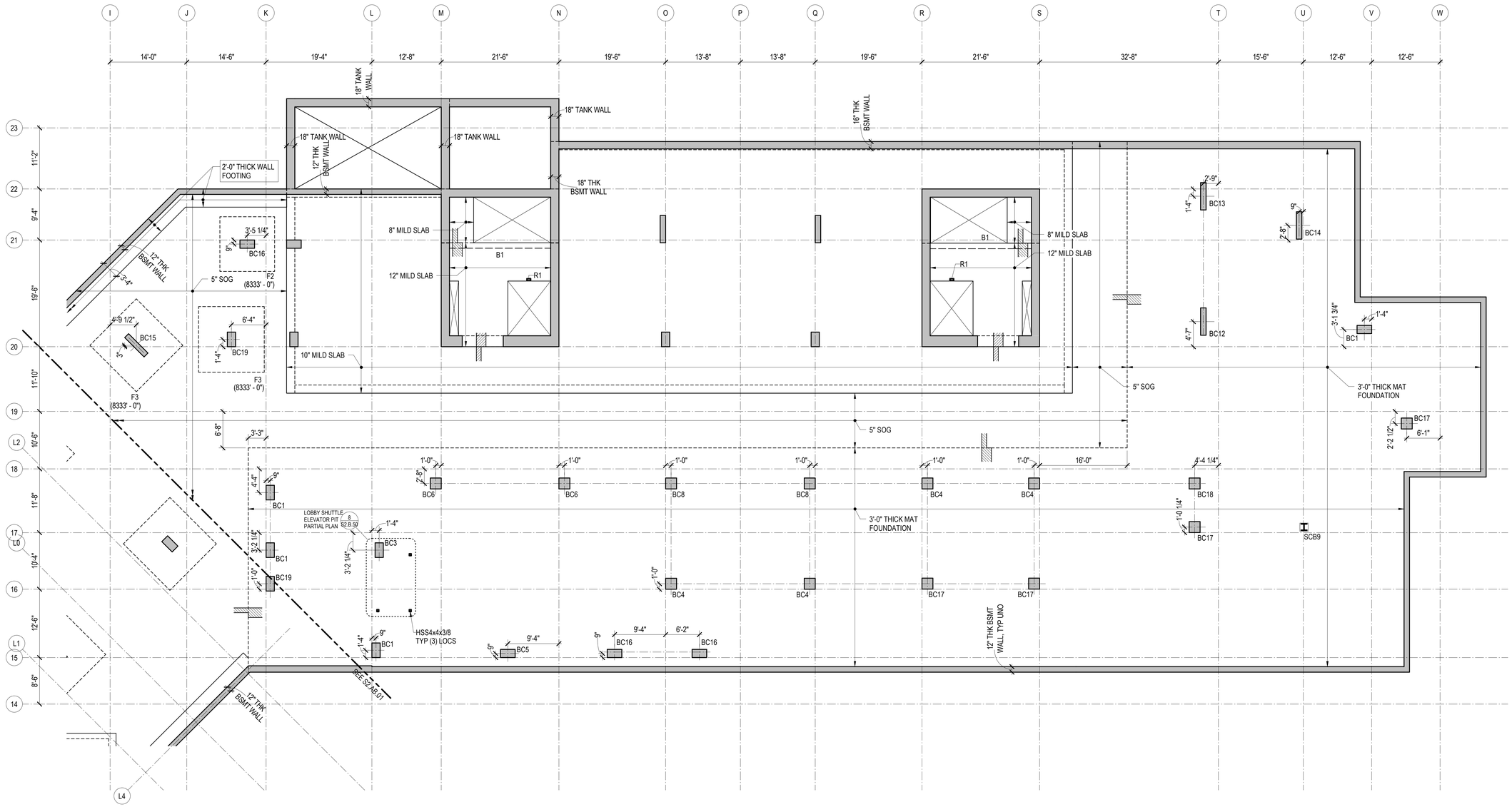
TOWER B LEVEL B1 SHEAR REINFORCING PLAN

S2.B.01.V



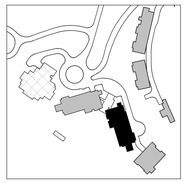
1 TOWER B - PARKING LEVEL 2 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE ELEVATION IS 8333' - 0". TOP OF MAT IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL SLAB EDGES, OPENINGS, SLOPES, AND DEPRESSIONS NOT DEFINED ON THE STRUCTURAL PLANS.
2. MAT FOUNDATION IS 3'-0" THICK UNLESS NOTED OTHERWISE. UPON REACHING THE MAT FOUNDATION SUBGRADE ELEVATION, SOIL CONDITIONS SHALL BE EVALUATED AND APPROVED BY THE GEOTECHNICAL ENGINEER OF RECORD.
3. SHEAR WALL OPENINGS, WALL ENDS, AND WALL LOCATIONS ARE DIMENSIONED RELATIVE TO GRID LINES ON THE SHEAR WALL ELEVATION.
4. BASEMENT WALLS SHALL BE RESTRAINED AT EACH FLOOR BY THE STRUCTURAL SLAB AND AT THE BOTTOM OF THE MAT, AND SHALL HAVE REACHED DESIGN STRENGTH PRIOR TO PLACING BACKFILL AND/OR DE-TENSIONING TIE-BACK ANCHORS.
5. BASEMENT WALLS ARE DESIGNED FOR A FULLY DRAINED CONDITION IN THE RETAINED SOIL.
6. BASEMENT WALL REINFORCEMENT IS SHOWN ON THE BASEMENT WALL ELEVATIONS.
7. THE STRUCTURAL SLAB IS A 10-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
8. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
9. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
10. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS PRIOR TO CASTING FOUNDATIONS.
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. PROVIDE DOCK LEVELER, ELEVATOR PITS, ESCALATOR PITS, SUMP PITS, M/E/P VAULTS, TRENCH AND AREA DRAINS, AND CONCRETE ENCASEMENTS/EMBEDMENTS/INSERTS/ETC. AS REQUIRED. REINFORCE PER TYPICAL DETAILS.
12. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR SIDEWALKS, PAVING, AND SITE DETAILS AT BUILDING EXTERIOR UNLESS NOTED OTHERWISE.
13. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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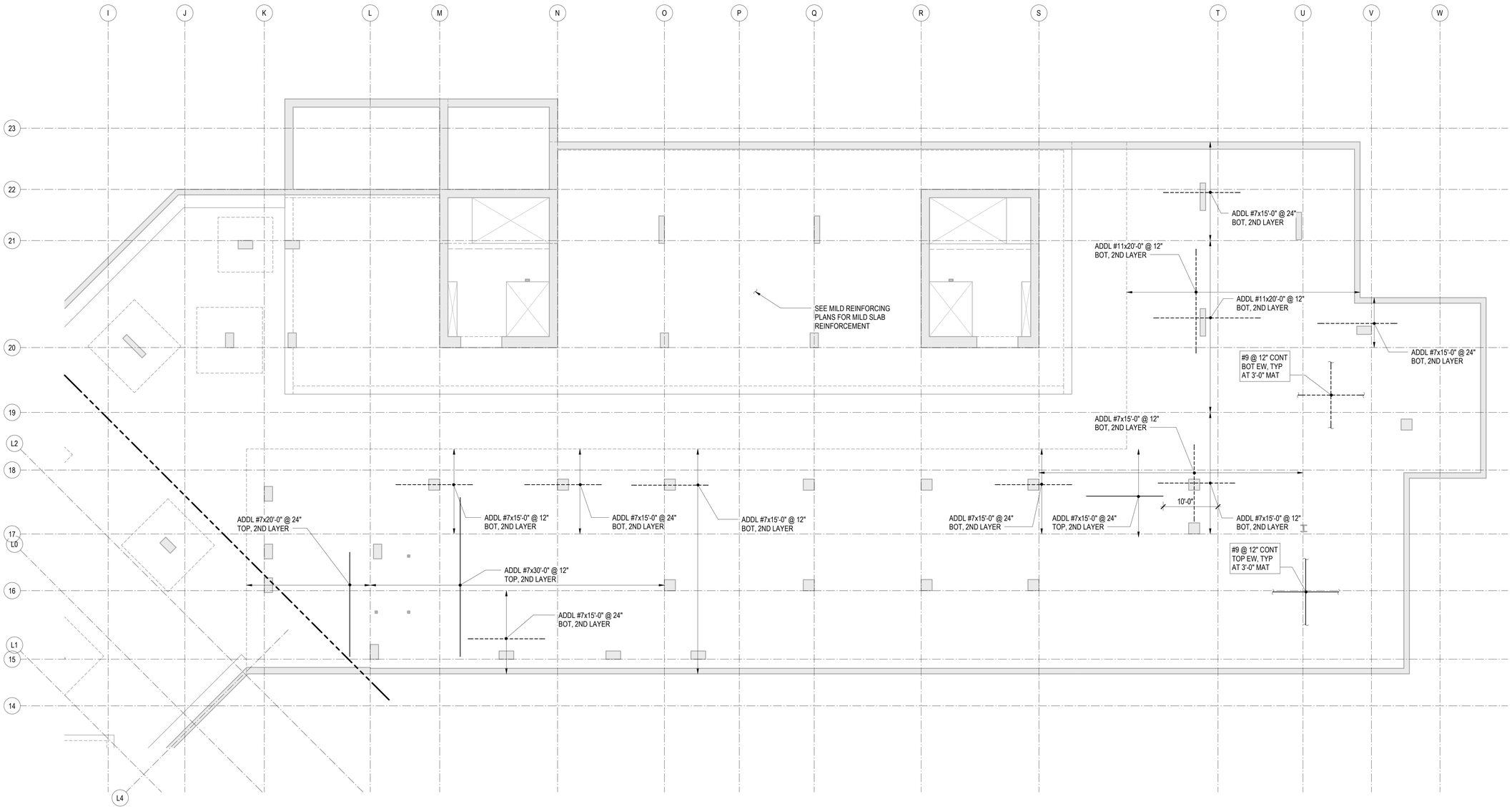
TOWER B LEVEL P2 FRAMING PLAN

S2.B.02

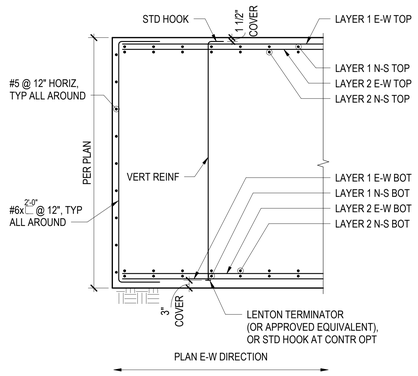


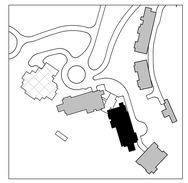
1 TOWER B - PARKING LEVEL 2 - MAT LONGITUDINAL REINFORCEMENT PLAN

1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.





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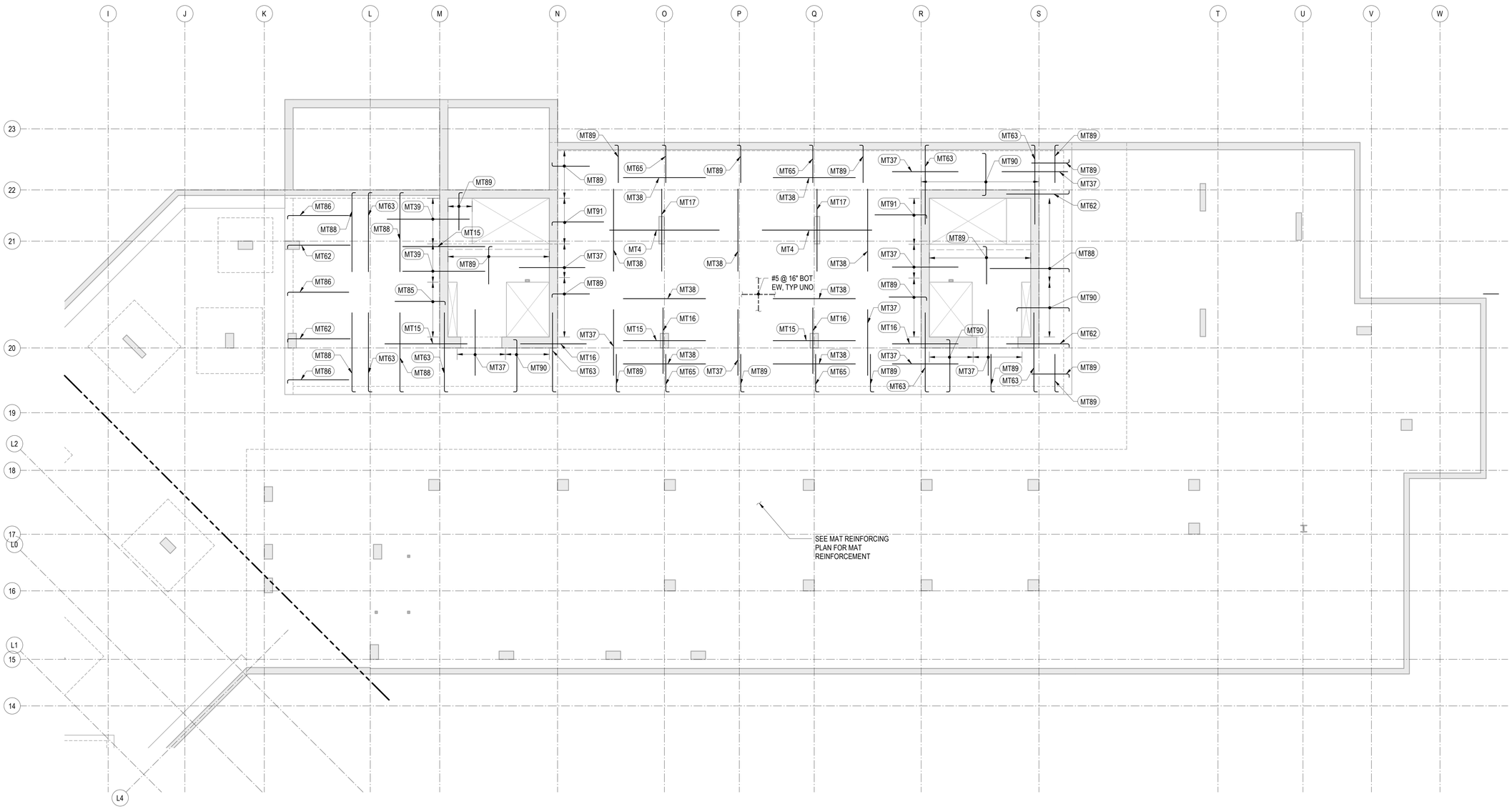
TOWER B LEVEL P2 MAT LONGITUDINAL REINFORCING PLAN

S2.B.02.B



1 TOWER B - PARKING LEVEL 2 - REINFORCEMENT PLAN
1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT3	(11) #7x20'-0" @ 12"	STAGGER 4'-0"
MT4	(11) #6x20'-0" @ 12"	STAGGER 4'-0"
MT5	(13) #5x15'-0" @ 10"	STAGGER 4'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x15'-0" @ 9"	STAGGER 3'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT11	(11) #5x12'-0" @ 12"	STAGGER 2'-0"
MT12	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT13	(21) #8x20'-0" @ 6"	STAGGER 5'-0"
MT14	(21) #7x20'-0" @ 6"	STAGGER 5'-0"
MT15	(11) #5x15'-0" @ 12"	STAGGER 3'-0"

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT16	(11) #4x12'-0" @ 12"	STAGGER 2'-0"
MT17	(11) #4x15'-0" @ 12"	STAGGER 3'-0"
MT18	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT31	#5x20'-0" @ 10"	STAGGER 2'-0"
MT32	#5x15'-0" @ 12"	STAGGER 2'-0"
MT33	#5x12'-0" @ 12"	STAGGER 2'-0"
MT34	#5x20'-0" @ 12"	STAGGER 4'-0"
MT35	#5x12'-0" @ 12"	STAGGER 1'-0"
MT36	#5x7'-6" @ 12"	STAGGER 0'-0"
MT37	#4x12'-0" @ 12"	STAGGER 1'-0"
MT38	#4x15'-0" @ 12"	STAGGER 1'-0"
MT39	#5x15'-0" @ 8"	STAGGER 2'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

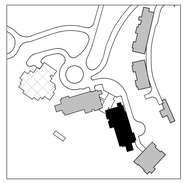
MARK	REINFORCING	REMARKS
MT51	(11) #5x6'-8" @ 12"	HOOK AT END
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14-2" @ 12"	HOOK AT END
MT55	(16) #6x14'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT57	(6) #6x9'-0" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT59	(11) #5x29'-3" @ 12"	HOOK AT END
MT60	(16) #7x10'-10" @ 8"	HOOK AT END
MT61	(11) #5x14'-2" @ 12"	HOOK AT END
MT62	(11) #4x11'-5" @ 12"	HOOK AT END
MT63	(11) #4x14'-5" @ 12"	HOOK AT END
MT64	(11) #4x19'-4" @ 12"	HOOK AT END
MT65	(11) #4x6'-10" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT87	#6x11'-0" @ 12"	HOOK AT END
MT88	#4x14'-5" @ 12"	HOOK AT END
MT89	#4x6'-10" @ 12"	HOOK AT END
MT90	#4 @ 12"	HOOK BOTH ENDS
MT91	#4x9'-5" @ 12"	HOOK AT END,
MT92	#6x14'-0" @ 12"	HOOK AT END
MT93	#5x19'-3" @ 10"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB1	#5x20'-0" @ 12"	STAGGER 2'-0"
MB2	#5x12'-0" @ 24"	STAGGER 2'-0"
MB4	(3) #5x12'-0" @ 14"	STAGGER 2'-0"
MB5	#5x20'-0" @ 18"	STAGGER 2'-0"
MB6	#5x20'-0" @ 16"	STAGGER 2'-0"
MB7	(6) #5x15'-0" @ 16"	STAGGER 2'-0"
MB8	(11) #5x20'-0" @ 12"	STAGGER 3'-0"
MB9	(3) #5x15'-0" @ 24"	STAGGER 2'-0"
MB15	(6) #5x30'-0" @ 24"	STAGGER 3'-0"



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TOWER B LEVEL P2
REINFORCING
PLAN

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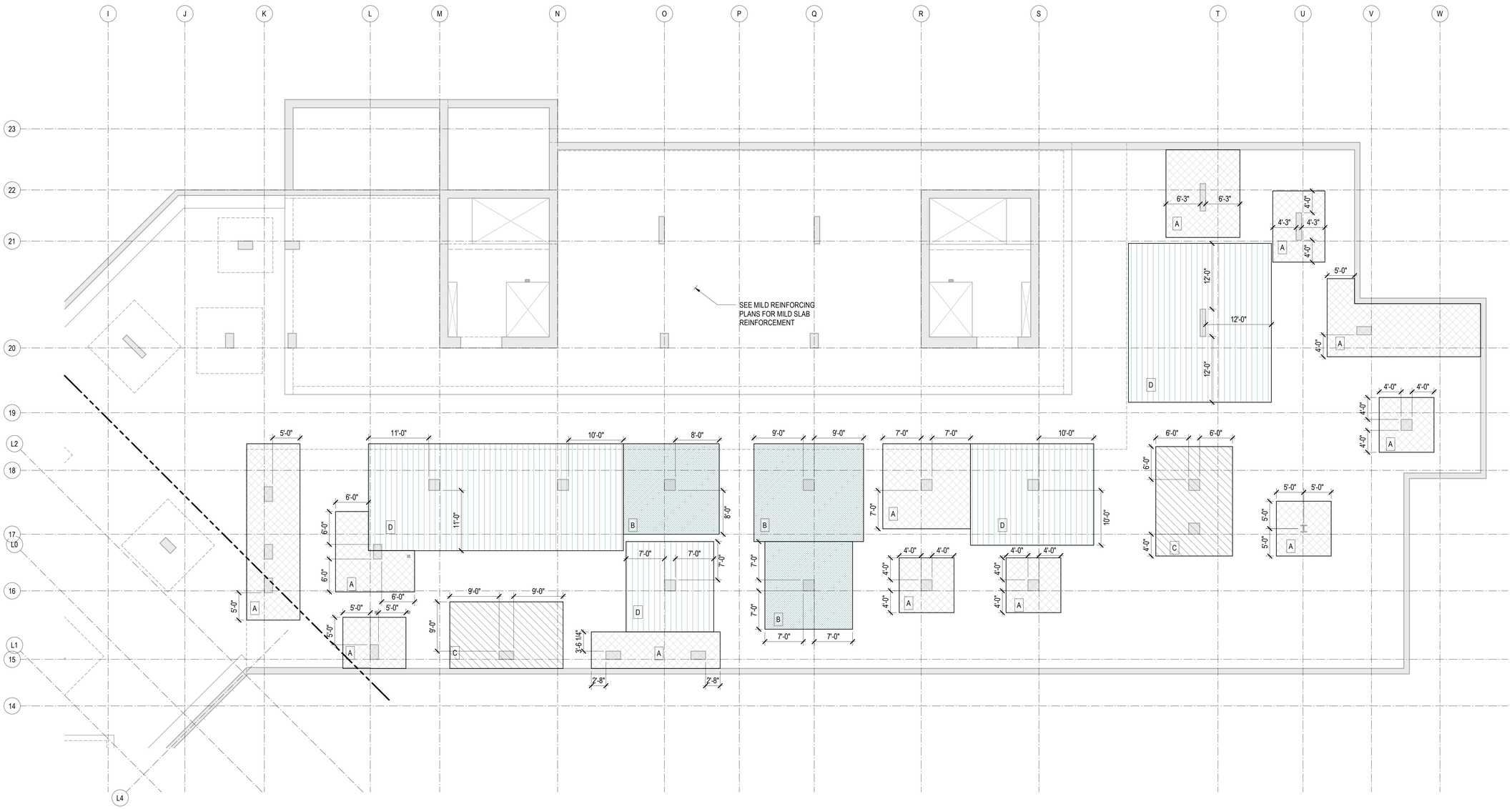


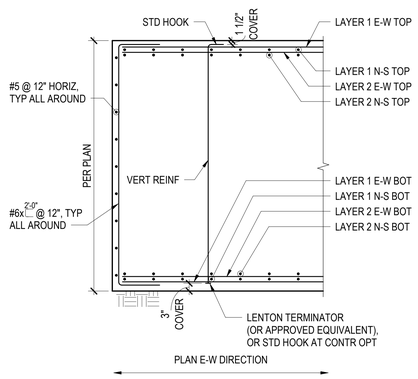
1 TOWER B - PARKING LEVEL 2 - MAT SHEAR REINFORCEMENT PLAN

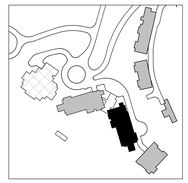
1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.

FOUNDATION VERTICAL REINFORCING SCHEDULE		
TYPE	REINFORCING	REMARKS
A	#7 @ 24" EACH WAY	1
B	#8 @ 24" EACH WAY	
C	#9 @ 24" EACH WAY	
D	#7 @ 24" EACH WAY	





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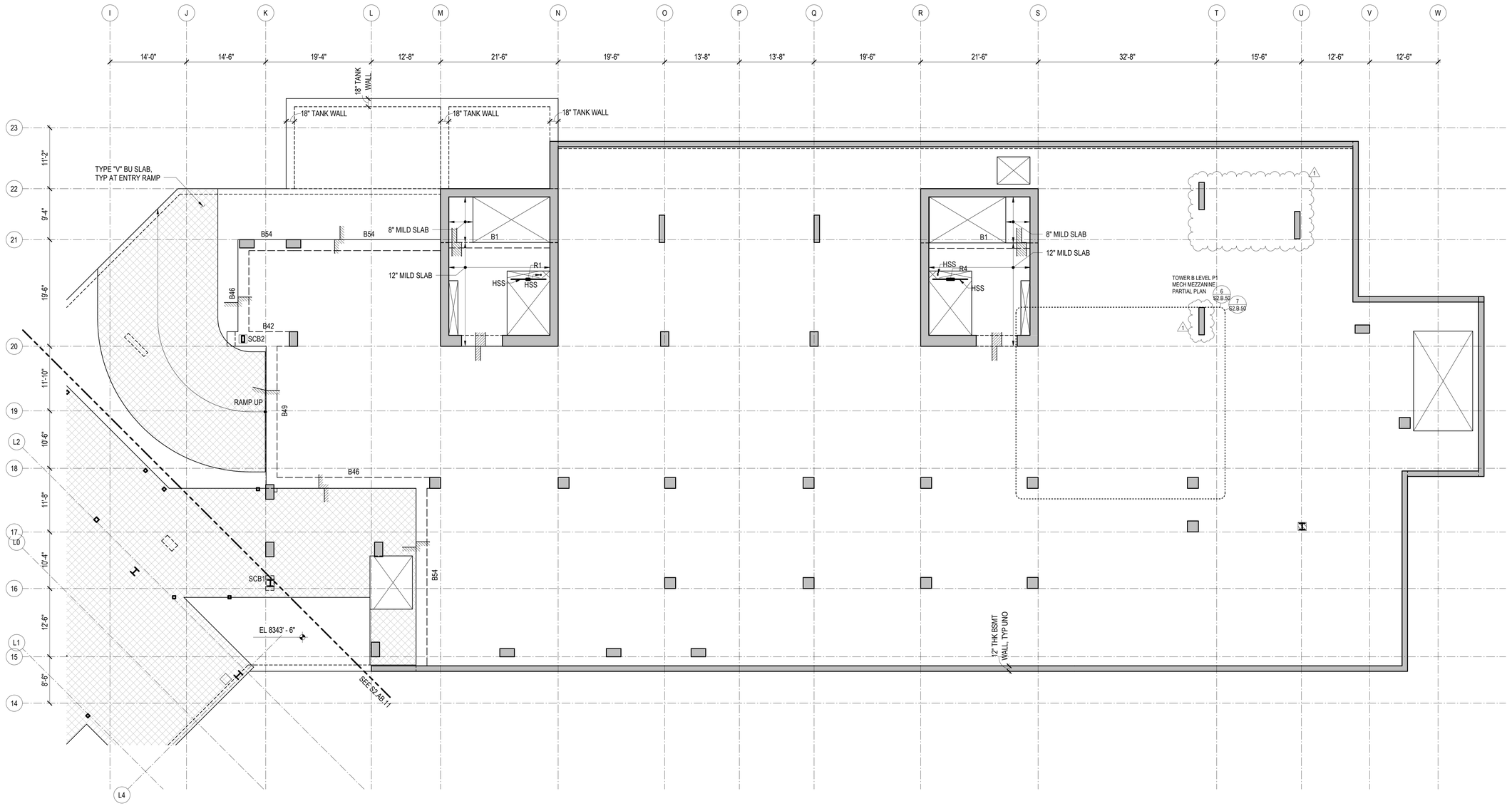
TOWER B LEVEL P2
MAT SHEAR
REINFORCING
PLAN

S2.B.02.V



1 TOWER B - PARKING LEVEL 1 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8345' - 0". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 14-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. [ ] INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.

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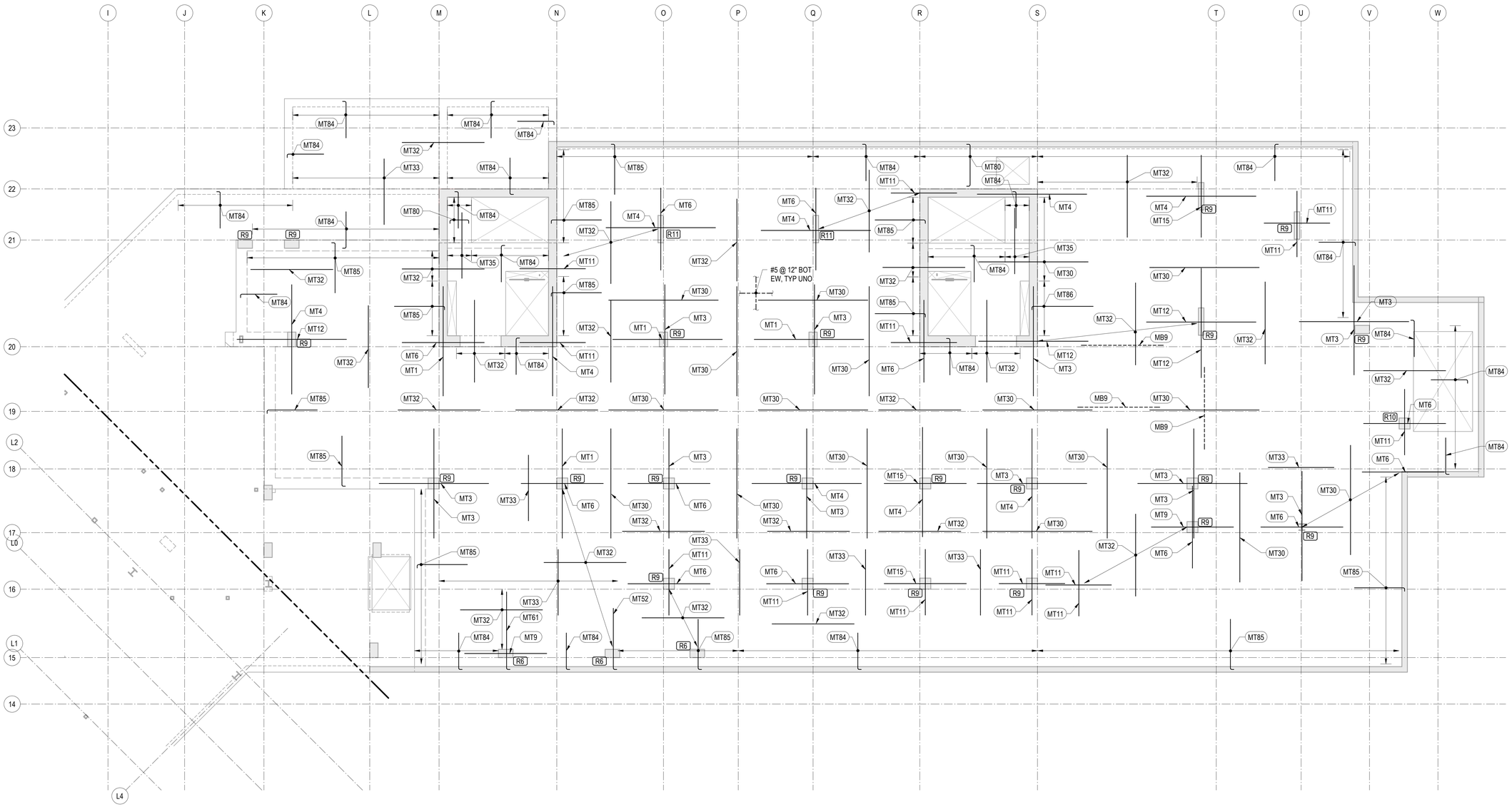
TOWER B LEVEL P1 FRAMING PLAN

S2.B.03



1 TOWER B - PARKING LEVEL 1 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT3	(11) #7x20'-0" @ 12"	STAGGER 4'-0"
MT4	(11) #6x20'-0" @ 12"	STAGGER 4'-0"
MT5	(13) #5x15'-0" @ 10"	STAGGER 4'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x15'-0" @ 9"	STAGGER 3'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT11	(11) #5x12'-0" @ 12"	STAGGER 2'-0"
MT12	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT13	(21) #8x20'-0" @ 6"	STAGGER 5'-0"
MT14	(21) #7x20'-0" @ 6"	STAGGER 5'-0"
MT15	(11) #5x15'-0" @ 12"	STAGGER 3'-0"

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT16	(11) #4x12'-0" @ 12"	STAGGER 2'-0"
MT17	(11) #4x15'-0" @ 12"	STAGGER 3'-0"
MT18	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT31	#5x20'-0" @ 10"	STAGGER 2'-0"
MT32	#5x15'-0" @ 12"	STAGGER 2'-0"
MT33	#5x12'-0" @ 12"	STAGGER 2'-0"
MT34	#5x20'-0" @ 12"	STAGGER 4'-0"
MT35	#5x12'-0" @ 12"	STAGGER 1'-0"
MT36	#5x7'-6" @ 12"	STAGGER 0'-0"
MT37	#4x12'-0" @ 12"	STAGGER 1'-0"
MT38	#4x15'-0" @ 12"	STAGGER 1'-0"
MT39	#5x15'-0" @ 8"	STAGGER 2'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT51	(11) #5x6'-8" @ 12"	HOOK AT END
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14-2" @ 12"	HOOK AT END
MT55	(16) #6x14'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT57	(6) #6x9'-0" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT59	(11) #5x29'-3" @ 12"	HOOK AT END
MT60	(16) #7x10'-10" @ 8"	HOOK AT END
MT61	(11) #5x14'-2" @ 12"	HOOK AT END
MT62	(11) #4x11'-5" @ 12"	HOOK AT END
MT63	(11) #4x14'-5" @ 12"	HOOK AT END
MT64	(11) #4x19'-4" @ 12"	HOOK AT END
MT65	(11) #4x6'-10" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT87	#6x11'-0" @ 12"	HOOK AT END
MT88	#4x14'-5" @ 12"	HOOK AT END
MT89	#4x6'-10" @ 12"	HOOK AT END
MT90	#4 @ 12"	HOOK BOTH ENDS
MT91	#4x9'-5" @ 12"	HOOK AT END,
MT92	#6x14'-0" @ 12"	HOOK AT END
MT93	#5x19'-3" @ 10"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB1	#5x20'-0" @ 12"	STAGGER 2'-0"
MB2	#5x12'-0" @ 24"	STAGGER 2'-0"
MB4	(3) #5x12'-0" @ 14"	STAGGER 2'-0"
MB5	#5x20'-0" @ 18"	STAGGER 2'-0"
MB6	#5x20'-0" @ 16"	STAGGER 2'-0"
MB7	(6) #5x15'-0" @ 16"	STAGGER 2'-0"
MB8	(11) #5x20'-0" @ 12"	STAGGER 3'-0"
MB9	(3) #5x15'-0" @ 24"	STAGGER 2'-0"
MB15	(6) #5x30'-0" @ 24"	STAGGER 3'-0"

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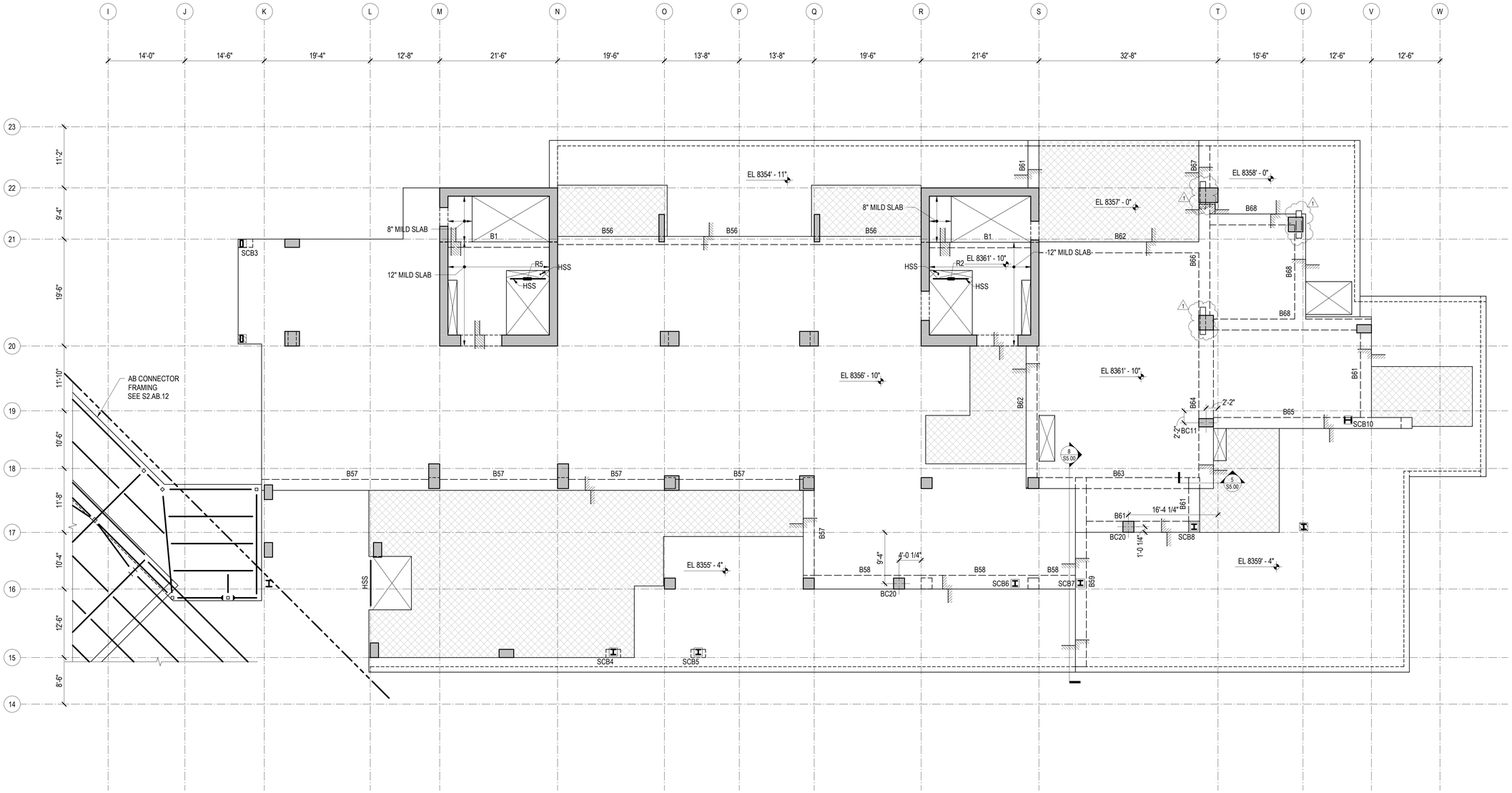
TOWER B LEVEL P1
REINFORCING
PLAN

S2.B.03.R



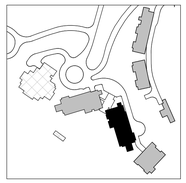
1 TOWER B - LEVEL 1 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8357' - 0". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 14-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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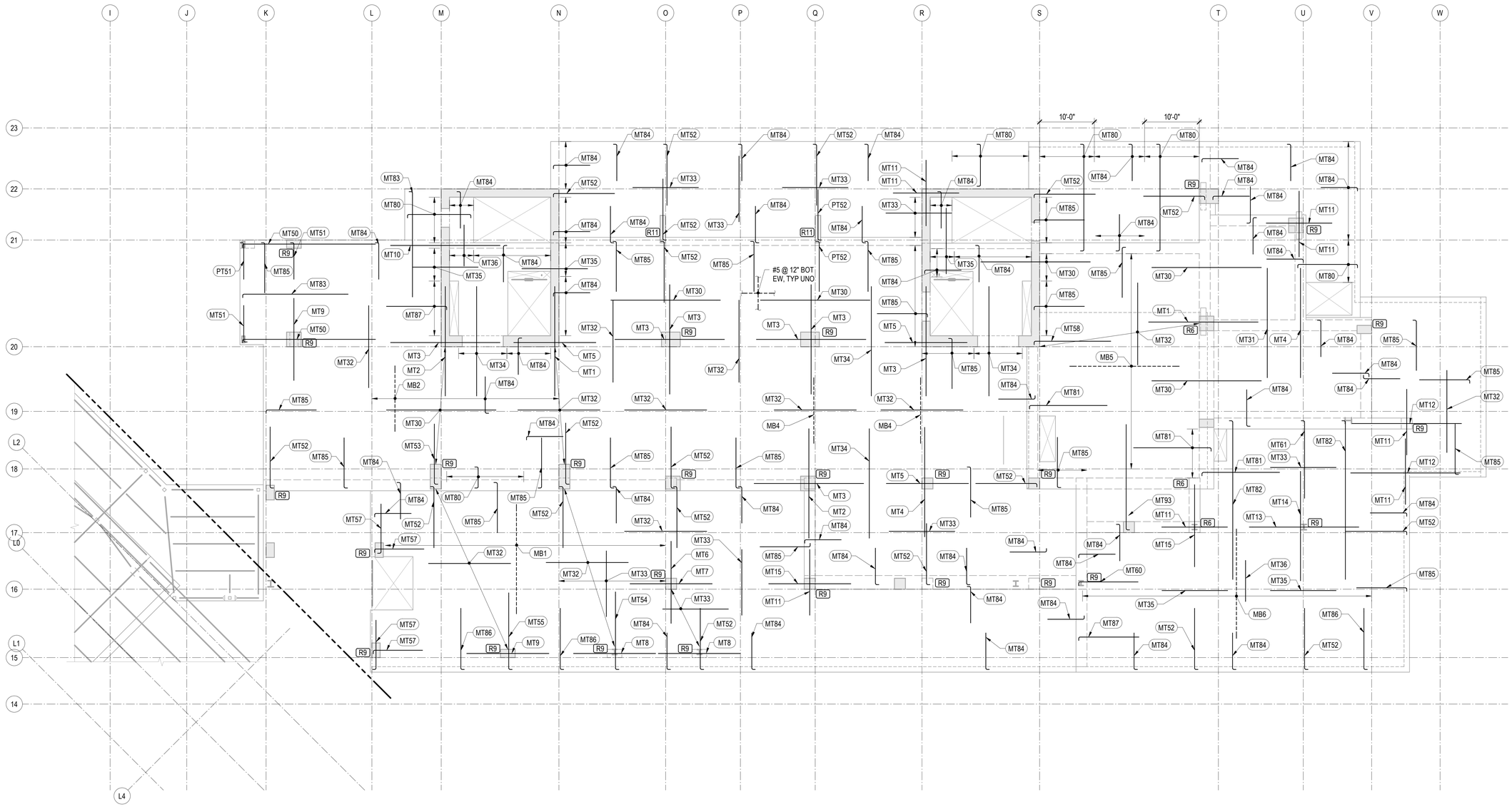
TOWER B LEVEL 1 FRAMING PLAN

S2.B.11



1 TOWER B - LEVEL 1 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT3	(11) #7x20'-0" @ 12"	STAGGER 4'-0"
MT4	(11) #6x20'-0" @ 12"	STAGGER 4'-0"
MT5	(13) #5x15'-0" @ 10"	STAGGER 4'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x15'-0" @ 9"	STAGGER 3'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT11	(11) #5x12'-0" @ 12"	STAGGER 2'-0"
MT12	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT13	(21) #8x20'-0" @ 6"	STAGGER 5'-0"
MT14	(21) #7x20'-0" @ 6"	STAGGER 5'-0"
MT15	(11) #5x15'-0" @ 12"	STAGGER 3'-0"

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT16	(11) #4x12'-0" @ 12"	STAGGER 2'-0"
MT17	(11) #4x15'-0" @ 12"	STAGGER 3'-0"
MT18	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT31	#5x20'-0" @ 10"	STAGGER 2'-0"
MT32	#5x15'-0" @ 12"	STAGGER 2'-0"
MT33	#5x12'-0" @ 12"	STAGGER 2'-0"
MT34	#5x20'-0" @ 12"	STAGGER 4'-0"
MT35	#5x12'-0" @ 12"	STAGGER 1'-0"
MT36	#5x7'-6" @ 12"	STAGGER 0'-0"
MT37	#4x12'-0" @ 12"	STAGGER 1'-0"
MT38	#4x15'-0" @ 12"	STAGGER 1'-0"
MT39	#5x15'-0" @ 8"	STAGGER 2'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

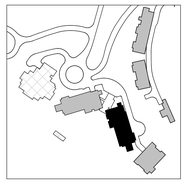
MARK	REINFORCING	REMARKS
MT51	(11) #5x6'-8" @ 12"	HOOK AT END
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14-2" @ 12"	HOOK AT END
MT55	(16) #6x14'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT57	(6) #6x9'-0" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT59	(11) #5x29'-3" @ 12"	HOOK AT END
MT60	(16) #7x10'-10" @ 8"	HOOK AT END
MT61	(11) #5x14'-2" @ 12"	HOOK AT END
MT62	(11) #4x11'-5" @ 12"	HOOK AT END
MT63	(11) #4x14'-5" @ 12"	HOOK AT END
MT64	(11) #4x19'-4" @ 12"	HOOK AT END
MT65	(11) #4x6'-10" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT87	#6x11'-0" @ 12"	HOOK AT END
MT88	#4x14'-5" @ 12"	HOOK AT END
MT89	#4x6'-10" @ 12"	HOOK AT END
MT90	#4 @ 12"	HOOK BOTH ENDS
MT91	#4x9'-5" @ 12"	HOOK AT END,
MT92	#6x14'-0" @ 12"	HOOK AT END
MT93	#5x19'-3" @ 10"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB1	#5x20'-0" @ 12"	STAGGER 2'-0"
MB2	#5x12'-0" @ 24"	STAGGER 2'-0"
MB4	(3) #5x12'-0" @ 14"	STAGGER 2'-0"
MB5	#5x20'-0" @ 18"	STAGGER 2'-0"
MB6	#5x20'-0" @ 16"	STAGGER 2'-0"
MB7	(6) #5x15'-0" @ 16"	STAGGER 2'-0"
MB8	(11) #5x20'-0" @ 12"	STAGGER 3'-0"
MB9	(3) #5x15'-0" @ 24"	STAGGER 2'-0"
MB15	(6) #5x30'-0" @ 24"	STAGGER 3'-0"



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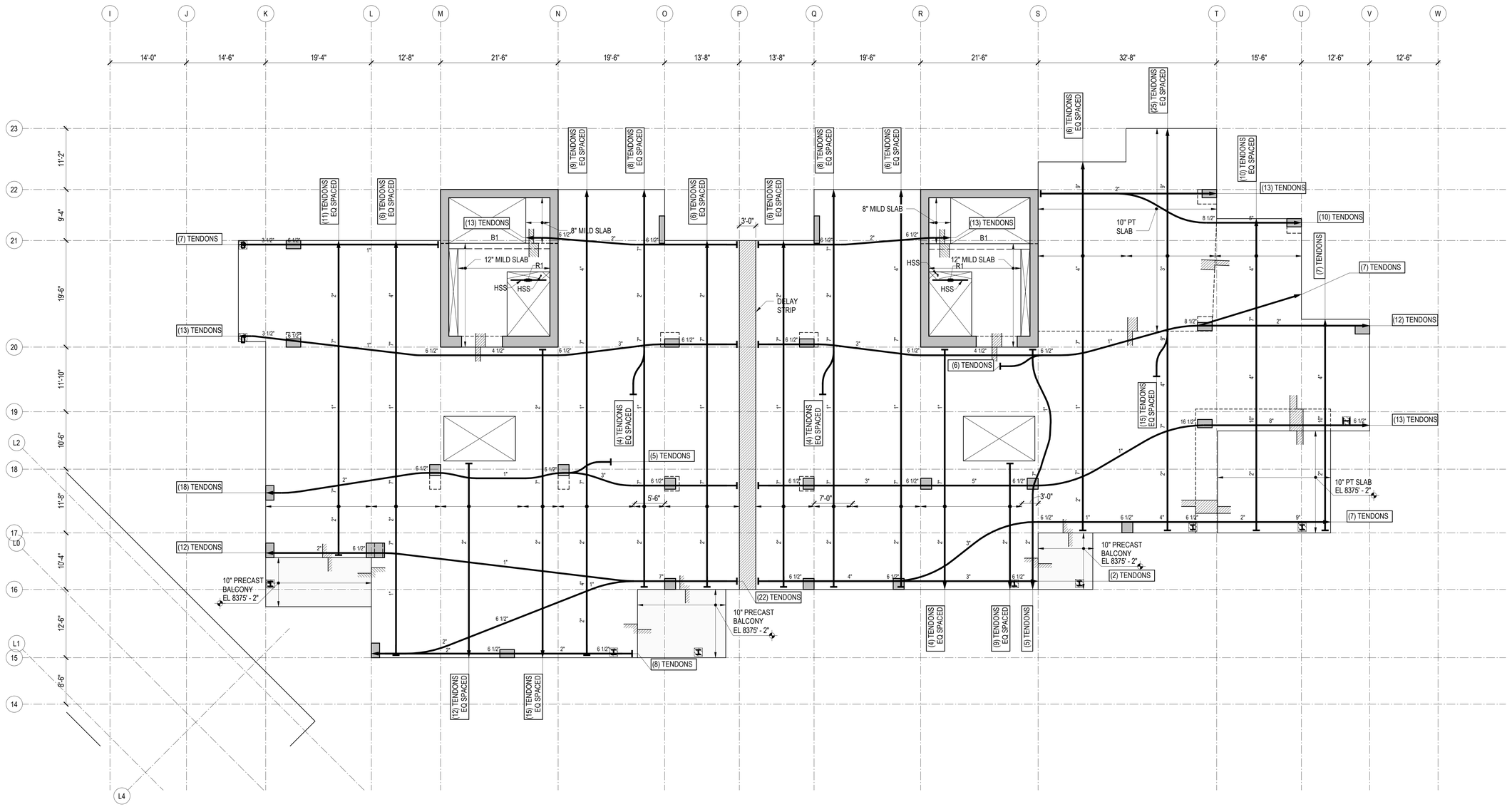
TOWER B LEVEL 1 REINFORCING PLAN

S2.B.11.R



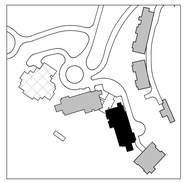
1 TOWER B - LEVEL 2 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8376' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8375' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.



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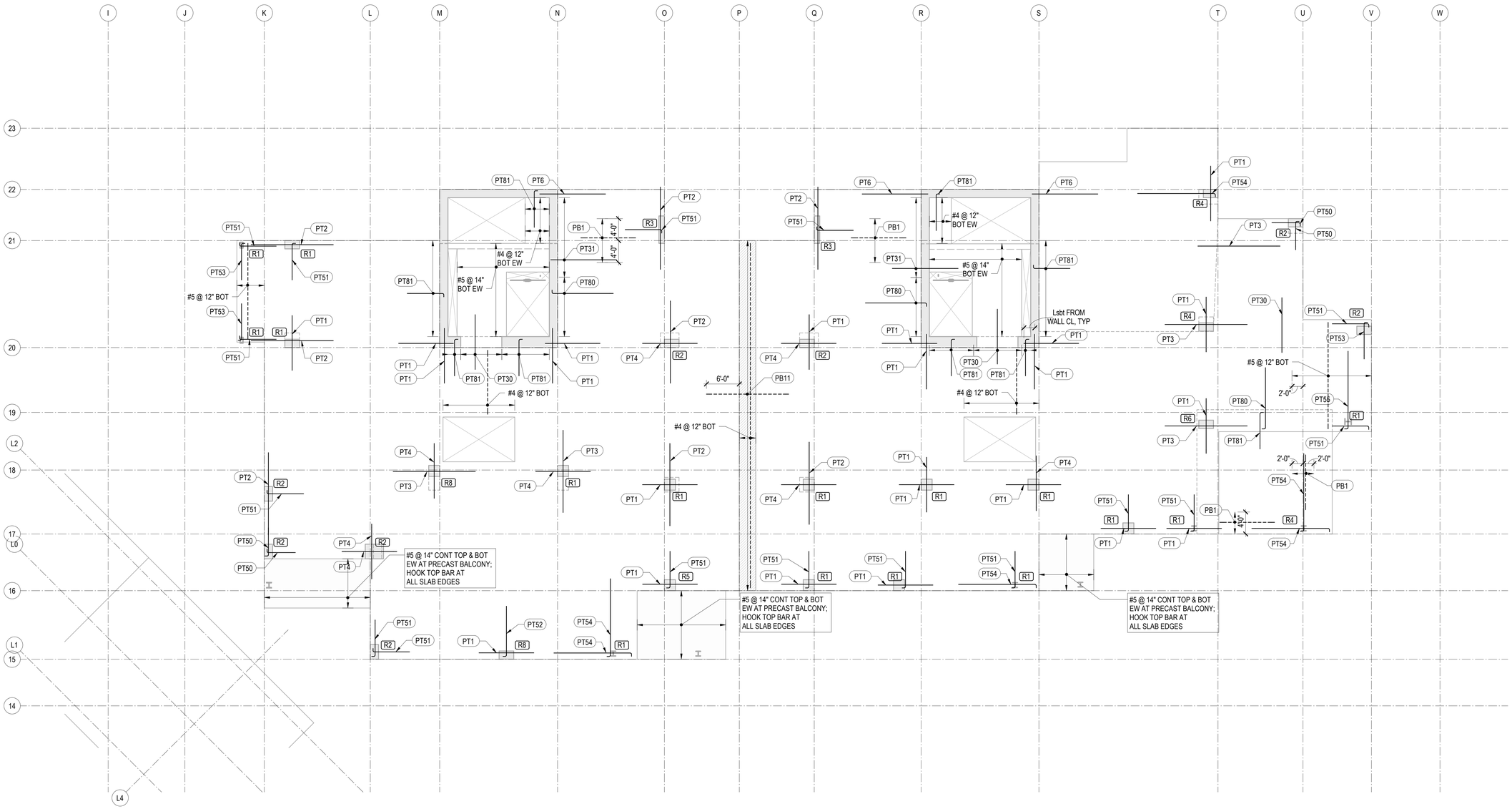
TOWER B LEVEL 2 FRAMING PLAN

S2.B.12



1 TOWER B - LEVEL 2 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

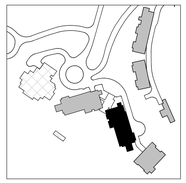
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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revisions:

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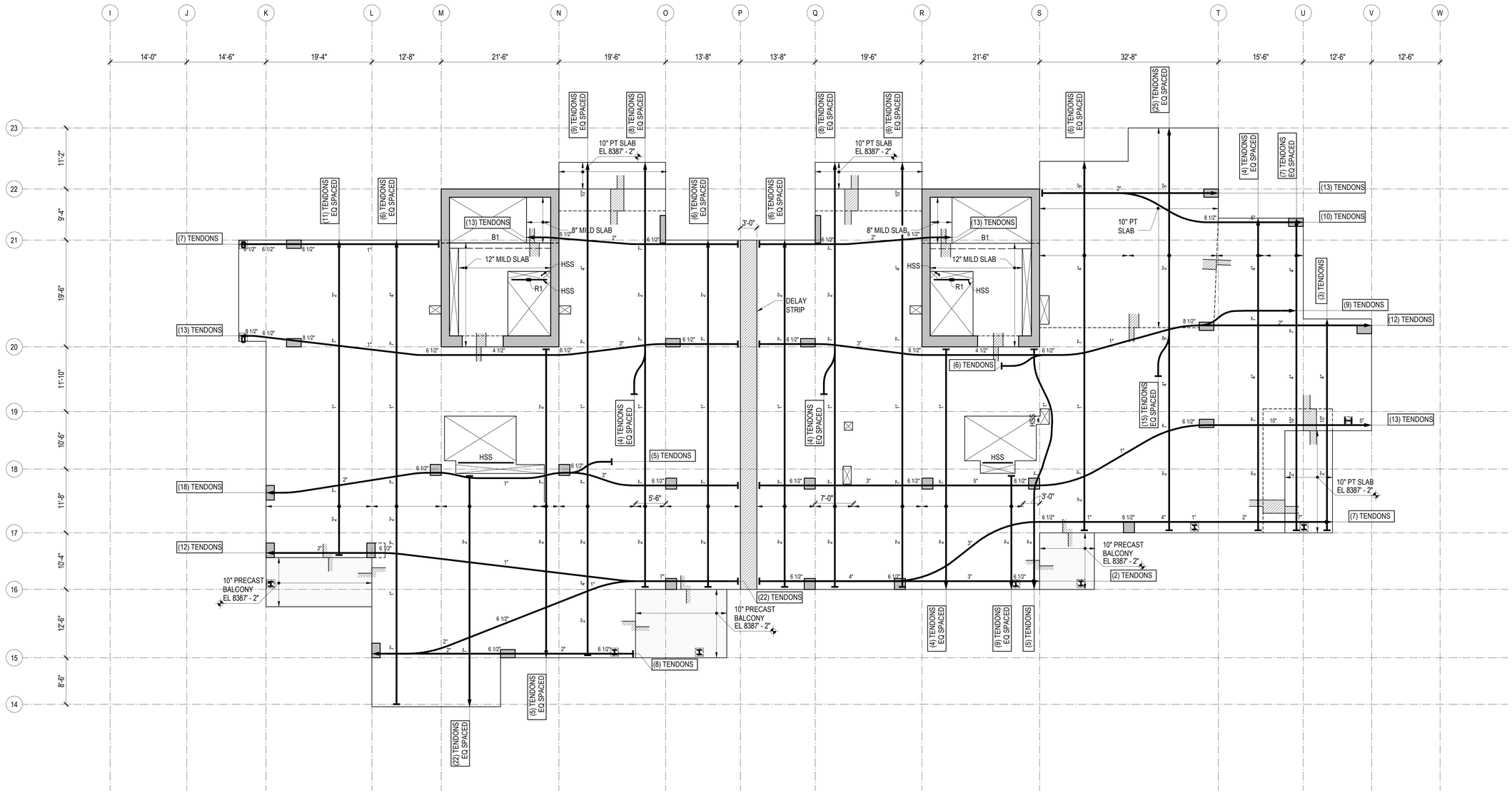
TOWER B LEVEL 2 REINFORCING PLAN

S2.B.12.R



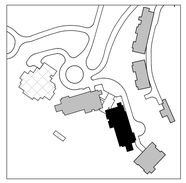
1 TOWER B - LEVEL 3 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8388' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8387' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.



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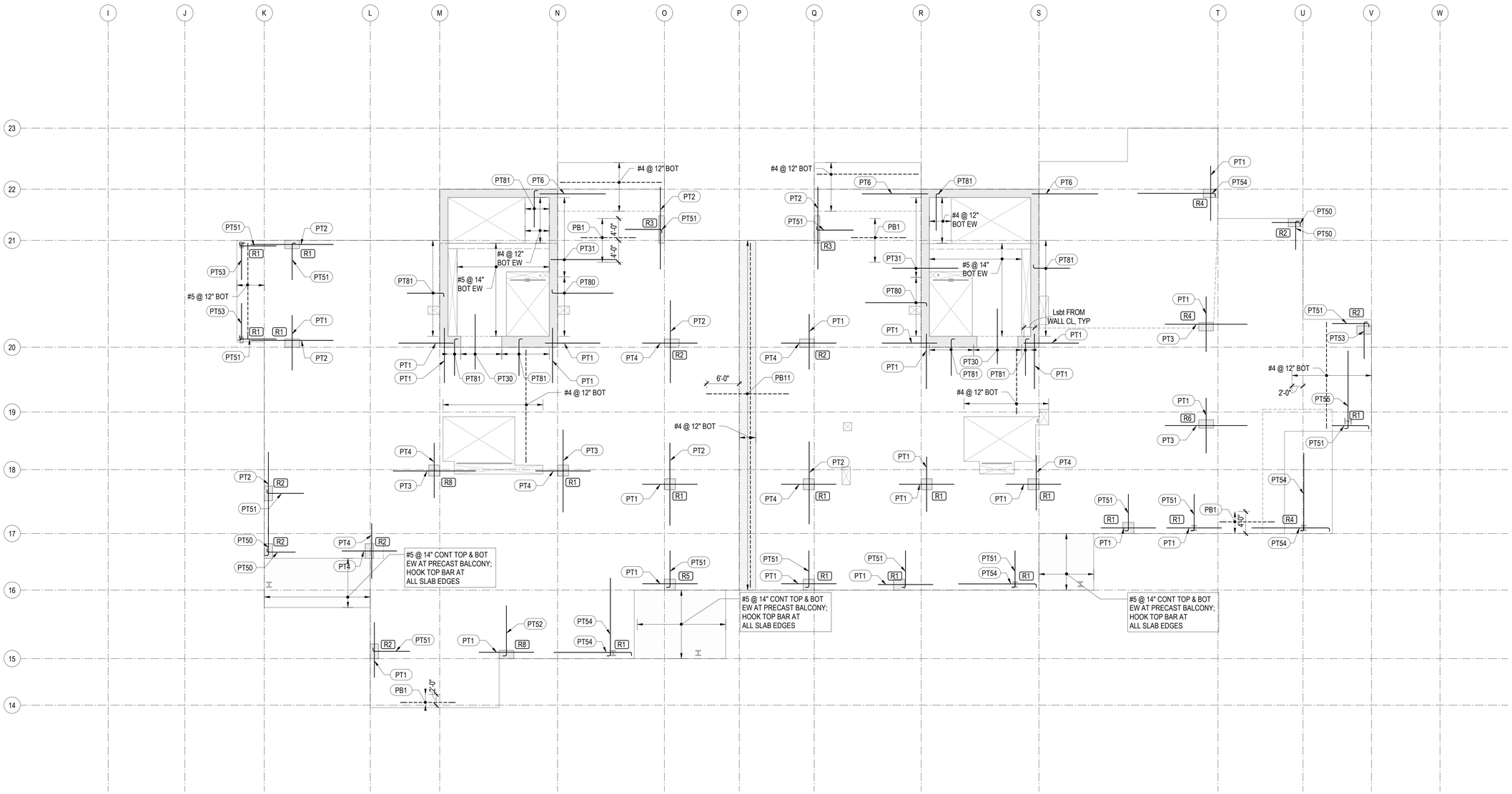
TOWER B LEVEL 3 FRAMING PLAN

S2.B.13



1 TOWER B - LEVEL 3 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

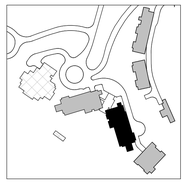
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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revisions:

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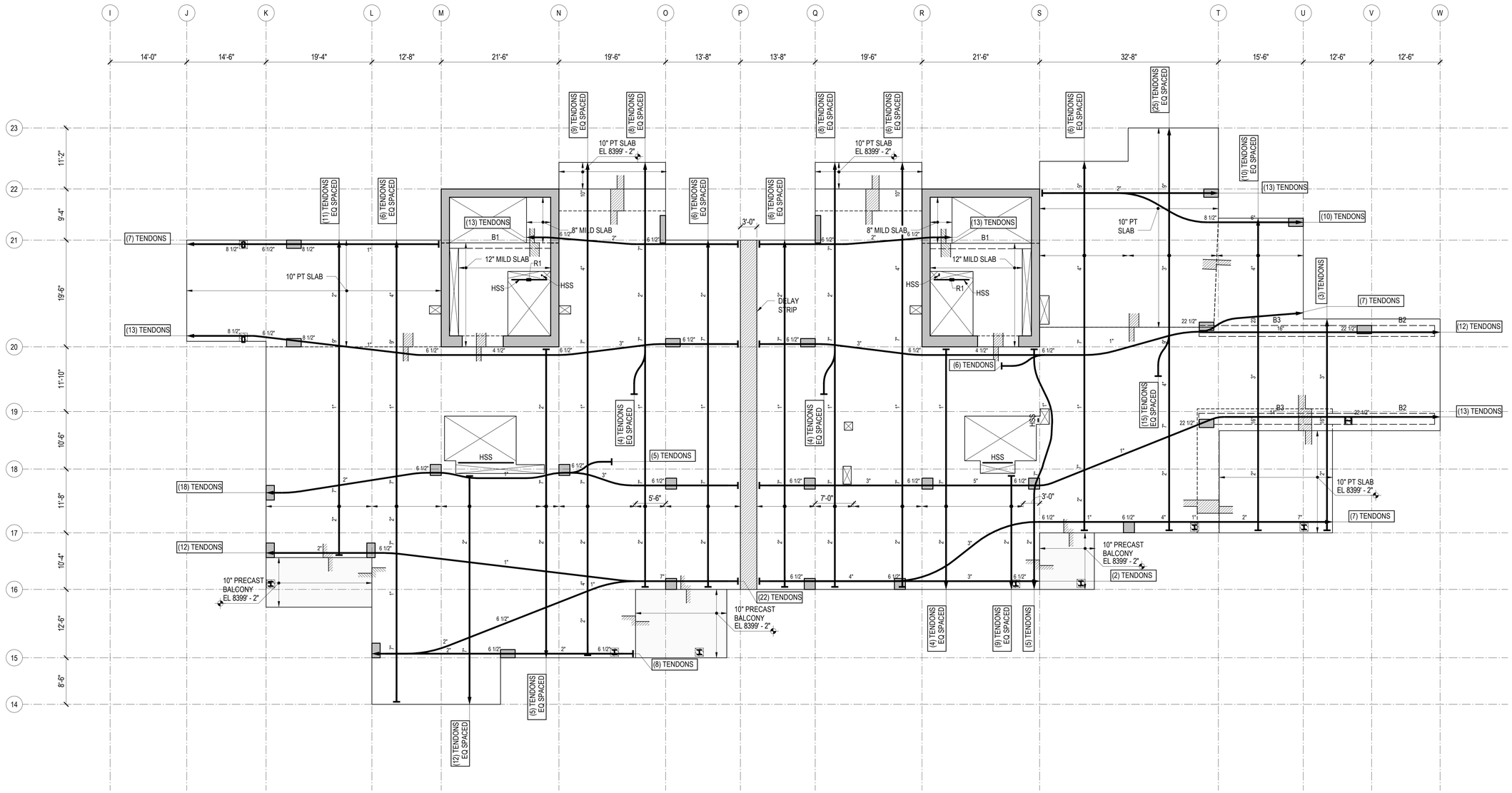
TOWER B LEVEL 3 REINFORCING PLAN

S2.B.13.R



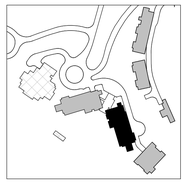
1 TOWER B - LEVEL 4 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8400' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8399' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.



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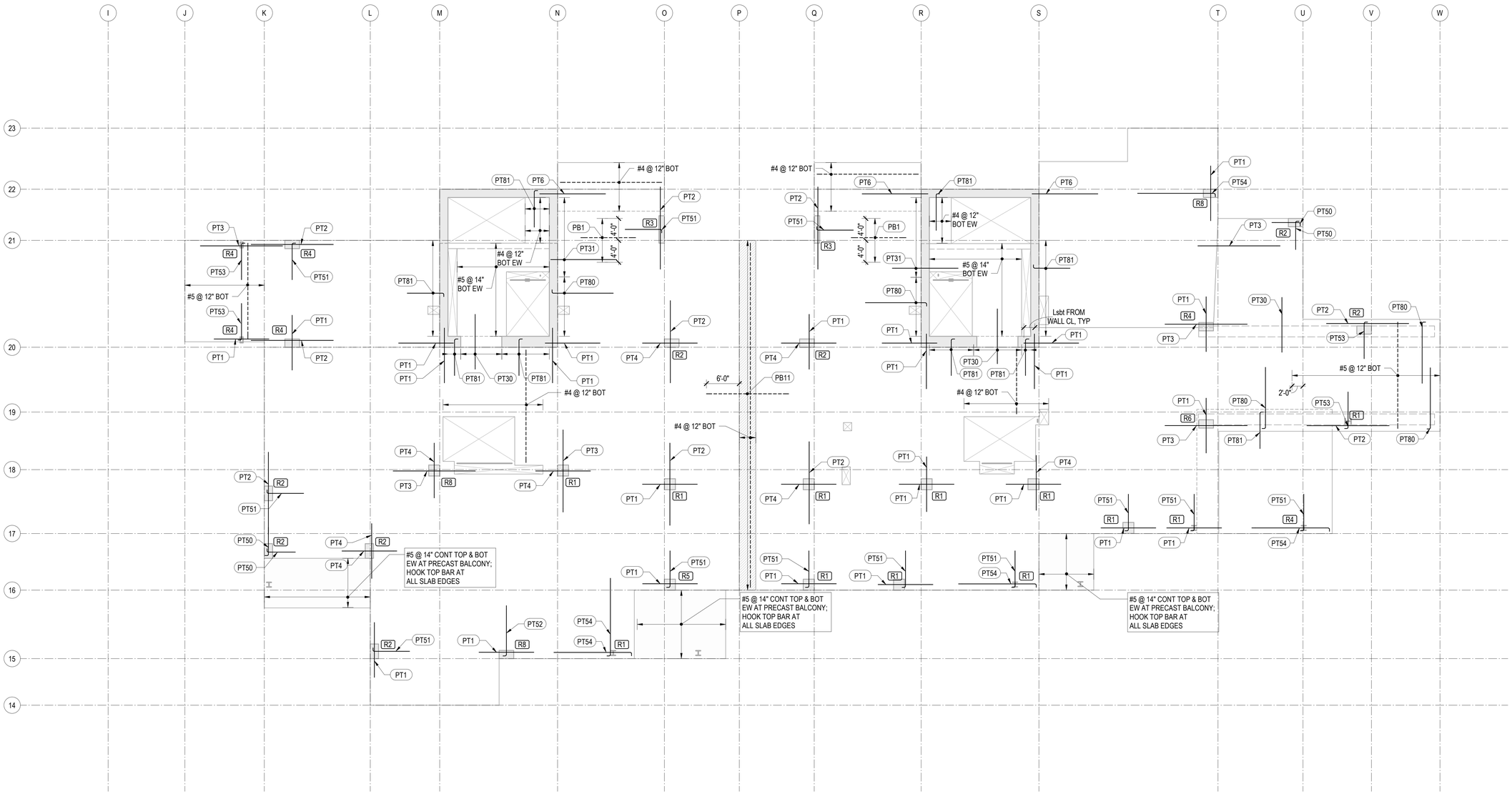
TOWER B LEVEL 4 FRAMING PLAN

S2.B.14



1 TOWER B - LEVEL 4 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

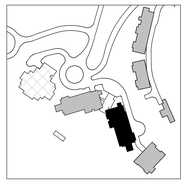
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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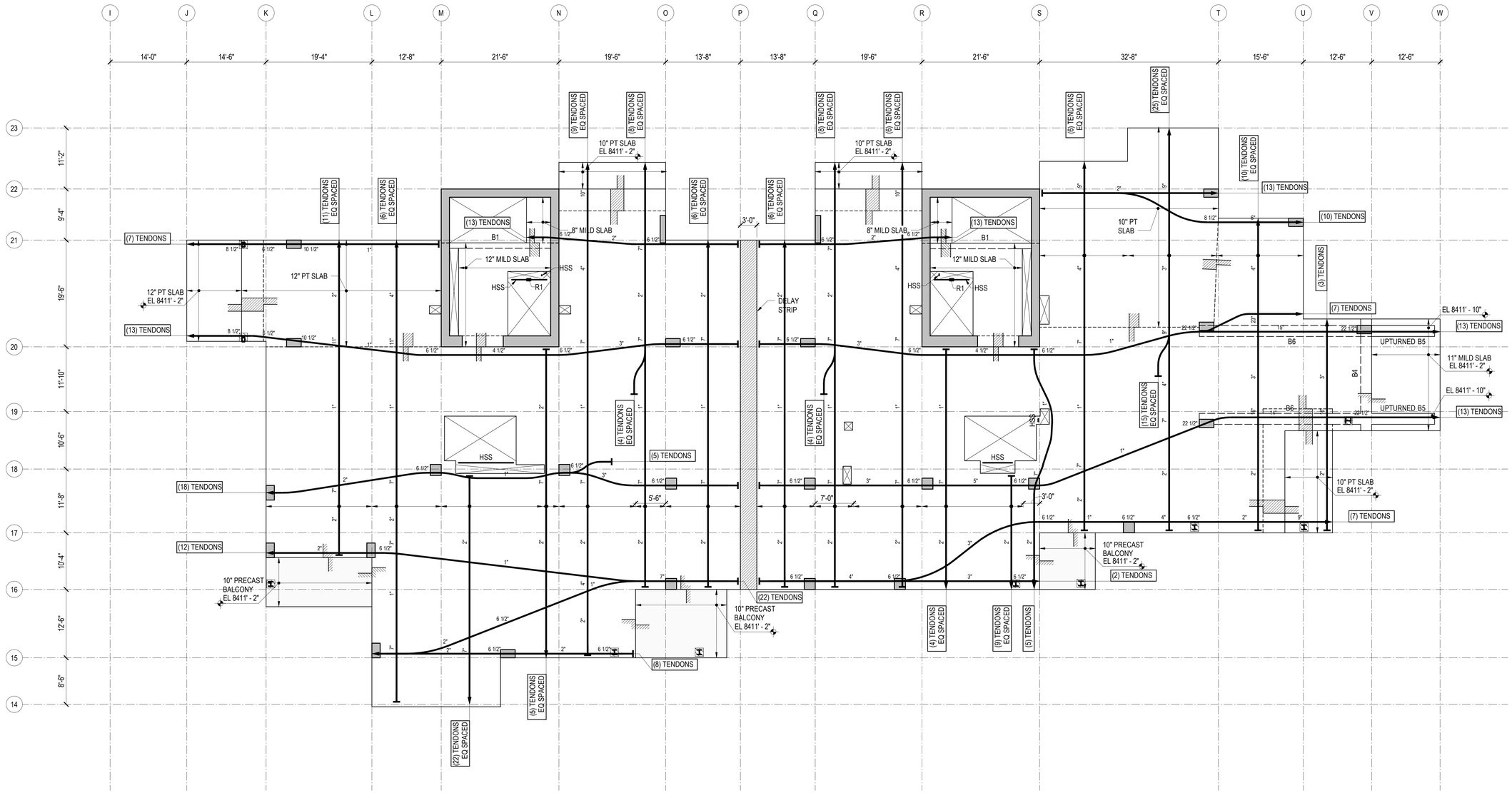
TOWER B LEVEL 4 REINFORCING PLAN

S2.B.14.R



1 TOWER B - LEVEL 5 FRAMING PLAN
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8412' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8411' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
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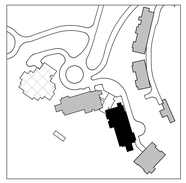
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.



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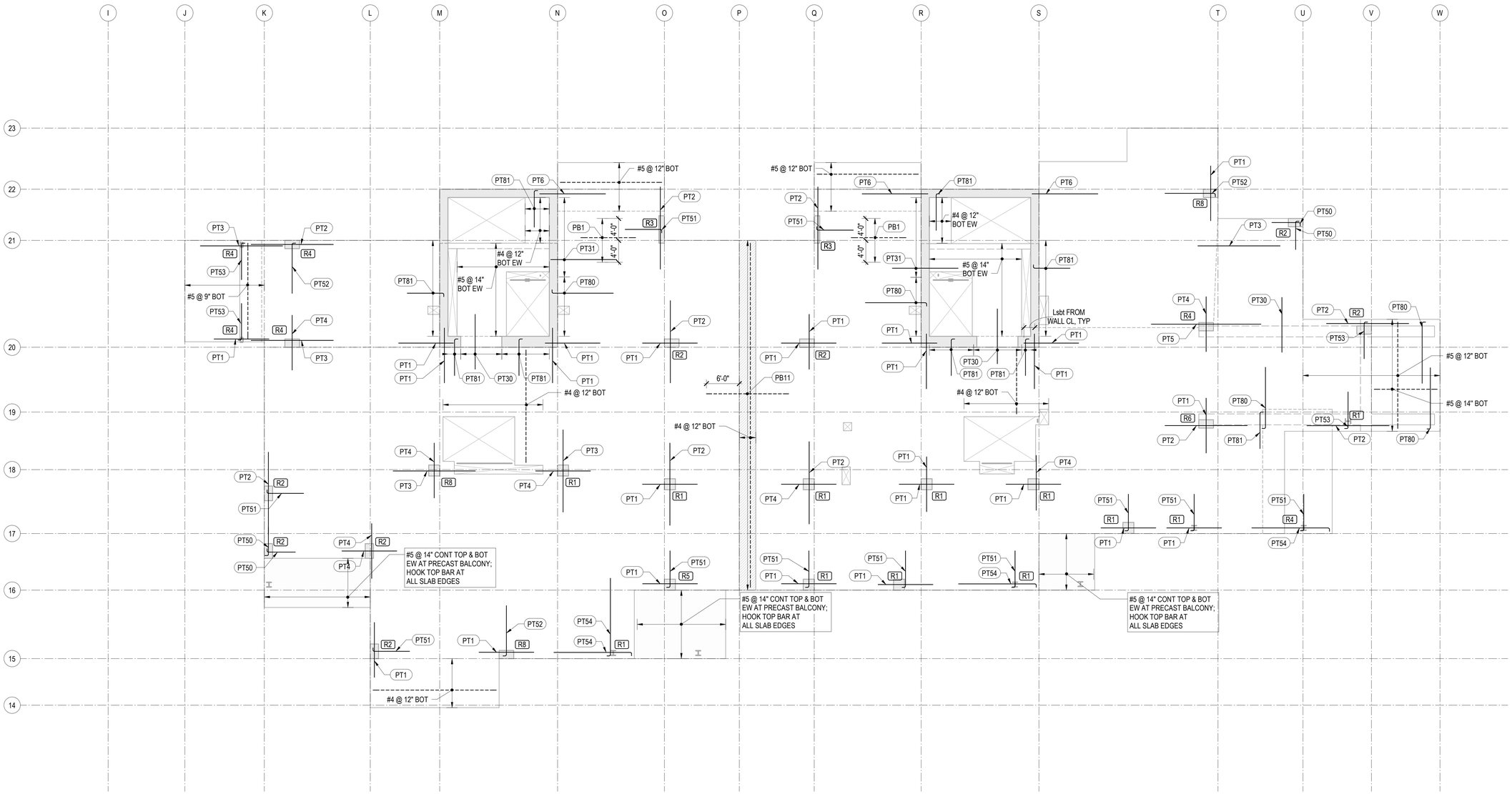
TOWER B LEVEL 5 FRAMING PLAN

S2.B.15



1 TOWER B - LEVEL 5 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
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7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

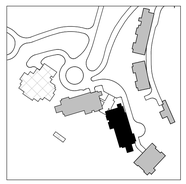
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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revisions:

no. date by

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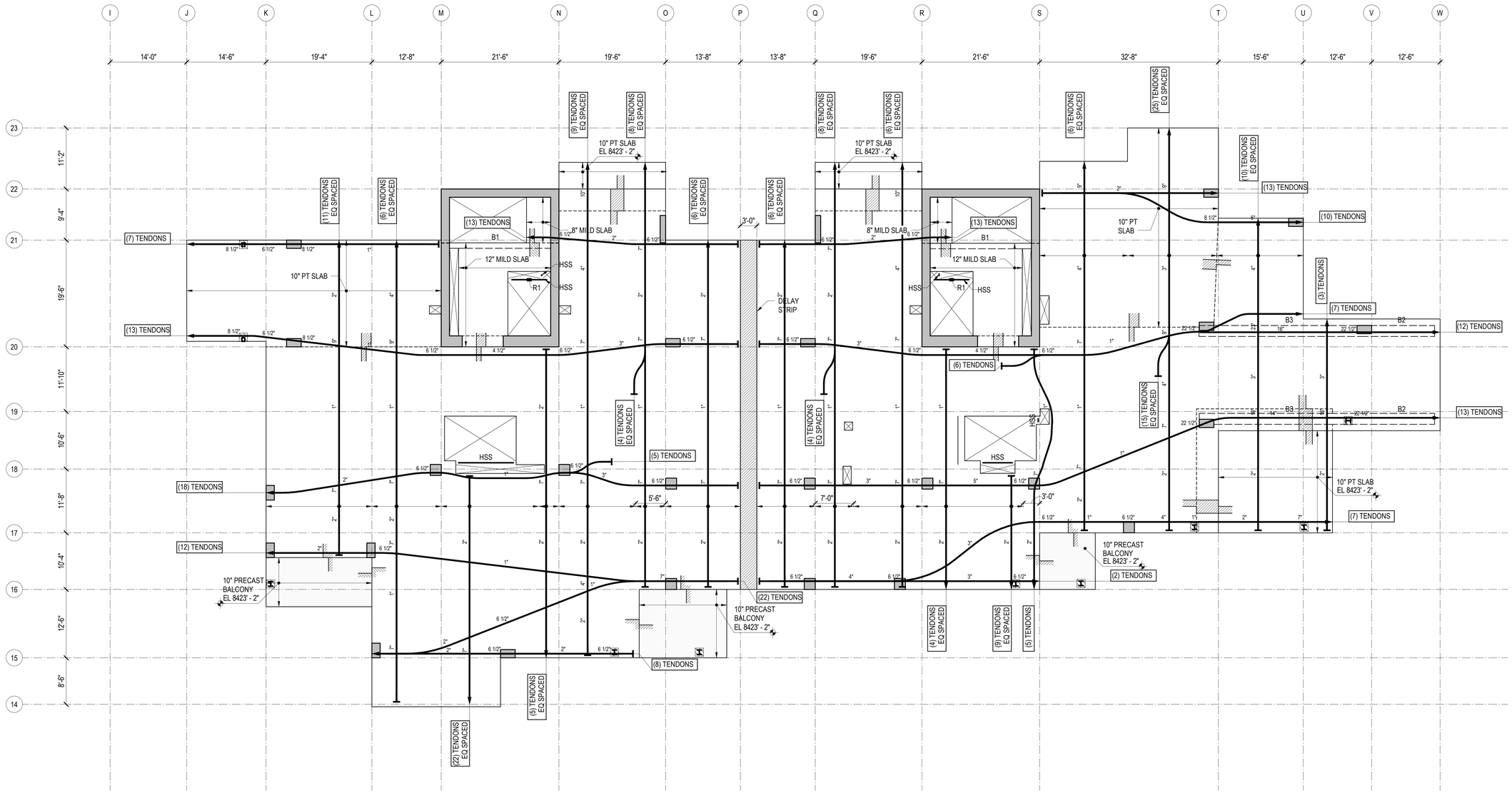
TOWER B LEVEL 5 REINFORCING PLAN

S2.B.15.R



1 TOWER B - LEVEL 6 FRAMING PLAN

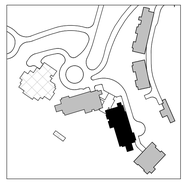
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8424' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8423' - 10", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.
10. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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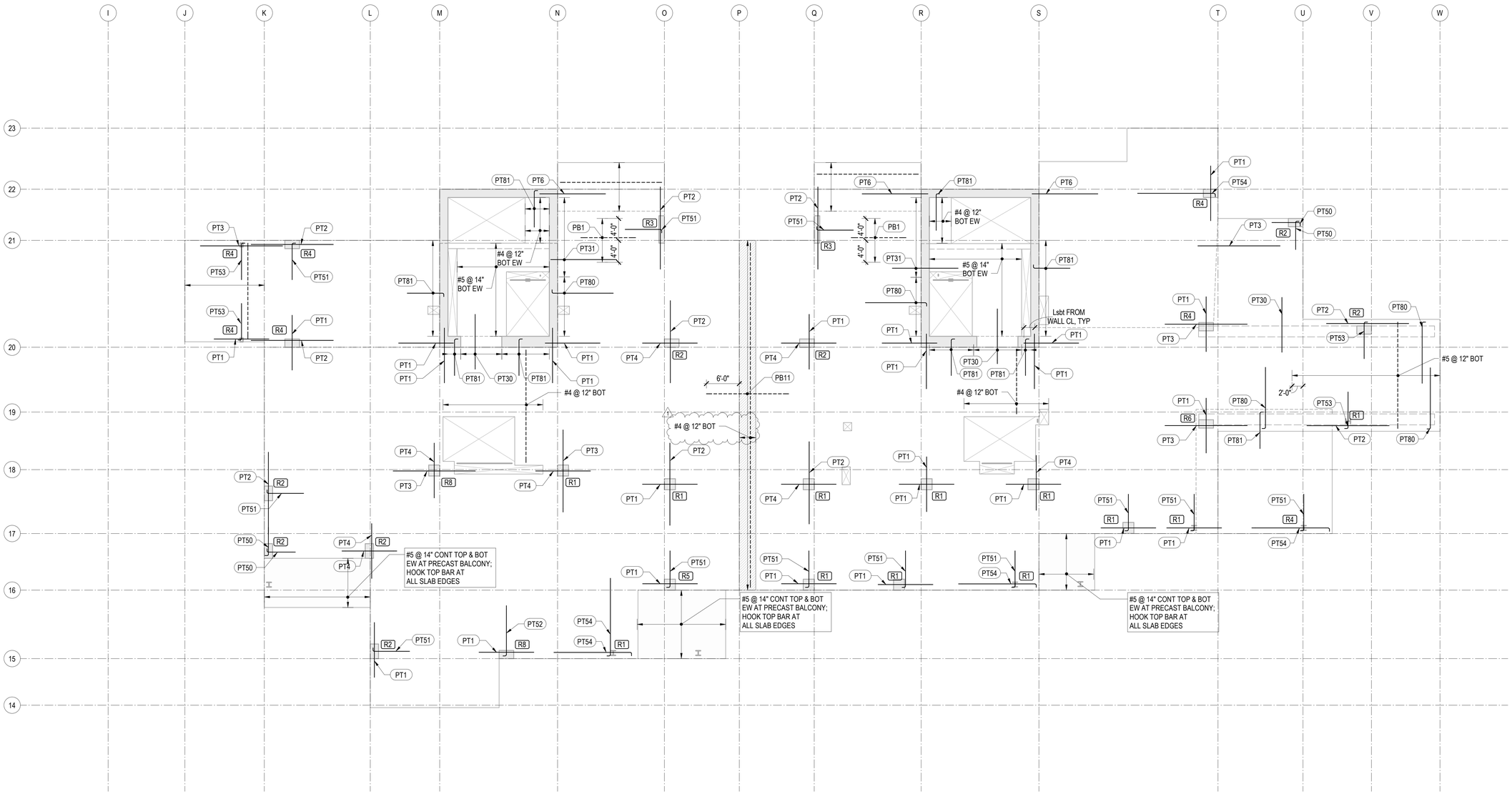
TOWER B LEVEL 6 FRAMING PLAN

S2.B.16



1 TOWER B - LEVEL 6 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
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7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

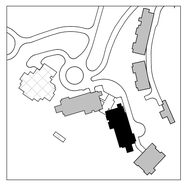
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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date 7/15/2022

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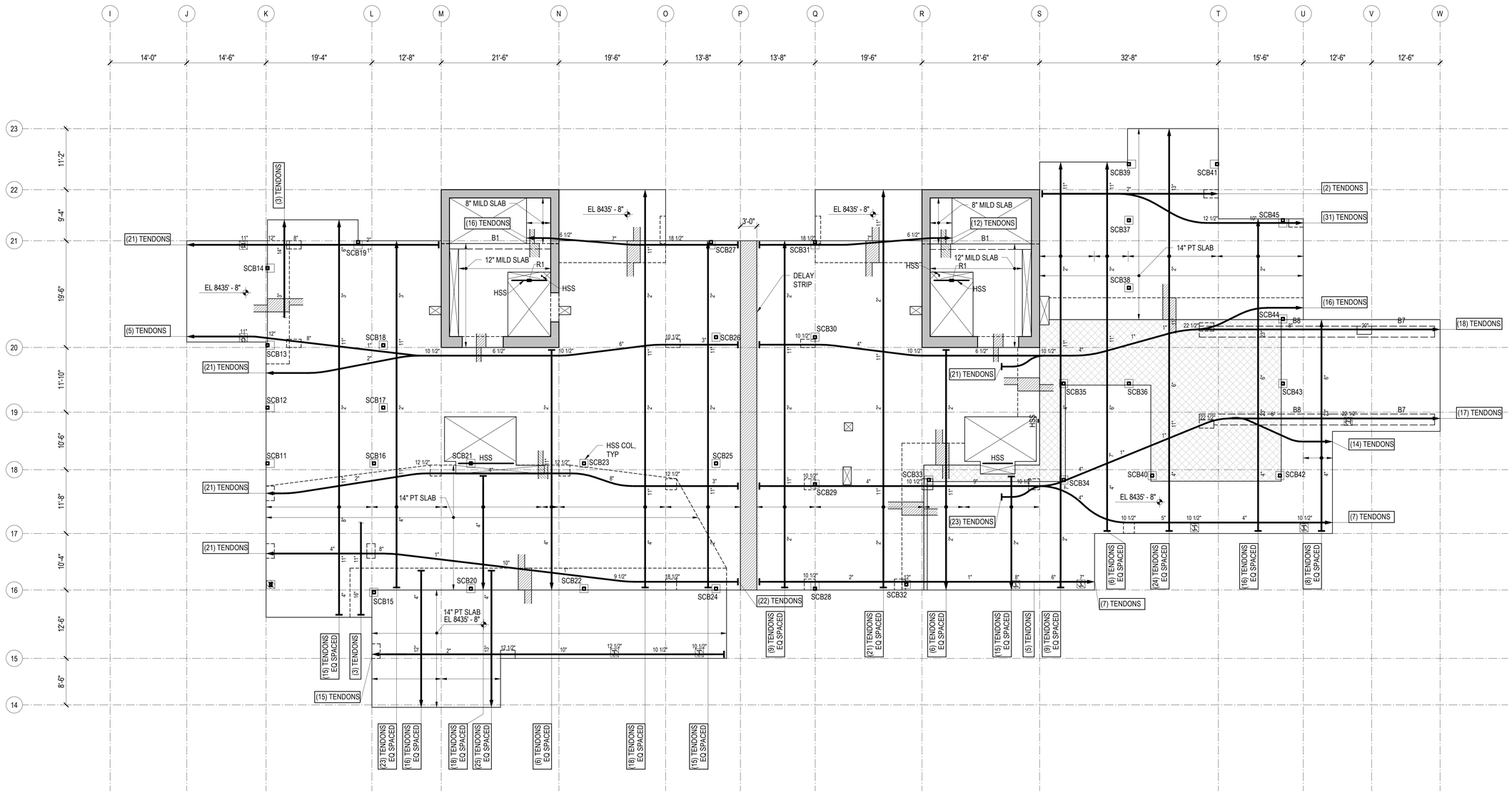
TOWER B LEVEL 6 REINFORCING PLAN

S2.B.16.R



1 TOWER B - LEVEL 7 FRAMING PLAN

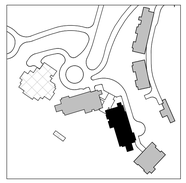
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8436' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8436' - 4" , UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS A 12-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▨ INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.
10. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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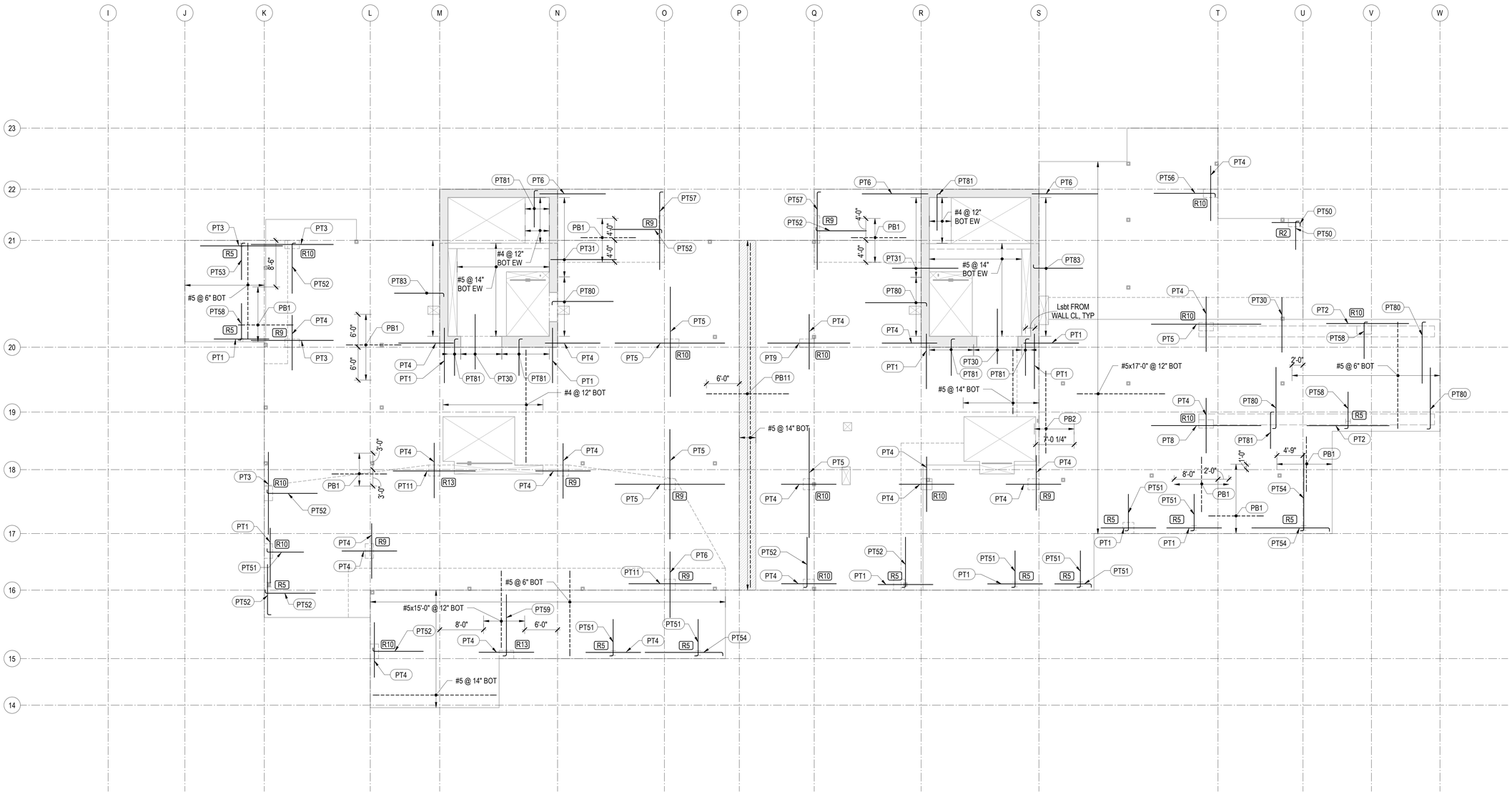
TOWER B LEVEL 7 FRAMING PLAN

S2.B.17



1 TOWER B - LEVEL 7 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY \ OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

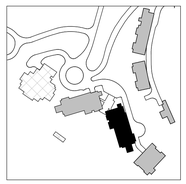
MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x20'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT8	(16) #6x20'-0"	
PT9	(13) #6x15'-0"	
PT10	(12) #5x20'-0"	
PT11	(12) #5x15'-0"	
PT30	#5x10'-0"@ 15"	
PT31	#5x12'-0"@ 12"	STAGGER 2'-0"
PT50	(3) #5x5'-2"	HOOK AT END

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT53	(8) #5x6'-8" @ 12"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT55	(8) #5x14'-2" @ 12"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(16) #5x14'-2"	HOOK AT END
PT58	(12) #5x6'-8"@12"	HOOK AT END
PT59	(14) #5x11'-2"@12"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0" @ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 12"	
PB7	#5x20'-0" @ 12"	
PB11	#5x15'-0" @ 12"	LAP SPLICE AT DELAY STRIP PER 12/S4.05



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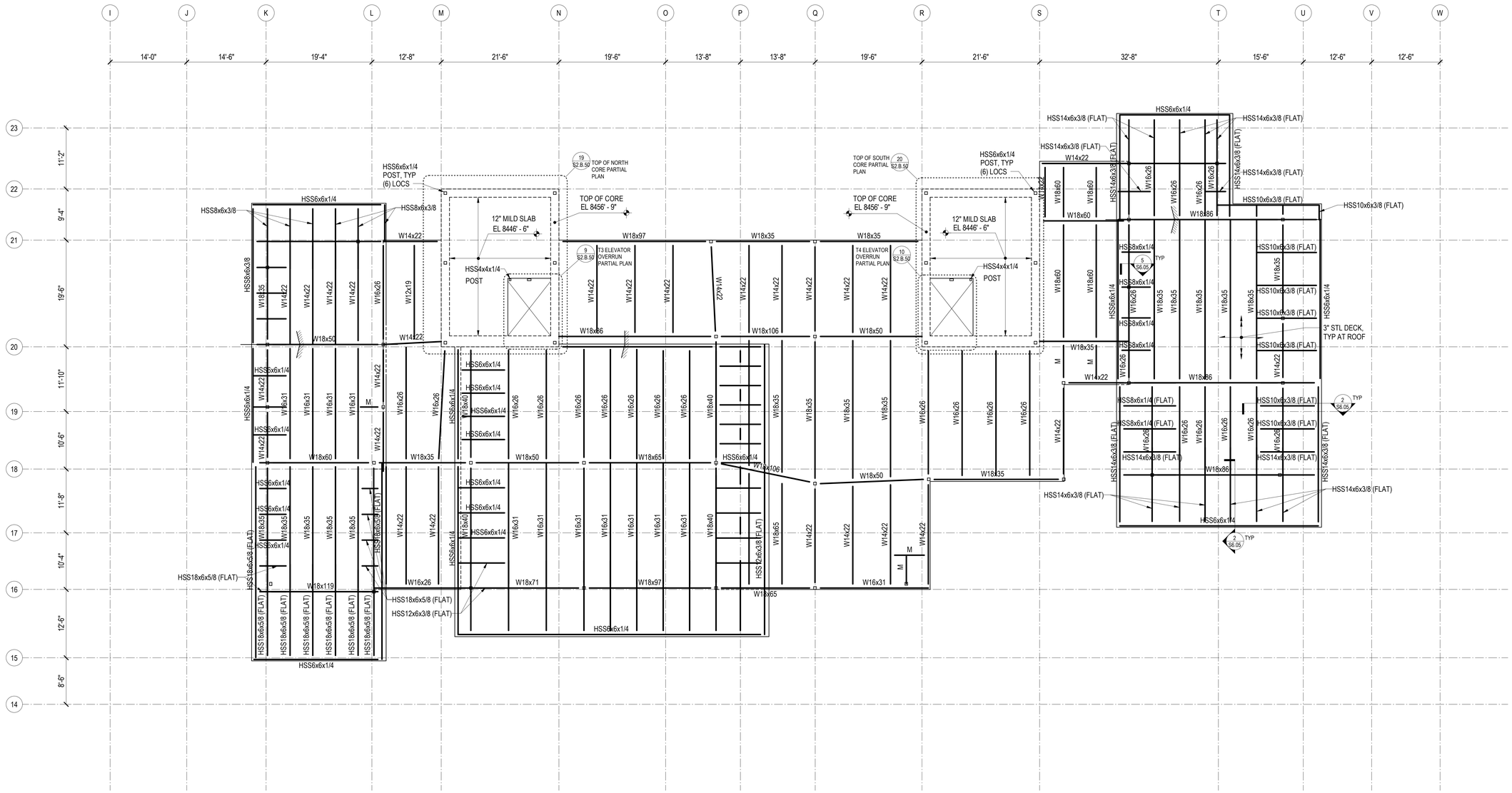
TOWER B LEVEL 7 REINFORCING PLAN

S2.B.17.R



1 TOWER B - ROOF FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

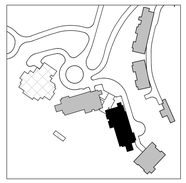
1. REFERENCE FLOOR ELEVATION IS 8448' - 6". TOP OF SLAB ON STEEL DECK IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. ROOF DECK IS MINIMUM 3 INCH X 20 GAUGE STEEL DECK. DECKING IS TO BE INSTALLED IN MINIMUM TWO SPAN CONDITION WHERE POSSIBLE. DECKING CONNECTIONS ARE TO BE AS FOLLOWS:

   SIDE LAP SEAM CONNECTIONS ARE 1-1/2 INCH TOP SEAM WELDS AT 18 INCHES MAXIMUM.

   CONNECTION TO SUPPORTING FRAMING SHALL BE A 24/4 WELD PATTERN WITH 3/4 INCH PUDDLE WELDS.

   CONNECTION TO FRAMING BELOW AND IN LINE WITH FLUTE SHALL BE 3/4 INCH PUDDLE WELDS AT 18 INCHES MAXIMUM.

   BUTTON PUNCHING OF SIDE LAP CONNECTIONS IS NOT PERMITTED.
3. REFERENCE TOP OF STEEL IS AT THE BOTTOM OF SLAB ON STEEL DECK UNLESS NOTED OTHERWISE.
4. STEEL SLOPES UNIFORMLY BETWEEN GIVEN TOP OF STEEL ELEVATIONS. WHERE BEAMS OR BEAMS AND COLUMNS INTERSECT, MATCH TOP OF STEEL UNLESS NOTED OTHERWISE.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING SLABS AND WALLS.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE 'TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE,' 'TYPICAL ROOF DECK OPENINGS,' AND 'TYPICAL COMPOSITE DECK OPENINGS' DETAILS FOR OPENING PLACEMENT CRITERIA AND REINFORCING OR FRAMING REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.



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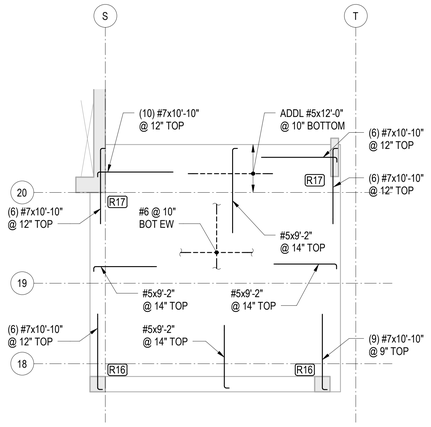
TOWER B ROOF FRAMING PLAN

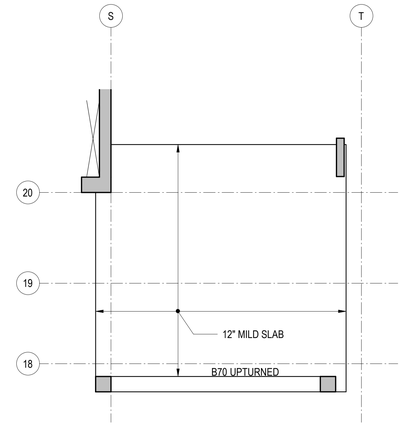
S2.B.18



REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   N-S BOTTOM BARS
   E-W BOTTOM BARS
   E-W TOP BARS
   N-S TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
6. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
7. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
8. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
9. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

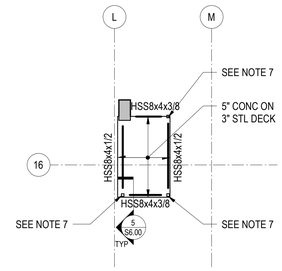
6 PARTIAL REINFORCING PLAN - TOWER B LEVEL P1 MECHANICAL MEZZANINE
1/8" = 1'-0"



NOTES

1. REFERENCE FLOOR ELEVATION IS 8355' - 0". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 12-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.

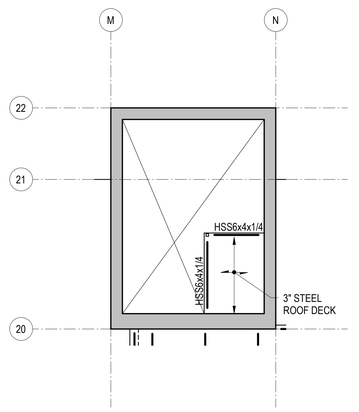
7 PARTIAL FRAMING PLAN - TOWER B LEVEL P1 MECHANICAL MEZZANINE
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8340'-0". TOP OF SLAB ON STEEL DECK IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS 5 INCHES OF NORMALWEIGHT CONCRETE ON 3-INCH COMPOSITE STEEL DECK UNLESS NOTED OTHERWISE. REINFORCE PER SECTION ON PLAN. REINFORCING SHOWN ON THE PLAN AND IN THE TYPICAL DETAILS IS IN ADDITION TO THIS REINFORCING.
3. REFERENCE TOP OF STEEL IS AT THE BOTTOM OF SLAB ON STEEL DECK UNLESS NOTED OTHERWISE.
4. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING SLABS AND WALLS.
5. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE 'TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE,' 'TYPICAL ROOF DECK OPENINGS,' AND 'TYPICAL COMPOSITE DECK OPENINGS' DETAILS FOR OPENING PLACEMENT CRITERIA AND REINFORCING OR FRAMING REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
6. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
7. WHERE NOTE APPLIES, HSS POST CONTINUES TO UNDERSIDE OF LEVEL P1 SLAB. PROVIDE "TYPICAL HSS POST TOP SLIP CONNECTION" AT UNDERSIDE OF SLAB ABOVE.

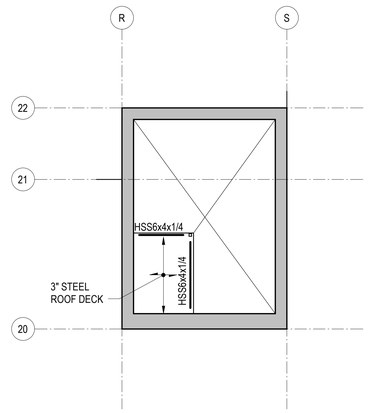
8 PARTIAL PLAN - LOBBY SHUTTLE ELEVATOR PIT
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8450'-7". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

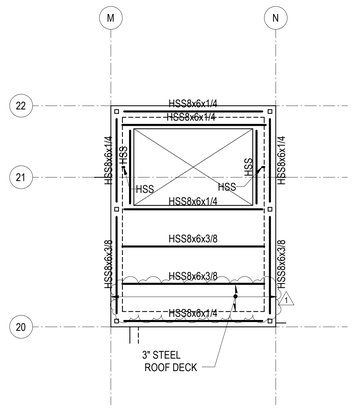
9 PARTIAL PLAN - T3 ELEVATOR OVERRUN
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8450'-7". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

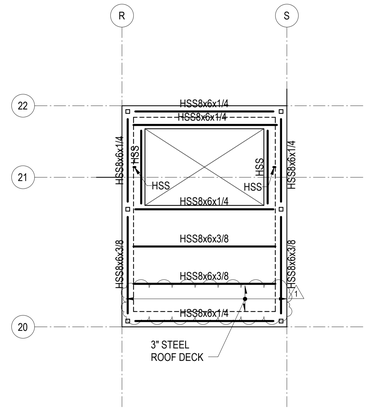
10 PARTIAL PLAN - T4 ELEVATOR OVERRUN
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8458'-9". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

19 PARTIAL PLAN - TOP OF NORTH CORE
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8458'-9". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

20 PARTIAL PLAN - TOP OF SOUTH CORE
1/8" = 1'-0"

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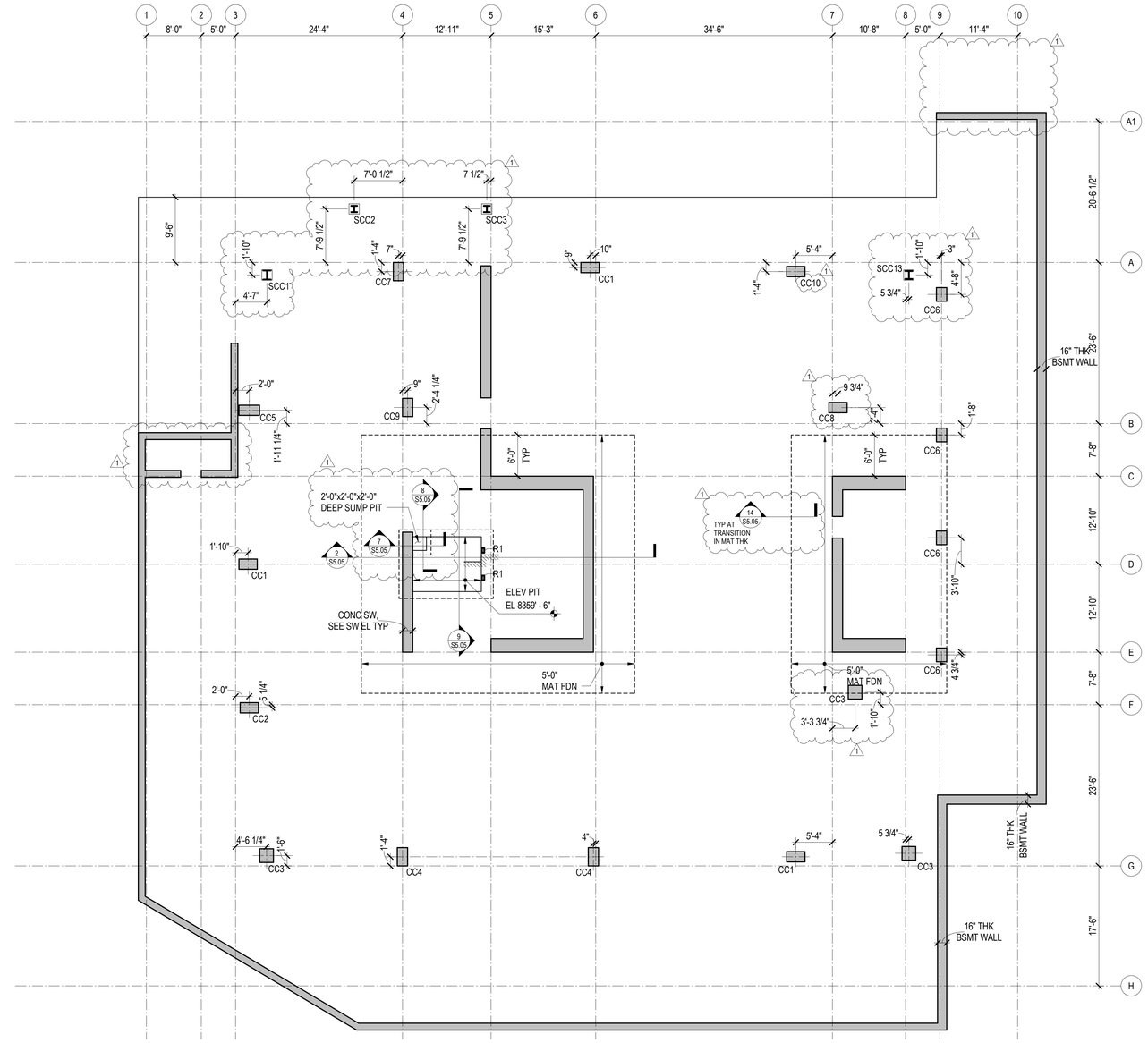
TOWER B PARTIAL PLANS

S2.B.50



1 TOWER C - FOUNDATION LEVEL FRAMING PLAN

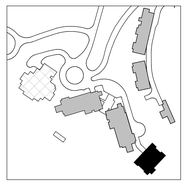
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE ELEVATION IS 8364' - 6". TOP OF MAT IS AT 8364'-0" UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL SLAB EDGES, OPENINGS, SLOPES, AND DEPRESSIONS NOT DEFINED ON THE STRUCTURAL PLANS.
2. MAT FOUNDATION IS 3'-0" THICK UNLESS NOTED OTHERWISE. UPON REACHING THE MAT FOUNDATION SUBGRADE ELEVATION, SOIL CONDITIONS SHALL BE EVALUATED AND APPROVED BY THE GEOTECHNICAL ENGINEER OF RECORD.
3. SHEAR WALL OPENINGS, WALL ENDS, AND WALL LOCATIONS ARE DIMENSIONED RELATIVE TO GRID LINES ON THE SHEAR WALL ELEVATION.
4. BASEMENT WALLS SHALL BE RESTRAINED AT EACH FLOOR BY THE STRUCTURAL SLAB AND AT THE BOTTOM OF THE MAT, AND SHALL HAVE REACHED DESIGN STRENGTH PRIOR TO PLACING BACKFILL AND/OR DE-TENSIONING TIE-BACK ANCHORS.
5. BASEMENT WALLS ARE DESIGNED FOR A FULLY DRAINED CONDITION IN THE RETAINED SOIL.
6. BASEMENT WALL REINFORCEMENT IS SHOWN ON THE BASEMENT WALL ELEVATIONS.
7. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS PRIOR TO CASTING FOUNDATIONS.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. PROVIDE DOCK LEVELER, ELEVATOR PITS, ESCALATOR PITS, SUMP PITS, M/E/P VAULTS, TRENCH AND AREA DRAINS, AND CONCRETE ENCASEMENTS/EMBEDMENTS/INSERTS/ETC. AS REQUIRED. REINFORCE PER TYPICAL DETAILS.
9. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR SIDEWALKS, PAVING, AND SITE DETAILS AT BUILDING EXTERIOR UNLESS NOTED OTHERWISE.



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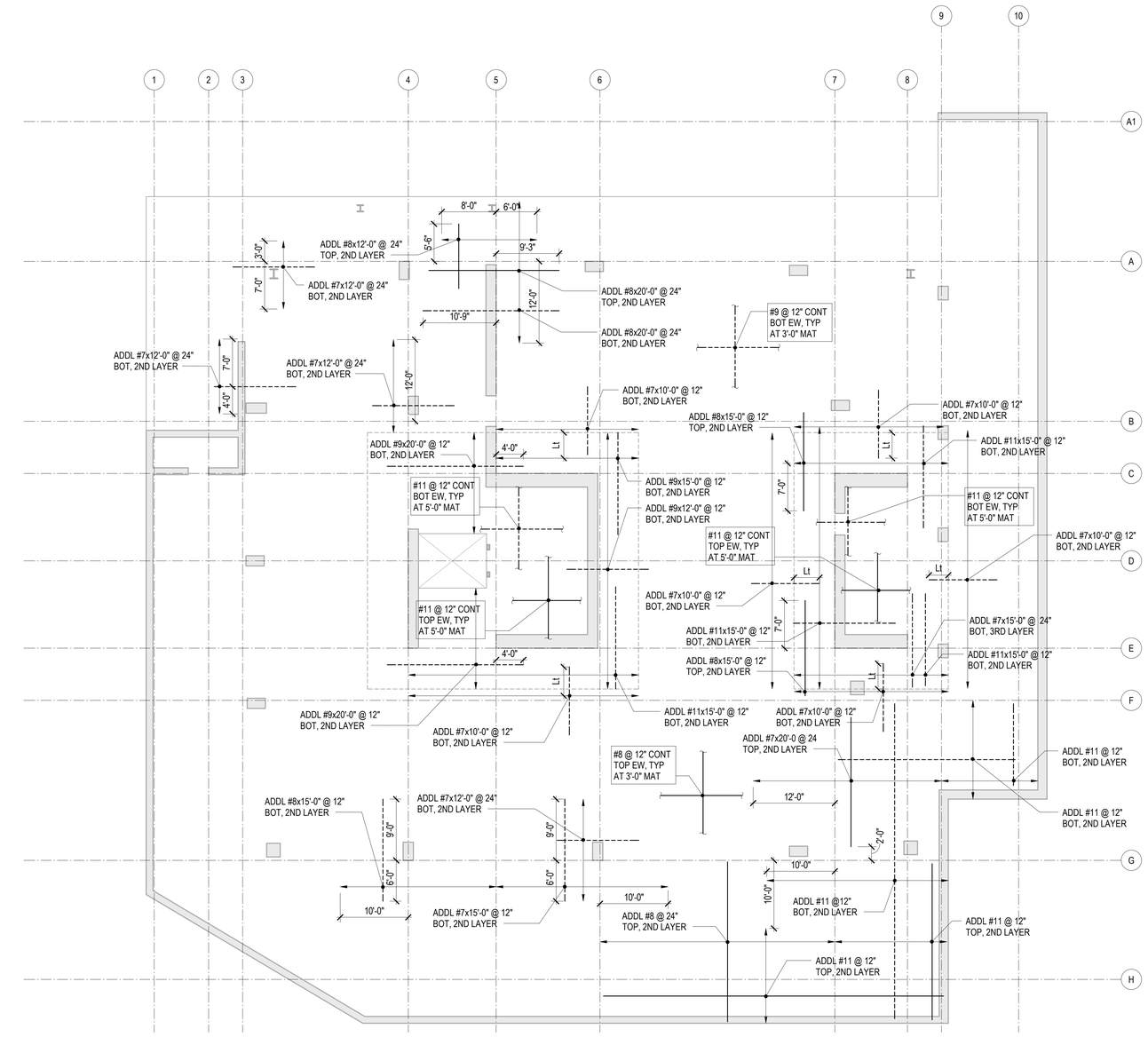
TOWER C
FOUNDATION
LEVEL FRAMING
PLAN

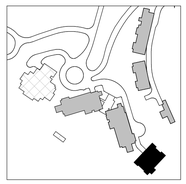
S2.C.01



1 TOWER C - FOUNDATION - LONGITUDINAL REINFORCEMENT PLAN
1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.



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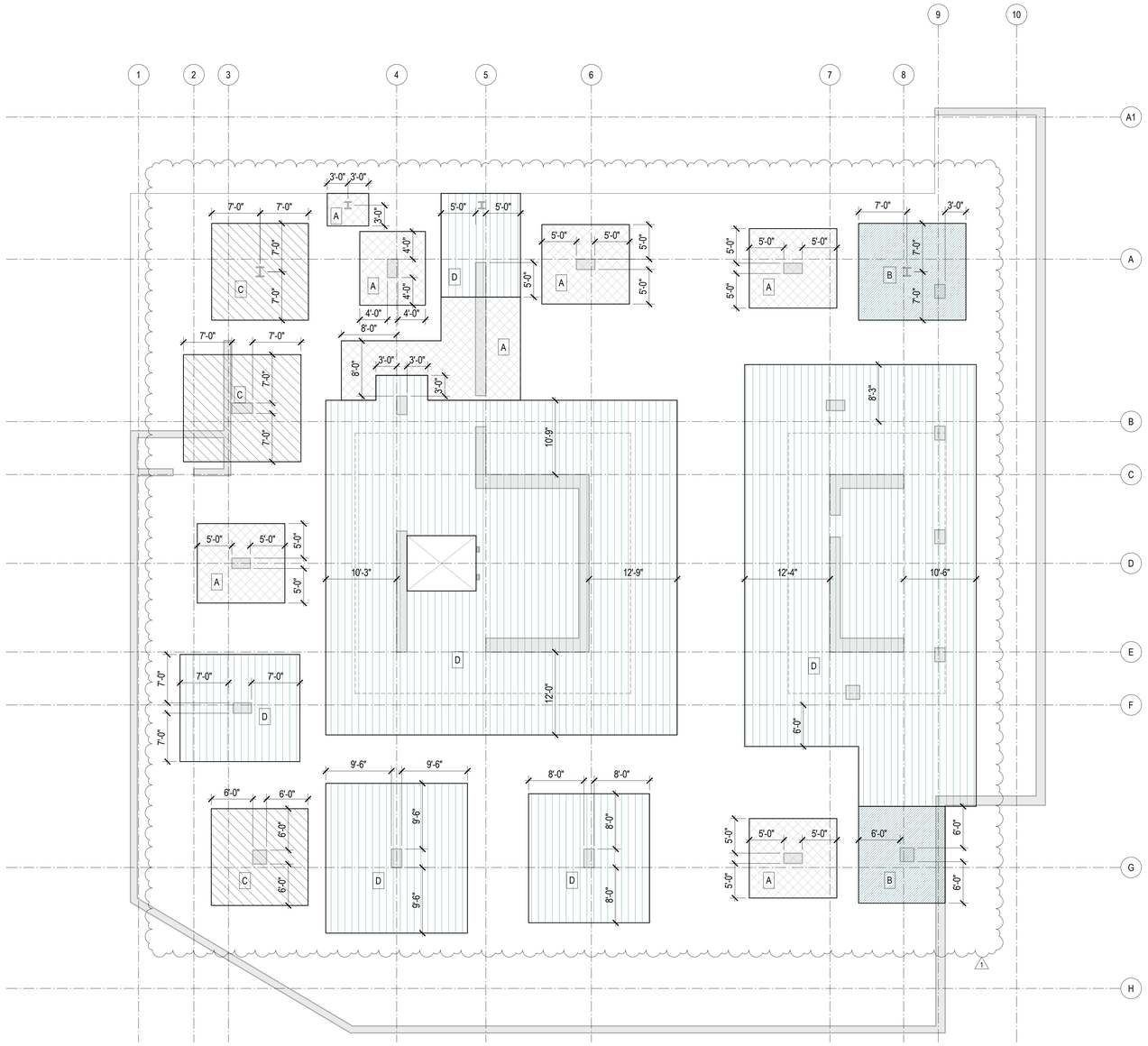
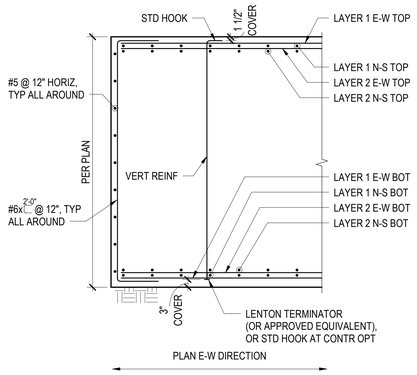
TOWER C
FOUNDATION
LONGITUDINAL
REINFORCING
PLAN

S2.C.01.B





1 TOWER C - FOUNDATION - SHEAR REINFORCEMENT PLAN

1/8" = 1'-0"

MAT FOUNDATION REINFORCING NOTES:

1. SEE THE "GENERAL NOTES" FOR GENERAL REINFORCING REQUIREMENTS.
2. SEE MAT REINFORCEMENT PLACEMENT DIAGRAM FOR LAYER AND DIRECTION KEY FOR MAT REINFORCEMENT PLACEMENT.
3. HOOK OR PROVIDE TERMINATORS AT ALL #11 BARS OR SMALLER AND PROVIDE TERMINATORS AT ALL #14 AND #18 BARS INTERRUPTED AT PITS AND OPENINGS.
4. PLACE BARS ON LAYER 1, UNLESS NOTED OTHERWISE.
5. ALL TOP AND BOTTOM REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.
6. VERTICAL REINFORCEMENT SHOWN ON MAT REINFORCEMENT PLANS SHALL BE GRADE 60 KSI.

FOUNDATION VERTICAL REINFORCING SCHEDULE

TYPE ⚠1	REINFORCING	REMARKS
A	#7 @ 24" EACH WAY	
B	#8 @ 24" EACH WAY	
C	#9 @ 24" EACH WAY	
D	#7 @ 24" EACH WAY	

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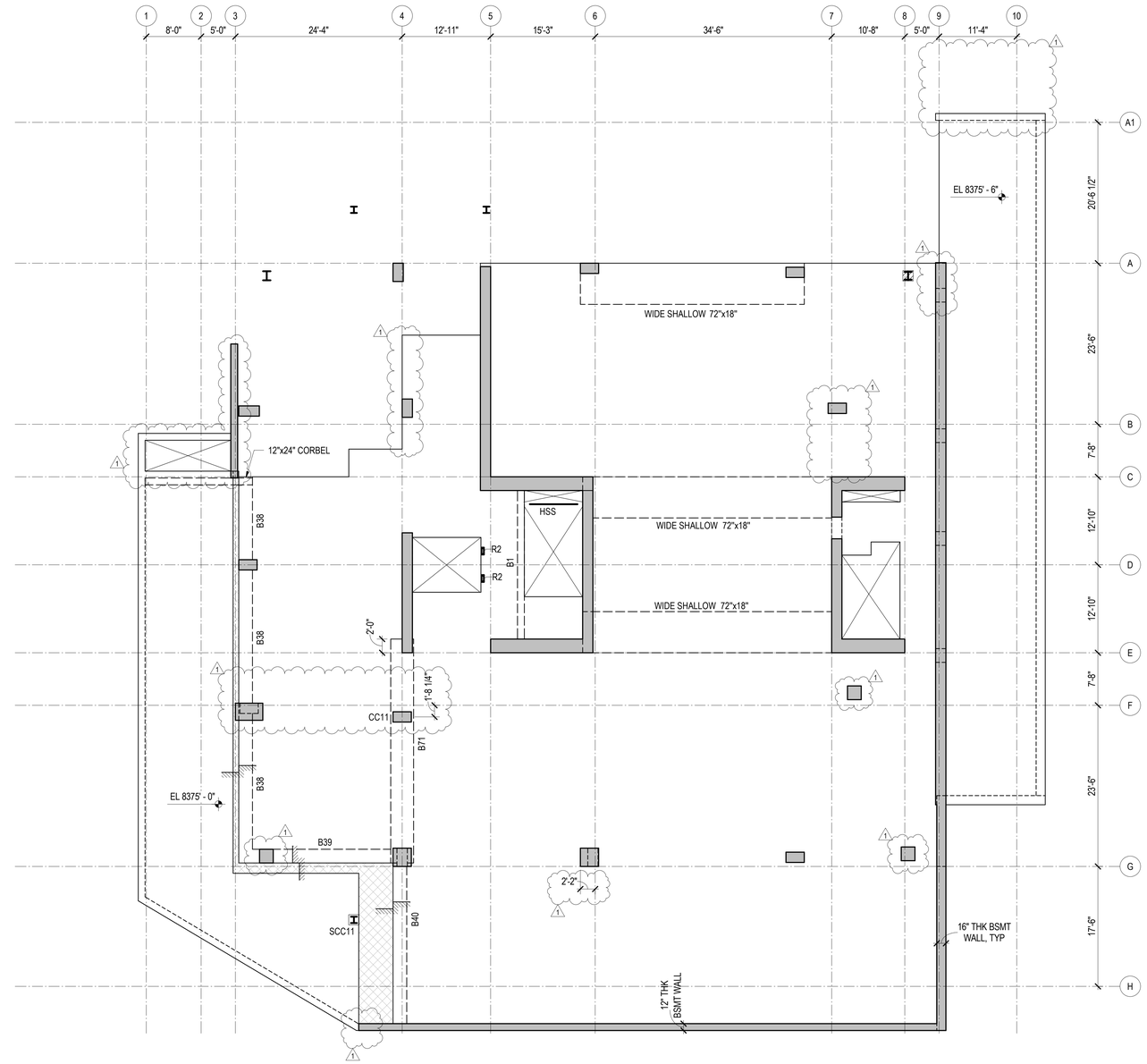
TOWER C
FOUNDATION
SHEAR
REINFORCING
PLAN

S2.C.01.V



1 TOWER C - LEVEL 1 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8376' - 6". TOP OF CONCRETE SLAB IS AT 8376'-5" UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 14-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.

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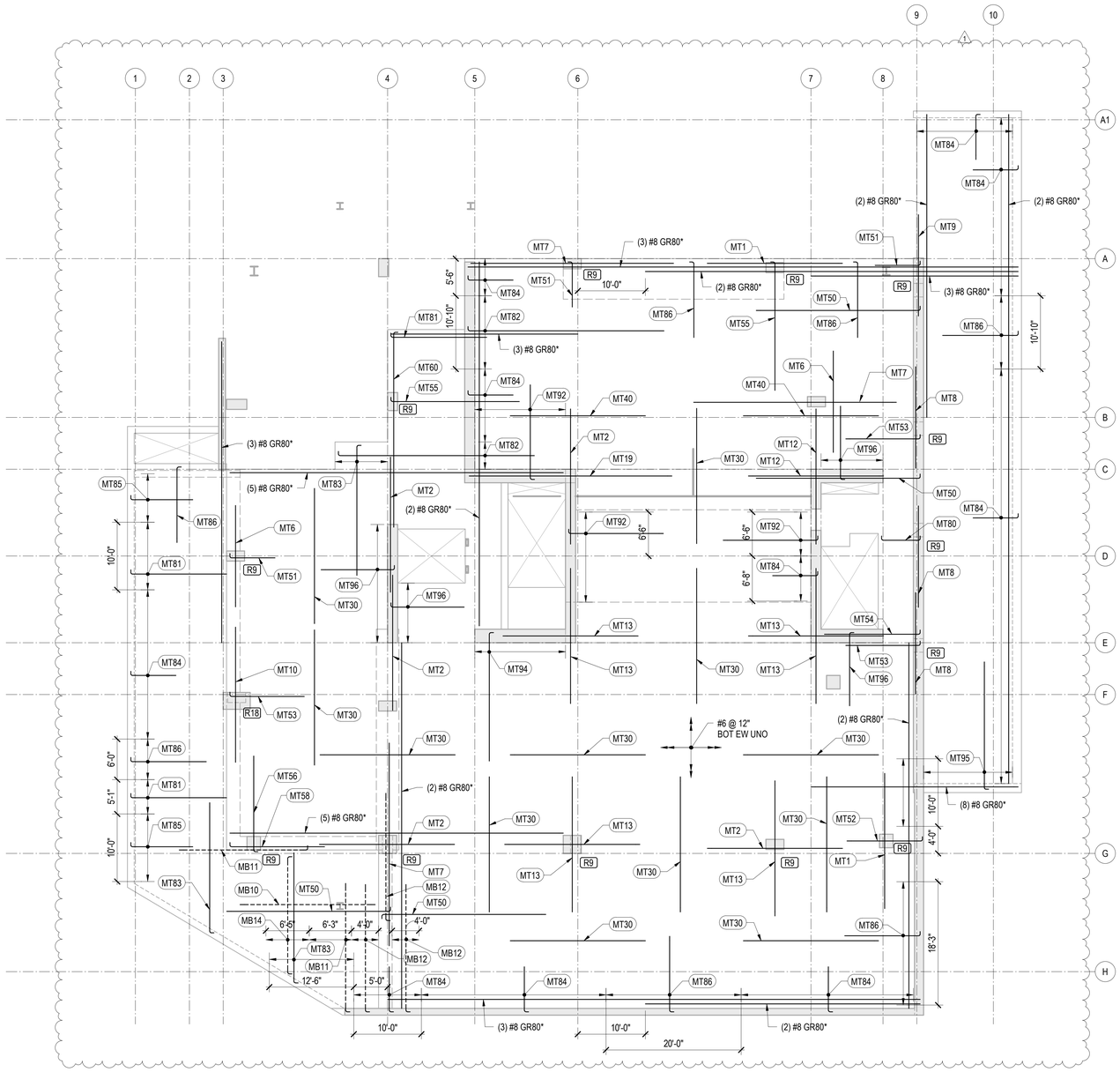
TOWER C LEVEL 1 FRAMING PLAN

S2.C.11



1 TOWER C - LEVEL 1 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL MILD SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   E-W BOTTOM BARS
   N-S BOTTOM BARS
   N-S TOP BARS
   E-W TOP BARS
4. FOR CONTINUOUS BOTTOM BARS, LAP BARS Lsb AS REQUIRED WITH LAPS AT 1/3 THE SLAB SPAN BETWEEN ADJACENT COLUMNS.
5. TWO OF THE CONTINUOUS BOTTOM BARS ARE TO BE PLACED EACH WAY THROUGH ALL COLUMNS WITH COLUMN VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
6. BOTTOM BARS CALLED OUT ARE IN ADDITION TO CONTINUOUS BOTTOM MAT.
7. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
8. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
9. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
10. WHERE NOTED AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.

1

11. * INDICATES DIAPHRAGM REINFORCEMENT THAT IS PART OF THE LATERAL FORCE RESISTING SYSTEM AND IS IN ADDITION TO OTHER BARS SHOWN. THIS REINFORCEMENT SHALL BE CENTERED IN SLAB MID-DEPTH, UNO. REINFORCEMENT SHALL MEET CENTER-TO-CENTER SPACING OF 3db BUT NOT LESS THAN 3-INCHES, UNLESS NOTED OTHERWISE. LAP Lsb AS REQUIRED, STAGGER LAPS.

MILD TOP REINFORCEMENT SCHEDULE

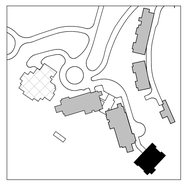
MARK	REINFORCING	REMARKS
MT1	(13) #6x20'-0" @ 10"	STAGGER 6'-0"
MT2	(13) #7x20'-0" @ 10"	STAGGER 5'-0"
MT6	(11) #6x15'-0" @ 12"	STAGGER 4'-0"
MT7	(15) #7x30'-0" @ 9"	STAGGER 5'-0"
MT8	(6) #5x15'-0" @ 12"	STAGGER 3'-0"
MT9	(6) #7x15'-0" @ 12"	STAGGER 3'-0"
MT10	(6) #6x20'-0" @ 12"	STAGGER 5'-0"
MT12	(11) #8x20'-0" @ 12"	STAGGER 5'-0"
MT13	(16) #8x20'-0" @ 8"	STAGGER 5'-0"
MT19	(21)#8x20'-0" @ 6"	STAGGER 5'-0"
MT30	#5x20'-0" @ 12"	STAGGER 3'-0"
MT40	#6x20'-0" @ 12"	STAGGER 4'-0"
MT50	(6) #5x24'-3" @ 12"	HOOK AT END
MT51	(11) #5x6'-8" @ 12"	HOOK AT END

MILD TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MT52	(11) #5x11'-2" @ 12"	HOOK AT END
MT53	(7) #6x11'-0" @ 12"	HOOK AT END
MT54	(11) #5x14'-2" @ 12"	HOOK AT END
MT55	(16) #6x19'-0" @ 8"	HOOK AT END
MT56	(6) #5x14'-2" @ 12"	HOOK AT END
MT58	(11) #6x14'-0" @ 12"	HOOK AT END
MT60	(21) #7x28'-10" @ 6"	HOOK AT END
MT80	#5 @ 12"	HOOK BOTH ENDS
MT81	#5x14'-2" @ 12"	HOOK AT END
MT82	#6x29'-0" @ 12"	HOOK AT END
MT83	#5x19'-3" @ 12"	HOOK AT END
MT84	#5x6'-8" @ 12"	HOOK AT END
MT85	#5x9'-2" @ 12"	HOOK AT END
MT86	#5x11'-2" @ 12"	HOOK AT END
MT92	#6x14'-0" @ 12"	HOOK AT END
MT94	#6x19'-0"@ 8"	HOOK AT END
MT95	#7x18'-10"@ 6"	HOOK AT END
MT96	#7x10'-10" @ 12"	HOOK AT END

MILD BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
MB10	#5x20'-0" @ 8"	STAGGER 2'-6"
MB11	#6x19'-0" @ 6"	HOOK AT END
MB12	#7x18'-10" @ 6"	STAGGER 3'-0"
MB14	#5 @ 12"	HOOK BOTH ENDS



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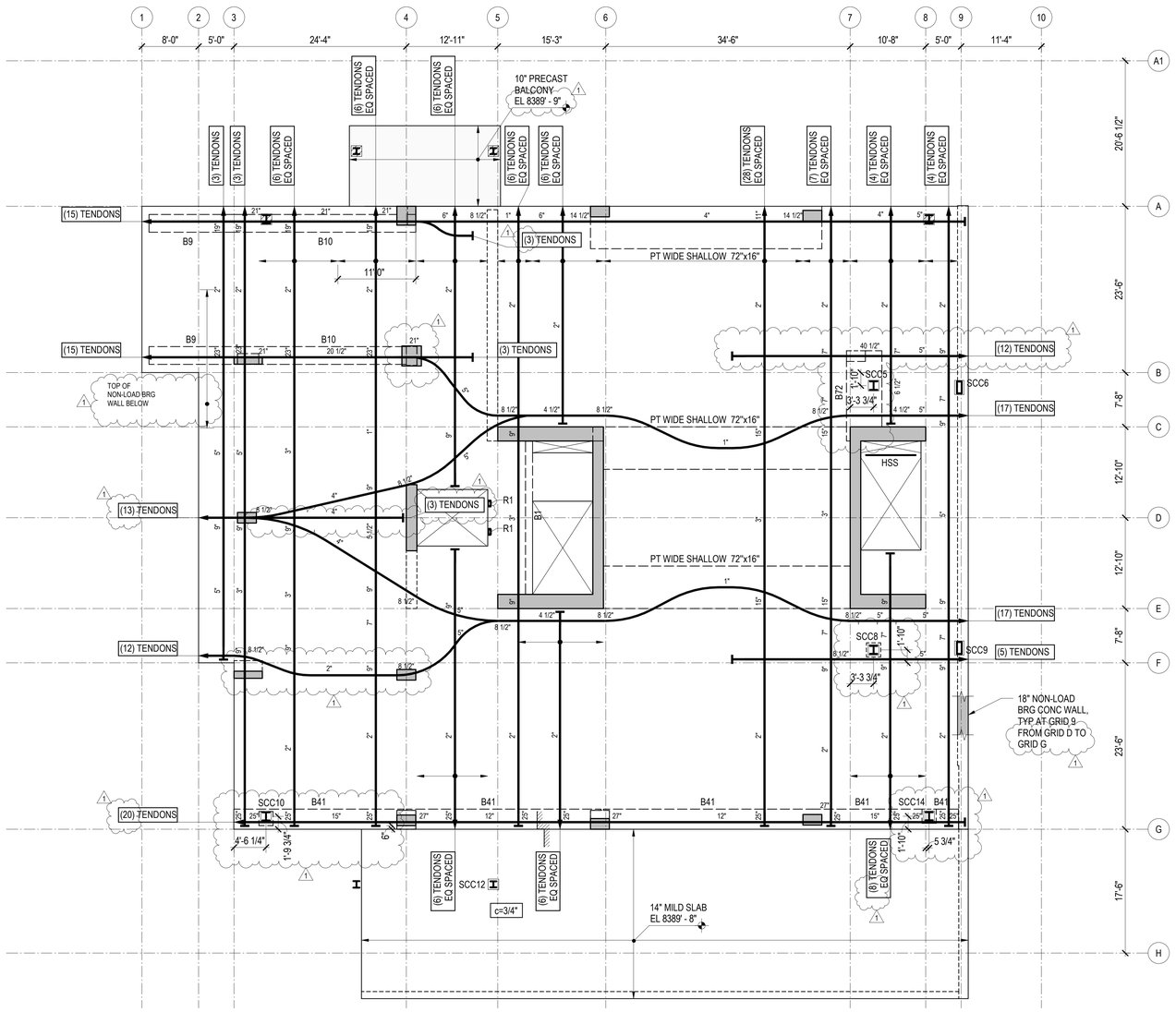
TOWER C LEVEL 1 REINFORCING PLAN

S2.C.11.R



1 TOWER C - LEVEL 2 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8390' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8390' - 5", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS A 10-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.

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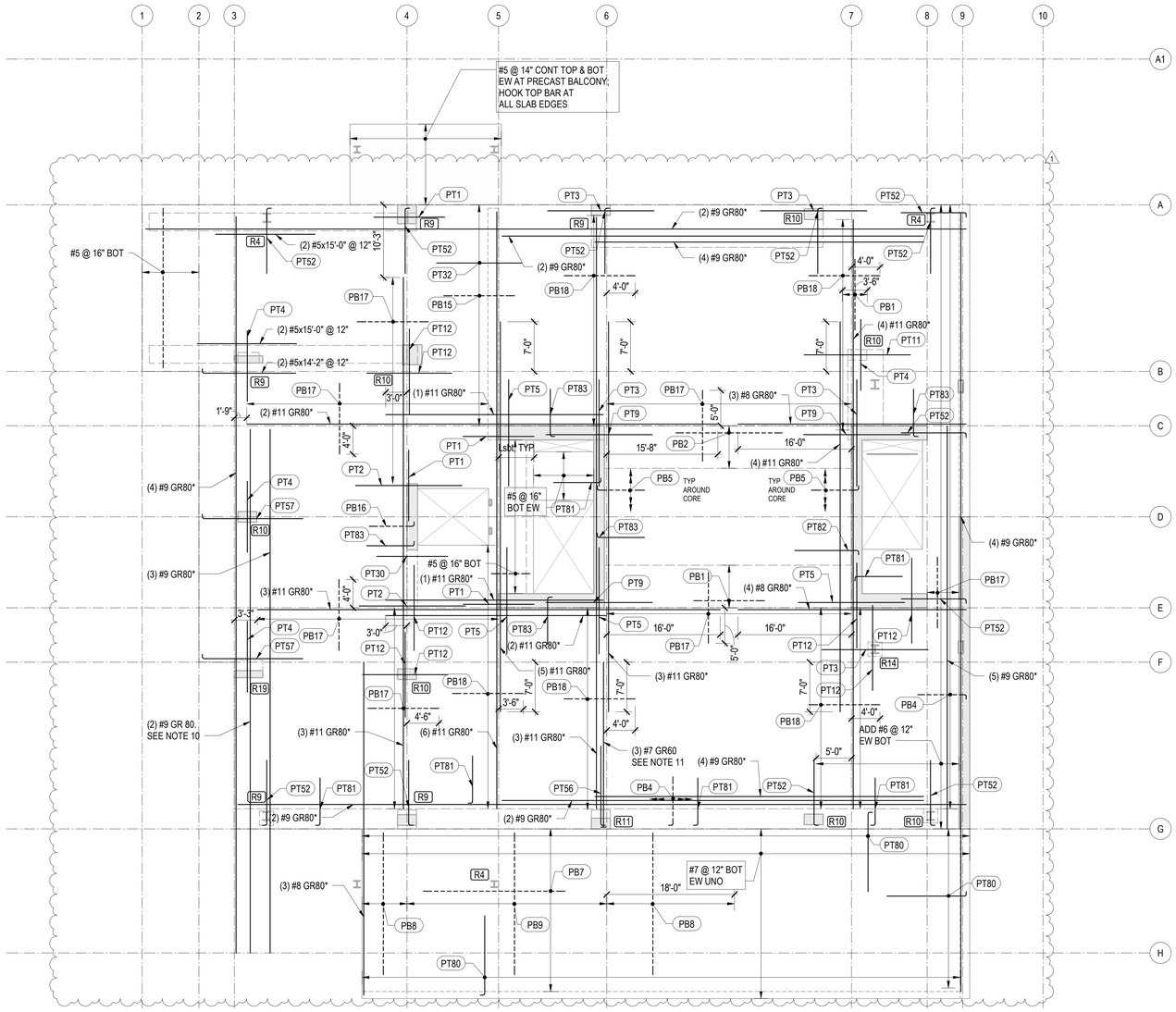
TOWER C LEVEL 2 FRAMING PLAN

S2.C.12



1 TOWER C - LEVEL 2 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.
9. * INDICATES DIAPHRAGM REINFORCEMENT THAT IS PART OF THE LATERAL FORCE RESISTING SYSTEM AND IS IN ADDITION TO OTHER BARS SHOWN. THIS REINFORCEMENT SHALL BE CENTERED IN SLAB MID-DEPTH, UNO. REINFORCEMENT SHALL MEET CENTER-TO-CENTER SPACING OF 3db BUT NOT LESS THAN 3-INCHES, UNLESS NOTED OTHERWISE. LAP Lsb AS REQUIRED, STAGGER LAPS.
10. WHERE NOTE APPLIES, REINFORCEMENT IS DIAPHRAGM REINFORCEMENT THAT IS PART OF THE LATERAL FORCE RESISTING SYSTEM AND IN ADDITION TO OTHER BARS SHOWN. REINFORCEMENT IS TO BE PLACED WITHIN VERTICALS OF COLUMN NEAR GRID 3/F. REINFORCEMENT SHALL BE CENTERED IN SLAB MID-DEPTH. REINFORCEMENT SHALL MEET CENTER-TO-CENTER SPACING OF 3db BUT NOT LESS THAN 3-INCHES, UNLESS NOTED OTHERWISE. LAP SPLICE IS NOT PERMITTED; PROVIDE MECHANICAL COUPLER IF NECESSARY.
11. WHERE NOTE APPLIES, REINFORCEMENT IS TO BE PLACED WITHIN VERTICALS OF COLUMN NEAR GRID 6/G, WITH TERMINATOR AT SOUTH END. REINFORCEMENT SHALL BE CENTERED IN SLAB MID-DEPTH. REINFORCEMENT SHALL MEET CENTER-TO-CENTER SPACING OF 3db BUT NOT LESS THAN 3-INCHES, UNLESS NOTED OTHERWISE. LAP SPLICE IS NOT PERMITTED; PROVIDE MECHANICAL COUPLER IF NECESSARY.

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(10) #5x14'-2"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0"@ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 6"	
PB4	#4x6'-10" @ 12"	HOOK AT END
PB5	#5x6'-8" @ 6"	HOOK AT END
PB7	#5x20'-0" @ 12"	
PB8	#7x20'-0" @ 12"	
PB9	#7x20'-0" @ 6"	
PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
PB17	#5x10'-0" @ 12"	
PB18	#7x10'-0" @ 12"	

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DEER VALLEY, UTAH

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Structural + Civil Engineers
Seattle Chicago
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206 292 1200

principal architect
project manager
drawn by Author
checked by Checker
job no. 20052
date 7/15/2022

revisions:

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1	7/15/2022 PROGRESS PRINT	

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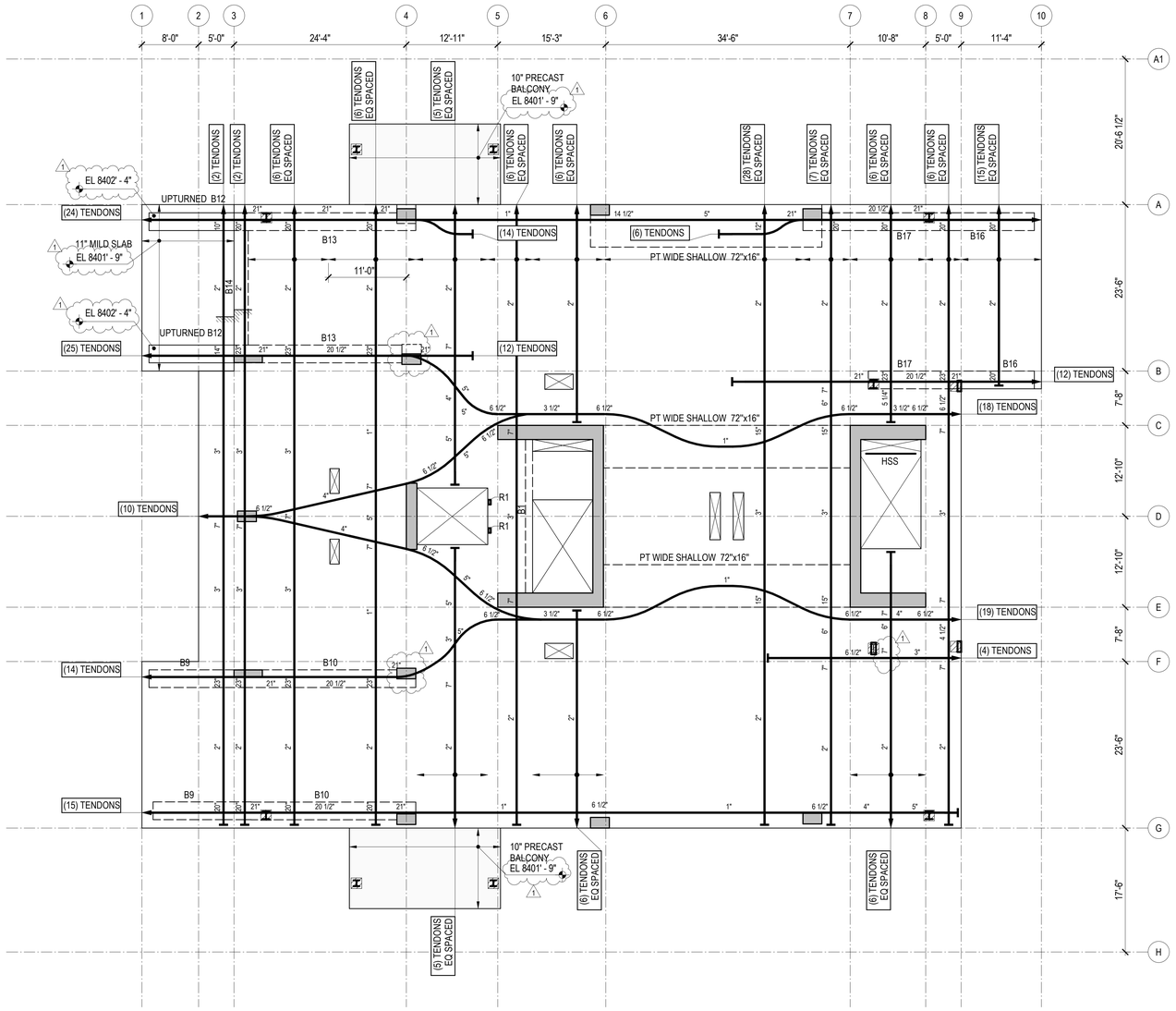
7/15/2022

TOWER C LEVEL 2 REINFORCING PLAN

S2.C.12.R



1 TOWER C - LEVEL 3 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8402' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8402' - 5", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMN DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.

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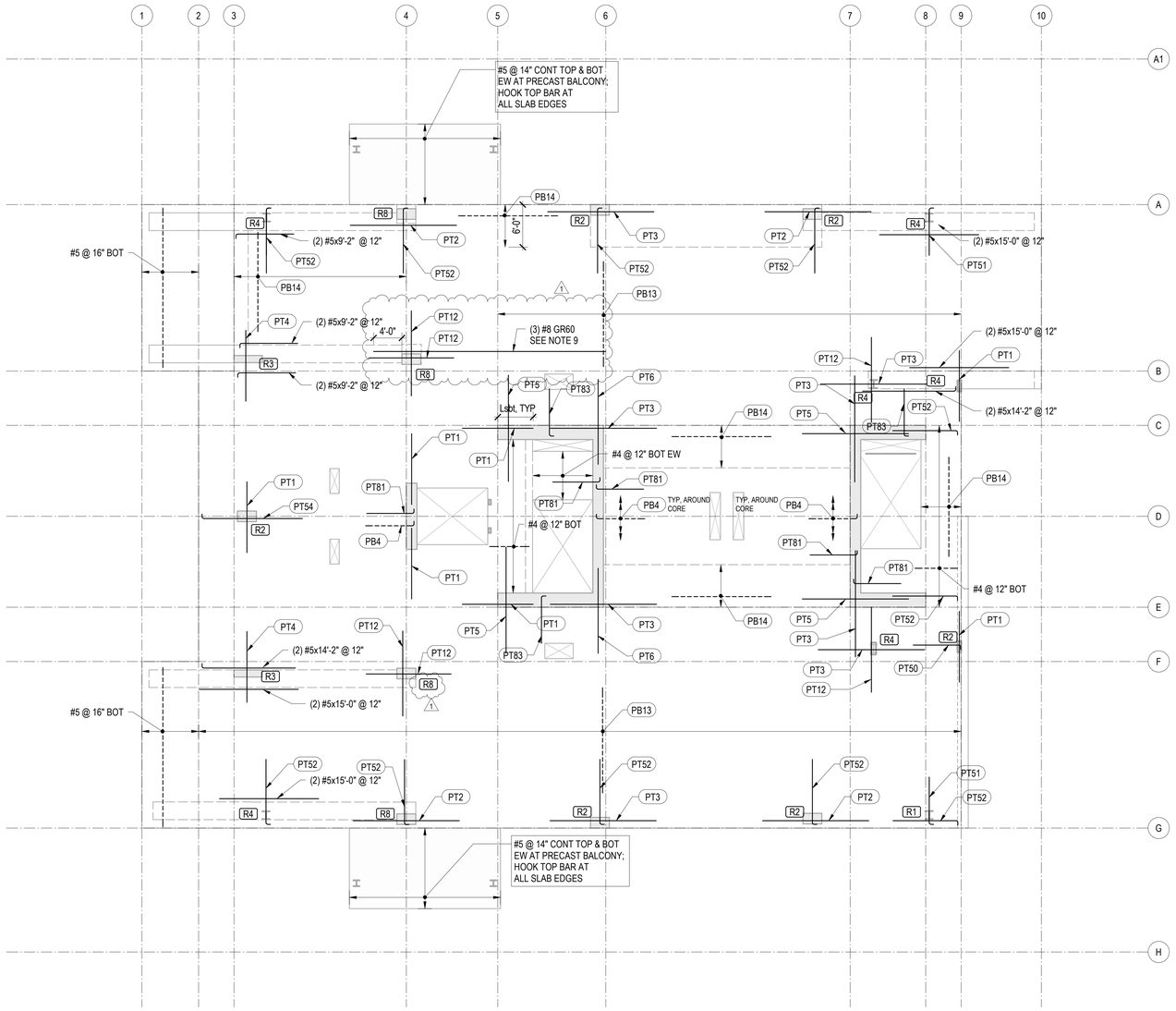
TOWER C LEVEL 3 FRAMING PLAN

S2.C.13



1 TOWER C - LEVEL 3 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

1

9. WHERE NOTE APPLIES, REINFORCEMENT IS TO BE PLACED WITHIN VERTICALS OF COLUMN NEAR GRID 4/B. REINFORCEMENT SHALL BE CENTERED IN SLAB MID-DEPTH. REINFORCEMENT SHALL MEET CENTER-TO-CENTER SPACING OF 3db BUT NOT LESS THAN 3-INCHES, UNLESS NOTED OTHERWISE. LAP SPLICE IS NOT PERMITTED; PROVIDE MECHANICAL COUPLER IF NECESSARY.

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(10) #5x14'-2"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0"@ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 6"	
PB4	#4x6'-10" @ 12"	HOOK AT END
PB5	#5x6'-8" @ 6"	HOOK AT END
PB7	#5x20'-0" @ 12"	
PB8	#7x20'-0" @ 12"	
PB9	#7x20'-0" @ 6"	
PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
PB17	#5x10'-0" @ 12"	
PB18	#7x10'-0" @ 12"	

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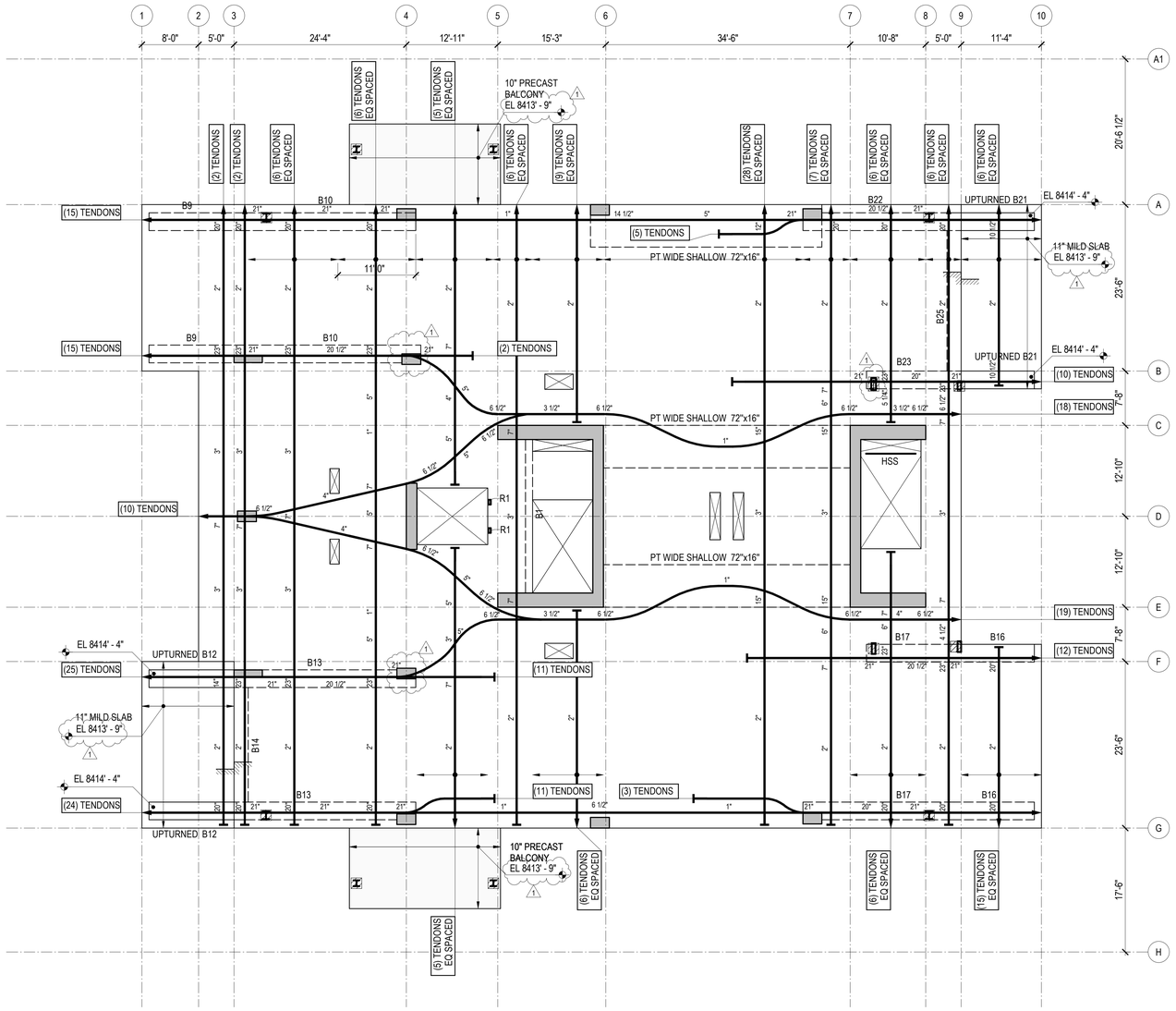
TOWER C LEVEL 3 REINFORCING PLAN

S2.C.13.R



1 TOWER C - LEVEL 4 FRAMING PLAN

1/8" = 1'-0"

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NOTES:

1. REFERENCE FLOOR ELEVATION IS 8414' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8414' - 5", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
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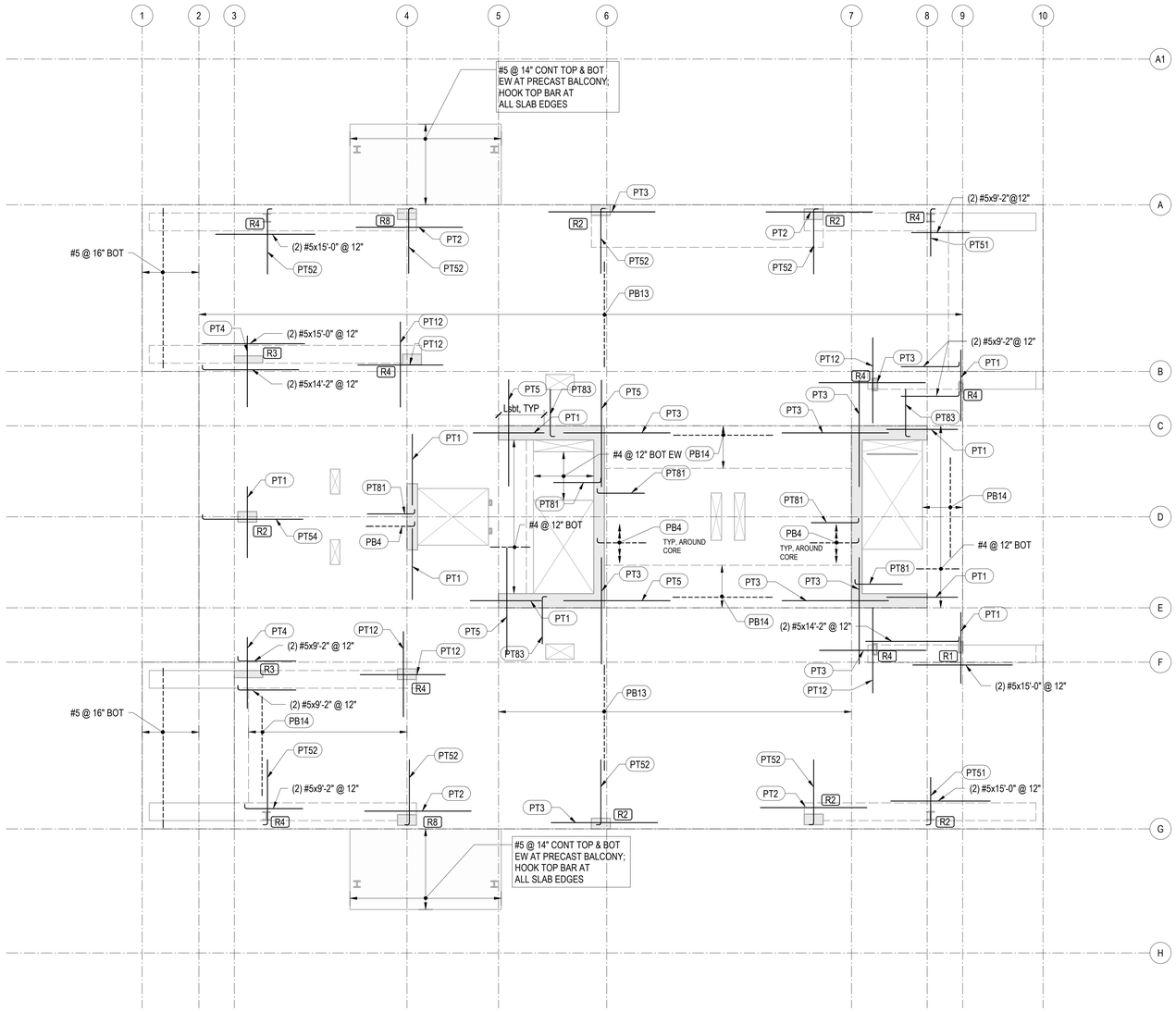
TOWER C LEVEL 4 FRAMING PLAN

S2.C.14



1 TOWER C - LEVEL 4 - REINFORCEMENT PLAN
1/8" = 1'-0"

REINFORCING NOTES:

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   BOT BARS IN DIRECTION OF BANDED TENDONS
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PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(10) #5x14'-2"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0"@ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 6"	
PB4	#4x6'-10" @ 12"	HOOK AT END
PB5	#5x6'-8" @ 6"	HOOK AT END
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PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
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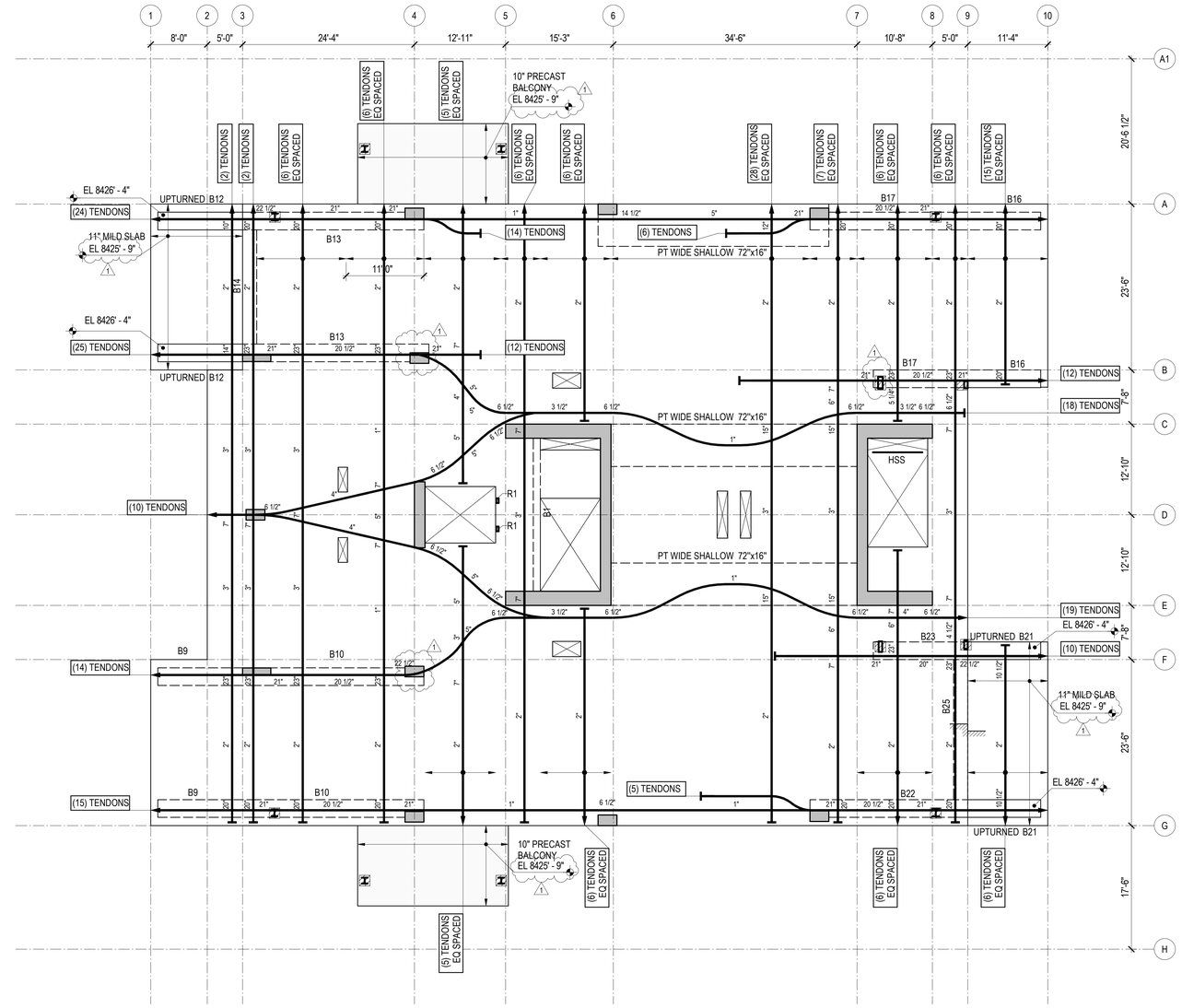
TOWER C LEVEL 4 REINFORCING PLAN

S2.C.14.R



1 TOWER C - LEVEL 5 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

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NOTES:

1. ⚠1 REFERENCE FLOOR ELEVATION IS 8426' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8426' - 5", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
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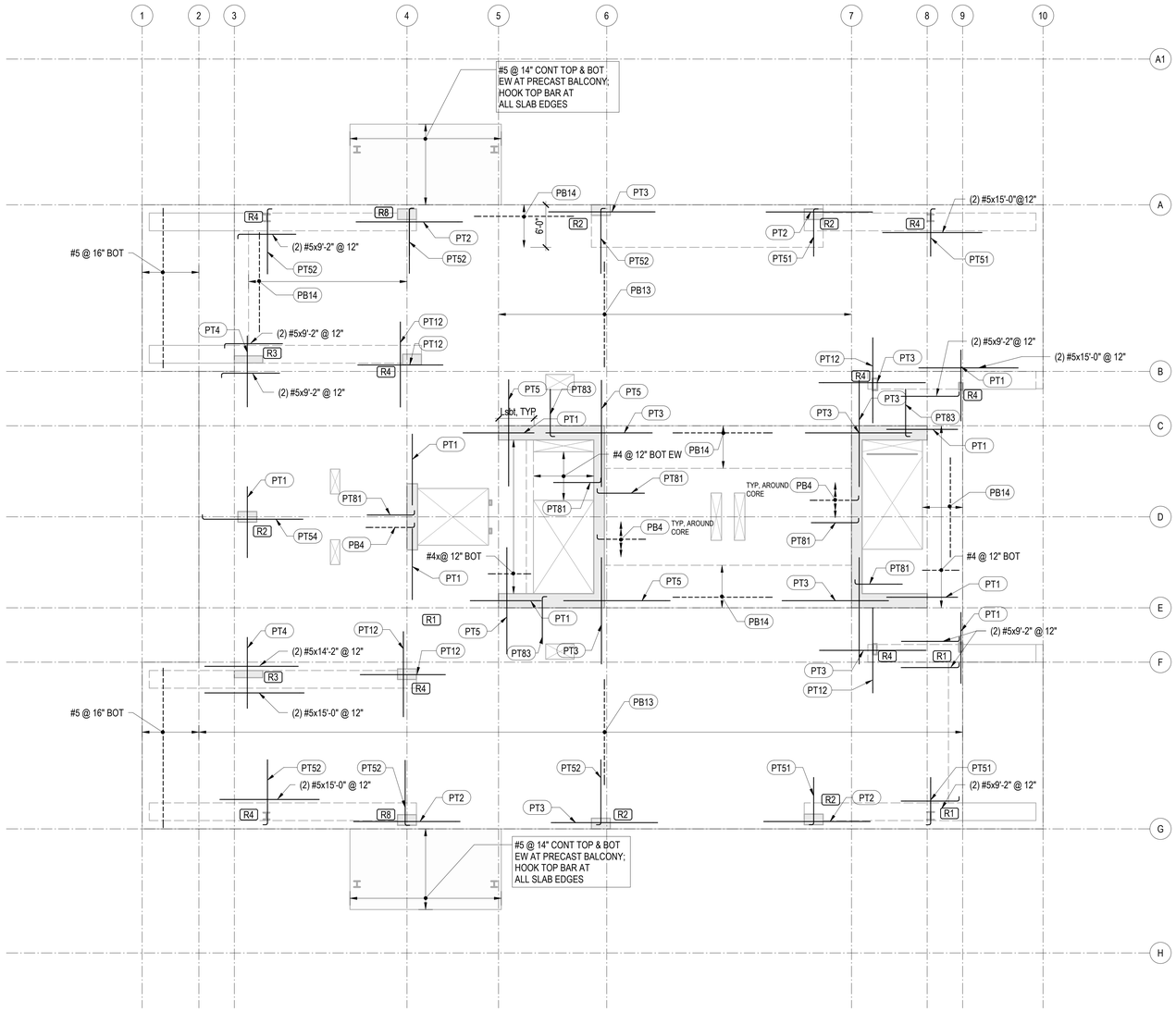
TOWER C LEVEL 5 FRAMING PLAN

S2.C.15



1 TOWER C - LEVEL 5 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
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   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
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PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
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PT80	#5x11'-2" @ 10"	HOOK AT END
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PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
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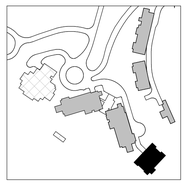
PB8	#7x20'-0" @ 12"	
PB9	#7x20'-0" @ 6"	
PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
PB17	#5x10'-0" @ 12"	
PB18	#7x10'-0" @ 12"	



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principal architect
project manager
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job no. 20052
date 7/15/2022

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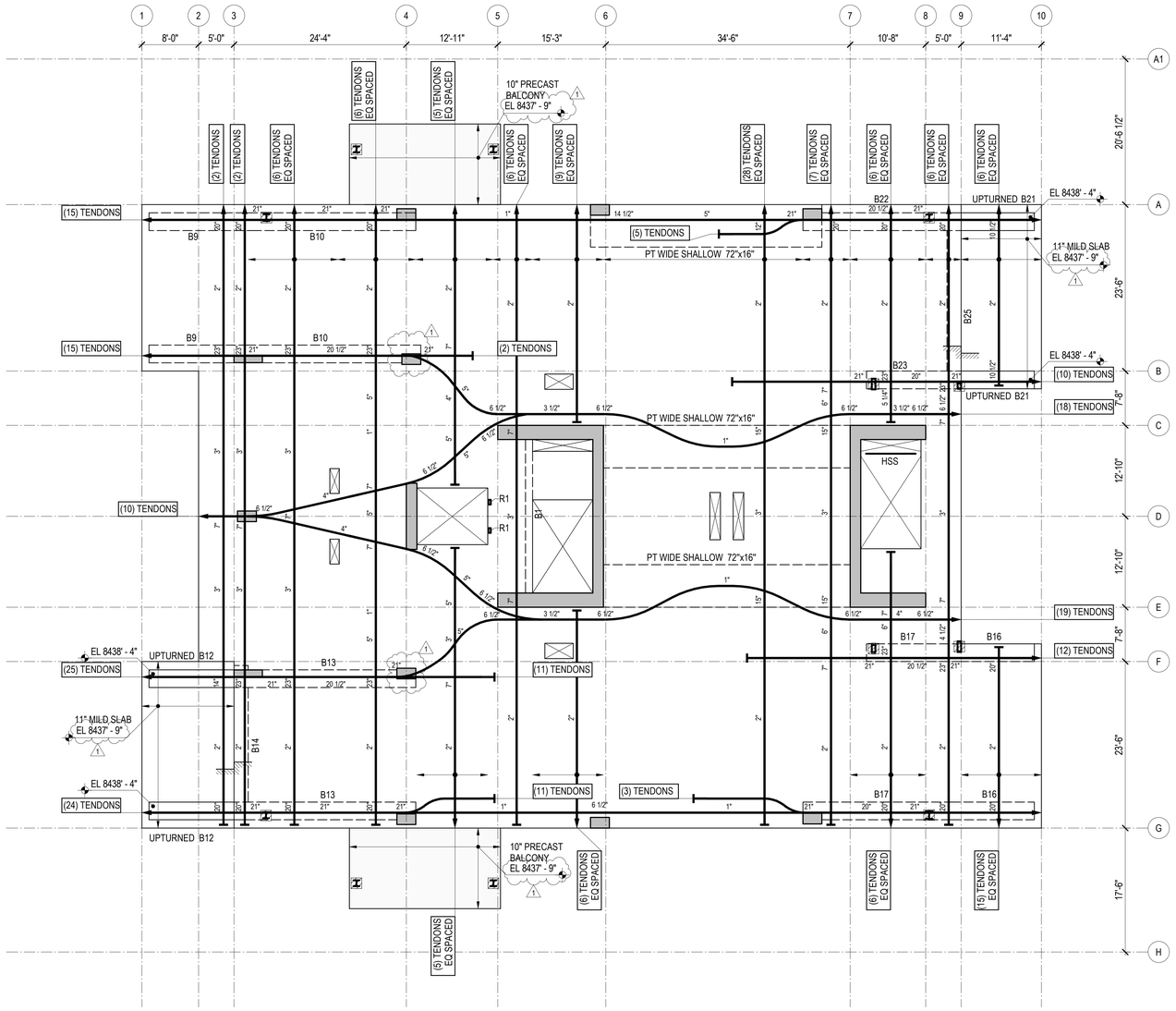
TOWER C LEVEL 5 REINFORCING PLAN

S2.C.15.R



1 TOWER C - LEVEL 6 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8438' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8438' - 5", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.

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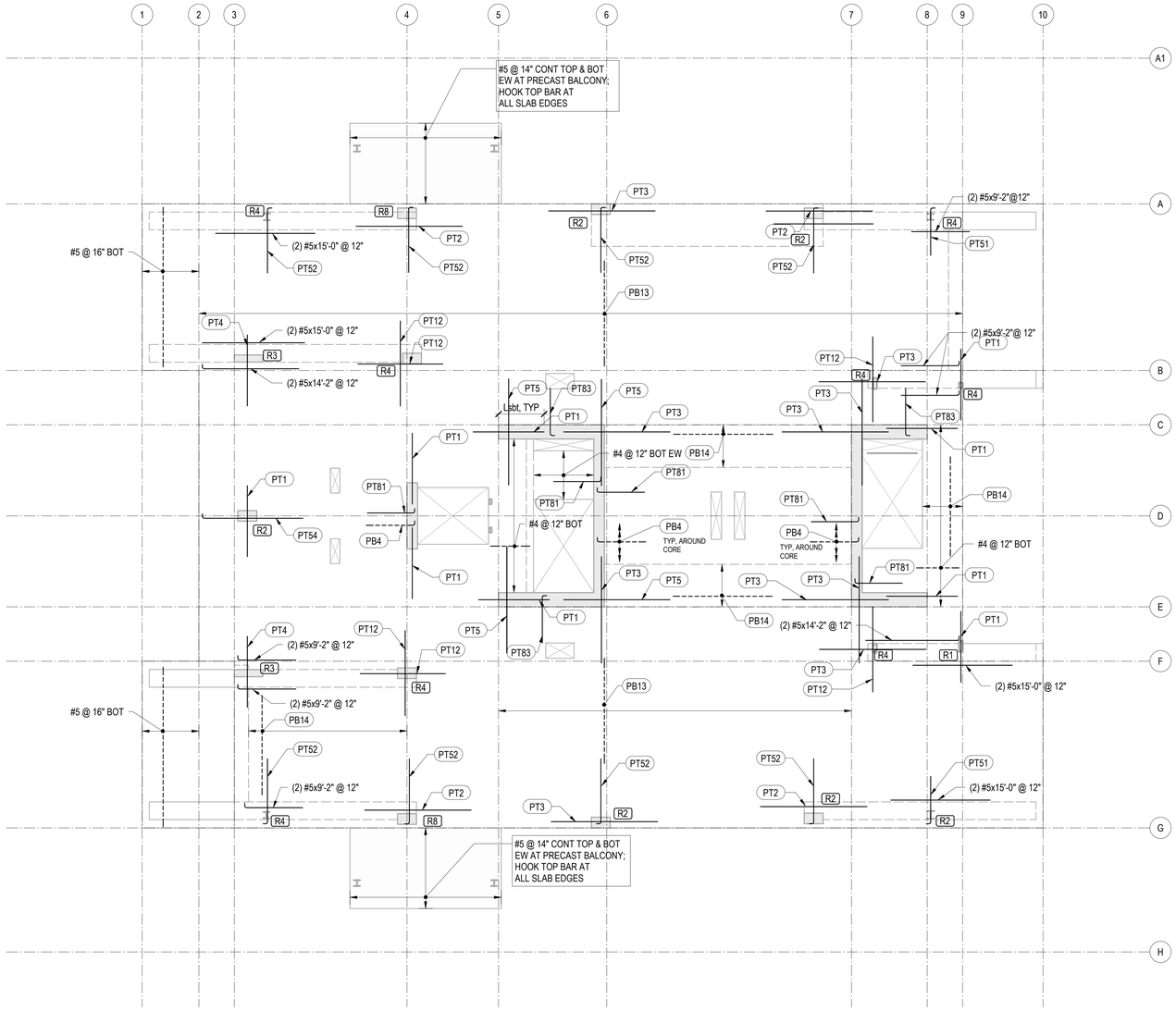
TOWER C LEVEL 6 FRAMING PLAN

S2.C.16



1 TOWER C - LEVEL 6 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

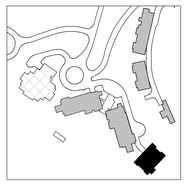
1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. (RX) INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(10) #5x14'-2"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0"@ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 6"	
PB4	#4x6'-10" @ 12"	HOOK AT END
PB5	#5x6'-8" @ 6"	HOOK AT END
PB7	#5x20'-0" @ 12"	
PB8	#7x20'-0" @ 12"	
PB9	#7x20'-0" @ 6"	
PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
PB17	#5x10'-0" @ 12"	
PB18	#7x10'-0" @ 12"	



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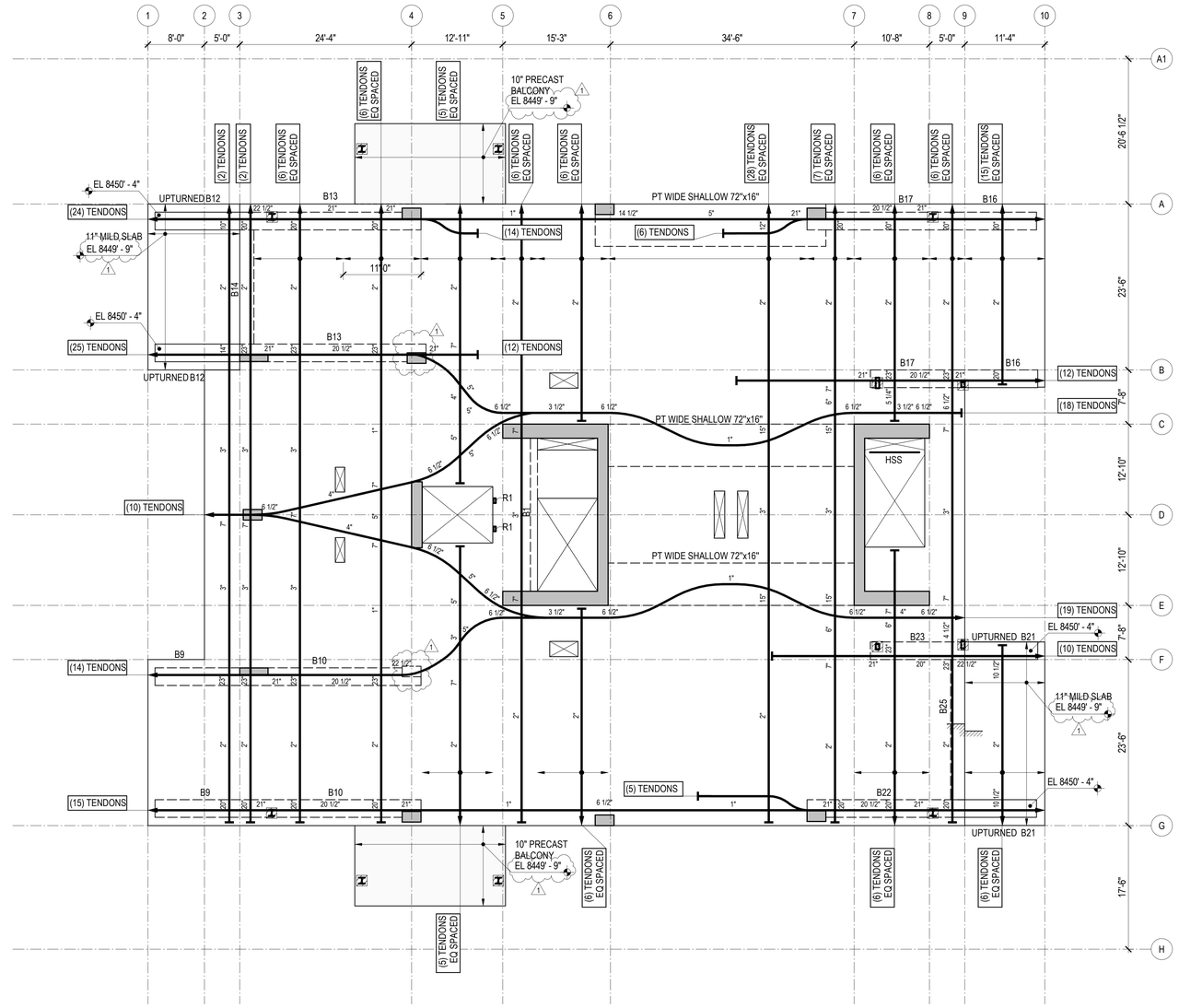
TOWER C LEVEL 6 REINFORCING PLAN

S2.C.16.R



1 TOWER C - LEVEL 7 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8450' - 6". TOP OF STRUCTURAL CONCRETE SLAB IS 8450' - 5", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
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9. ▨ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.

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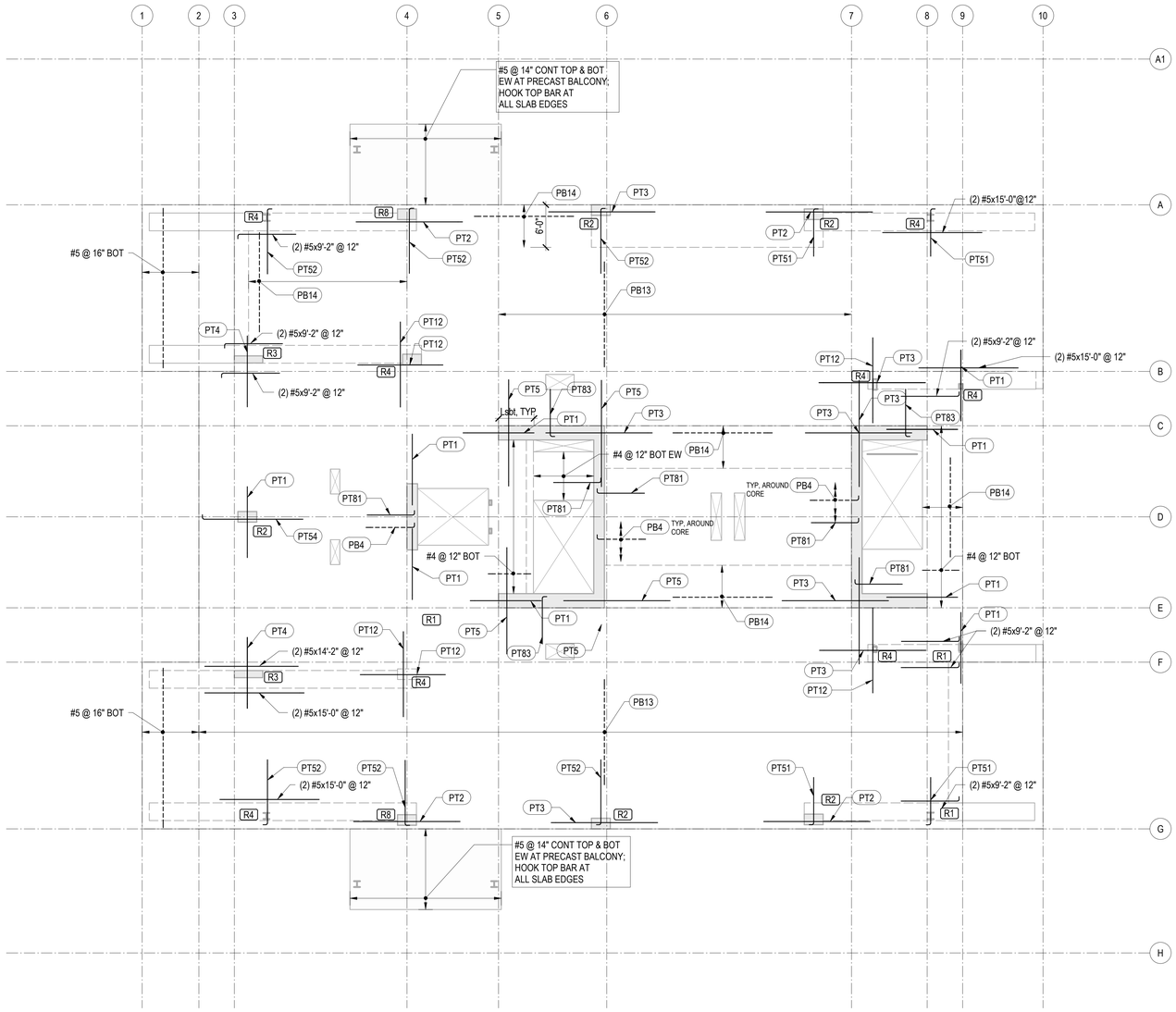
TOWER C LEVEL 7 FRAMING PLAN

S2.C.17



1 TOWER C - LEVEL 7 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
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   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
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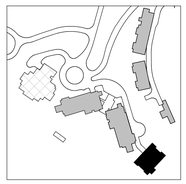
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(10) #5x14'-2"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0"@ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 6"	
PB4	#4x6'-10" @ 12"	HOOK AT END
PB5	#5x6'-8" @ 6"	HOOK AT END
PB7	#5x20'-0" @ 12"	
PB8	#7x20'-0" @ 12"	
PB9	#7x20'-0" @ 6"	
PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
PB17	#5x10'-0" @ 12"	
PB18	#7x10'-0" @ 12"	



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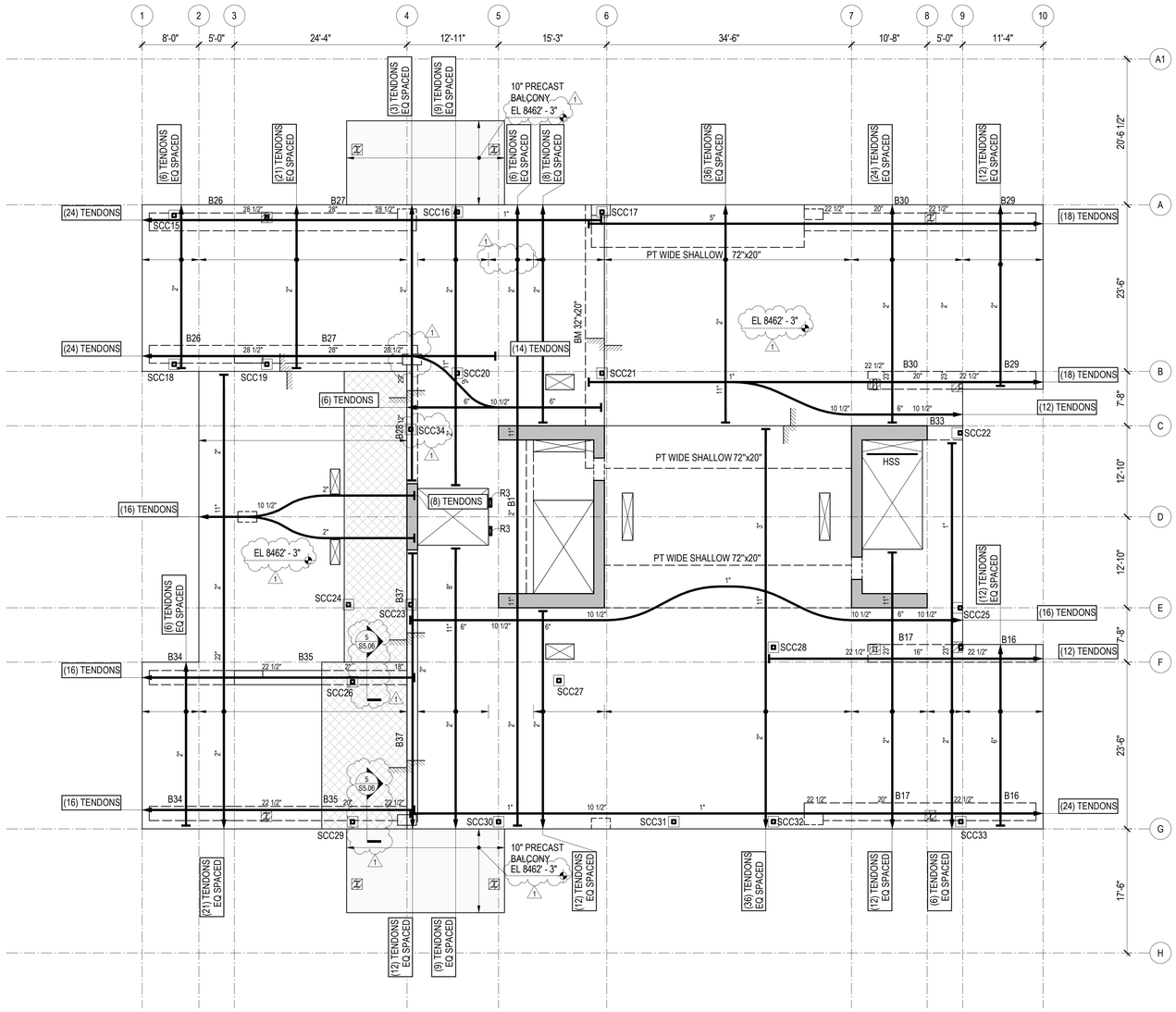
TOWER C LEVEL 7 REINFORCING PLAN

S2.C.17.R



1 TOWER C - LEVEL 8 FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8463' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8462' - 11"UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS A 12-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
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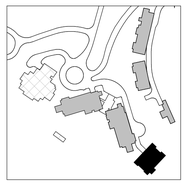
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. ▭ INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



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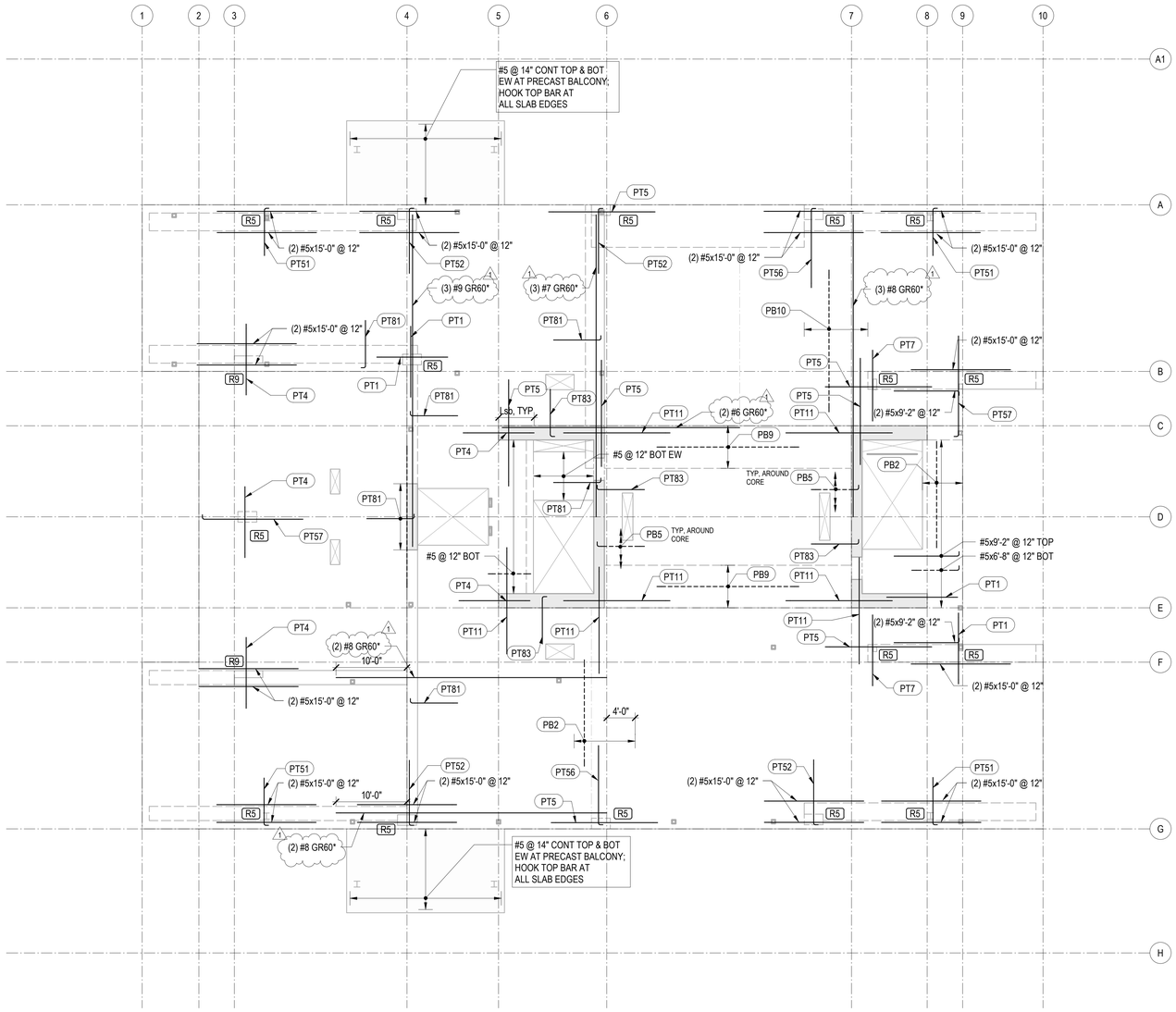
TOWER C LEVEL 8 FRAMING PLAN

S2.C.18



1 TOWER C - LEVEL 8 - REINFORCEMENT PLAN

1/8" = 1'-0"

REINFORCING NOTES:

1. SEE "GENERAL NOTES" FOR REINFORCING REQUIREMENTS.
2. SEE "TYPICAL POST-TENSIONED SLAB DETAILS" FOR ADDITIONAL INFORMATION.
3. SLAB REINFORCING SHALL BE PLACED IN THE FOLLOWING SEQUENCE:
   BOT BARS IN DIRECTION OF DISTRIBUTED TENDONS
   BOT BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF BANDED TENDONS
   TOP BARS IN DIRECTION OF DISTRIBUTED TENDONS
4. RX INDICATES STUD RAIL. STUD RAILS SHALL BE PLACED AT ALL COLUMNS. SEE "TYPICAL STUD RAIL REINFORCEMENT AT COLUMNS" DETAIL AND STUD RAIL SCHEDULE.
5. SEE "TYPICAL CONCRETE OPENINGS AND EMBEDMENTS" FOR ADDITIONAL REINFORCEMENT REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY. ADDITIONAL REINFORCEMENT MAY BE REQUIRED.
6. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK BAR.
7. WHERE NOTES AS "HOOKED", PROVIDE 90 OR 180 DEGREE HOOK AS SHOWN ON PLAN. NOTED BAR LENGTH IS LENGTH OF STRAIGHT PORTION OF BAR.
8. PROVIDE INTEGRITY BOTTOM BARS PER STUD RAIL SCHEDULE AT ALL COLUMNS. CENTER REINFORCEMENT ON COLUMNS AND PLACE INTEGRITY BARS EACH WAY WITHIN COLUMN VERTICAL REINFORCEMENT. TRIM AND HOOK AT SLAB EDGE AS REQUIRED.
9. * INDICATES DIAPHRAGM REINFORCEMENT THAT IS PART OF THE LATERAL FORCE RESISTING SYSTEM AND IS IN ADDITION TO OTHER BARS SHOWN. THIS REINFORCEMENT SHALL BE CENTERED IN SLAB MID-DEPTH, UNO. REINFORCEMENT SHALL MEET CENTER-TO-CENTER SPACING OF 3db BUT NOT LESS THAN 3-INCHES, UNLESS NOTED OTHERWISE. LAP Lsb AS REQUIRED, STAGGER LAPS.

PT TOP REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PT1	(6) #5x10'-0"	
PT2	(6) #5x15'-0"	
PT3	(8) #5x15'-0"	
PT4	(12) #5x10'-0"	
PT5	(10) #5x15'-0"	
PT6	(18) #6x12'-0" @ 5"	STAGGER 3'-0"
PT7	(14) #5x10'-0"	
PT9	(13)#6x15'-0"	
PT11	(13) #6x15'-0"	
PT12	(10) #5x12'-0"	
PT30	#5x10'-0"@ 15"	
PT32	#6x12'-0"@ 6"	
PT50	(4) #5x6'-8"	HOOK AT END
PT51	(6) #5x6'-8"	HOOK AT END
PT52	(10) #5x9'-2"	HOOK AT END
PT54	(6) #5x14'-2"	HOOK AT END
PT56	(16) #5x11'-2"	HOOK AT END
PT57	(10) #5x14'-2"	HOOK AT END
PT80	#5x11'-2" @ 10"	HOOK AT END
PT81	#5x6'-8" @ 10"	HOOK AT END
PT82	#6x9'-0"@ 4"	HOOK AT END
PT83	#6x9'-0" @ 6"	HOOK AT END

PT BOTTOM REINFORCEMENT SCHEDULE

MARK	REINFORCING	REMARKS
PB1	#5x10'-0" @ 6"	
PB2	#5x15'-0" @ 6"	
PB4	#4x6'-10" @ 12"	HOOK AT END
PB5	#5x6'-8" @ 6"	HOOK AT END
PB7	#5x20'-0" @ 12"	
PB8	#7x20'-0" @ 12"	
PB9	#7x20'-0" @ 6"	
PB10	#6x20'-0" @ 6"	
PB13	#5x15'-0" @ 24"	
PB14	#5x15'-0" @ 12"	
PB15	#7x10'-0" @ 8"	
PB16	#7x6'-4" @ 8"	HOOK AT END
PB17	#5x10'-0" @ 12"	
PB18	#7x10'-0" @ 12"	

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date 7/15/2022

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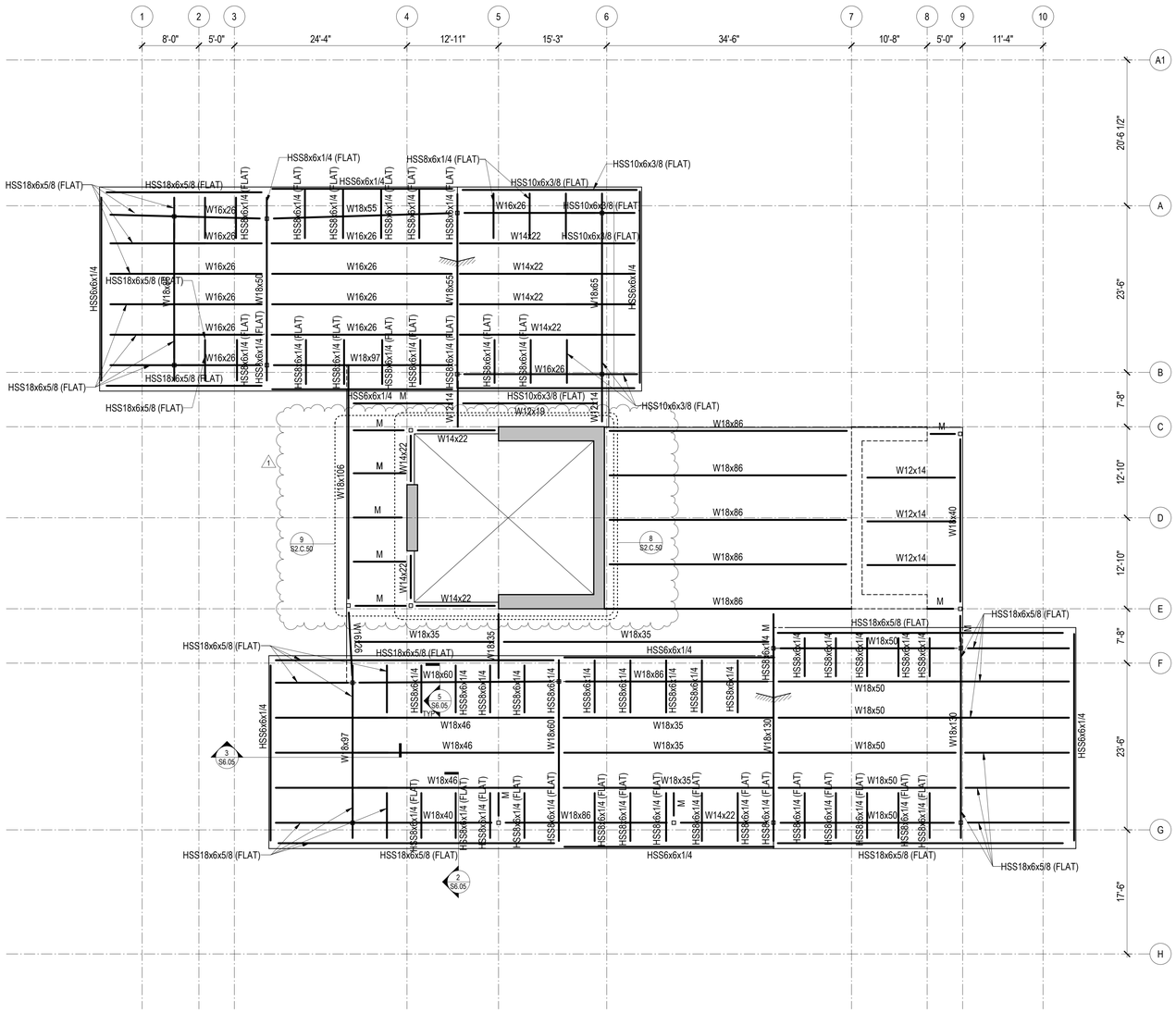
TOWER C LEVEL 8 REINFORCING PLAN

S2.C.18.R



1 TOWER C - ROOF LEVEL FRAMING PLAN

1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

1. REFERENCE FLOOR ELEVATION IS 8475' - 0". TOP OF SLAB ON STEEL DECK IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.

2. ROOF DECK IS MINIMUM 3 INCH X 20 GAUGE STEEL DECK. DECKING IS TO BE INSTALLED IN MINIMUM TWO SPAN CONDITION WHERE POSSIBLE. DECKING CONNECTIONS ARE TO BE AS FOLLOWS:

   SIDE LAP SEAM CONNECTIONS ARE 1-1/2 INCH TOP SEAM WELDS AT 18 INCHES MAXIMUM.

   CONNECTION TO SUPPORTING FRAMING SHALL BE A 24/4 WELD PATTERN WITH 3/4 INCH PUDDLE WELDS.

   CONNECTION TO FRAMING BELOW AND IN LINE WITH FLUTE SHALL BE 3/4 INCH PUDDLE WELDS AT 18 INCHES MAXIMUM.

   BUTTON PUNCHING OF SIDE LAP CONNECTIONS IS NOT PERMITTED.

3. REFERENCE TOP OF STEEL IS AT THE BOTTOM OF SLAB ON STEEL DECK UNLESS NOTED OTHERWISE.

4. STEEL SLOPES UNIFORMLY BETWEEN GIVEN TOP OF STEEL ELEVATIONS. WHERE BEAMS OR BEAMS AND COLUMNS INTERSECT, MATCH TOP OF STEEL UNLESS NOTED OTHERWISE.

5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING SLABS AND WALLS.

6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE 'TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE,' 'TYPICAL ROOF DECK OPENINGS,' AND 'TYPICAL COMPOSITE DECK OPENINGS' DETAILS FOR OPENING PLACEMENT CRITERIA AND REINFORCING OR FRAMING REQUIREMENTS. NOTIFY STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.

7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, EDGE ANGLES, AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

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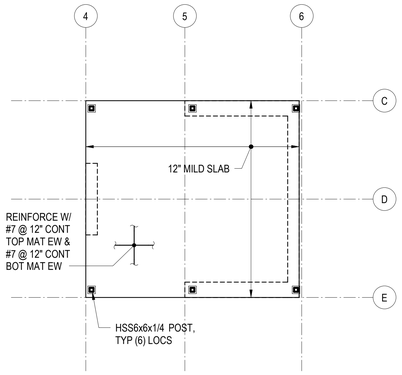
TOWER C ROOF LEVEL FRAMING PLAN

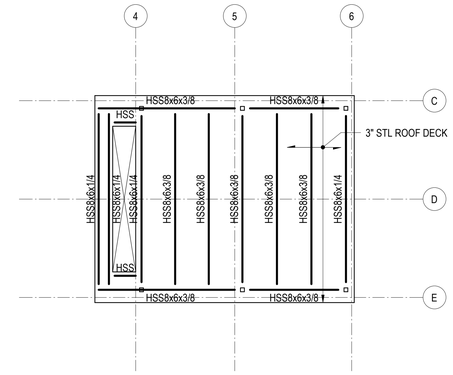
S2.C.19



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8482' - 2". TOP OF CONCRETE SLAB IS AT THE REFERENCE ELEVATION UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 12-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
4. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
5. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.

8 PARTIAL FRAMING PLAN - ELEVATOR OVERRUN
1/8" = 1'-0"



NOTES:

1. REFERENCE FLOOR ELEVATION IS 8485'-3". REFERENCE TOP OF STRUCTURAL STEEL IS 3-INCHES BELOW THE REFERENCE FLOOR ELEVATION, TYPICAL UNLESS NOTED OTHERWISE.
2. ROOF DECK IS MINIMUM 3-INCH x 20 GAUGE STEEL DECKING. DECKING IS TO BE INSTALLED IN MINIMUM THREE SPAN CONDITIONS WHERE POSSIBLE.

9 PARTIAL PLAN - TOP OF CORE
1/8" = 1'-0"

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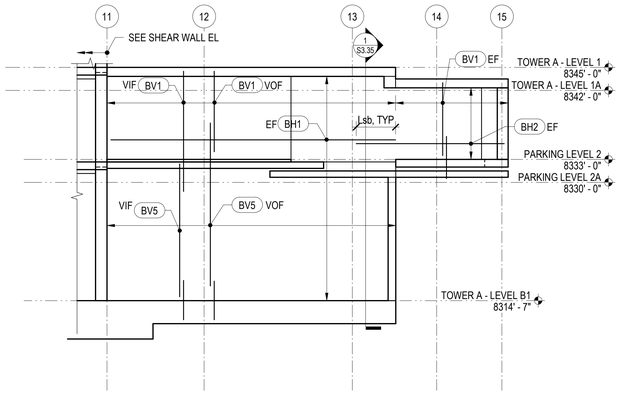
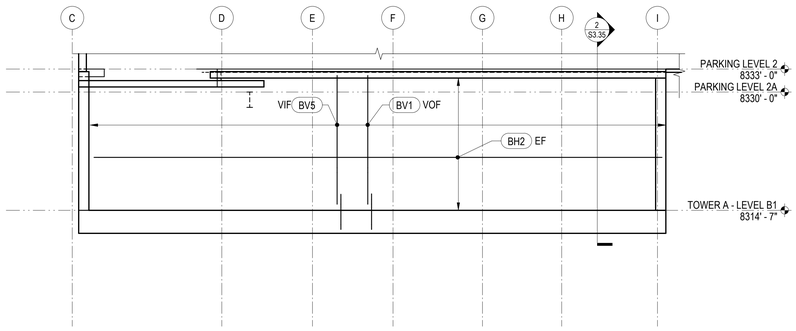
TOWER C PARTIAL PLANS

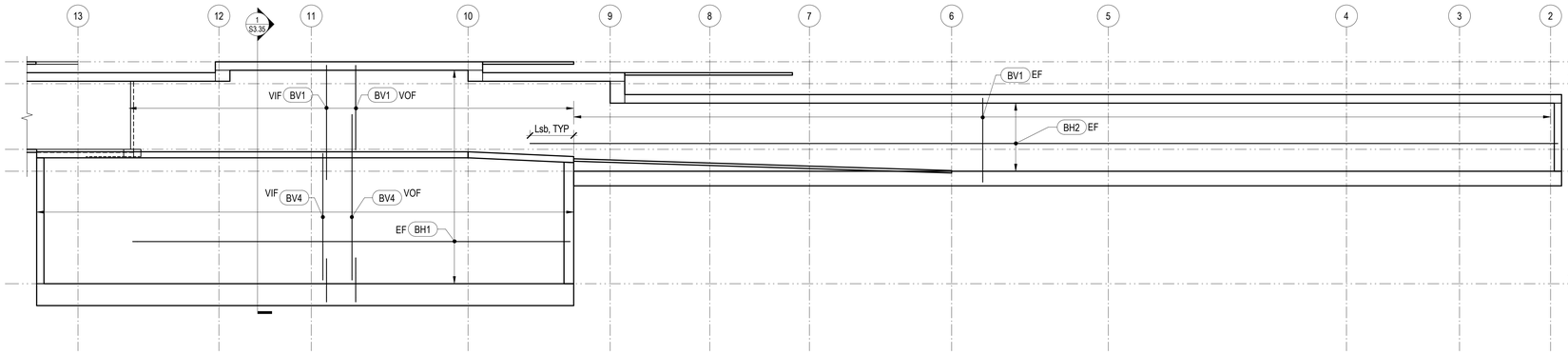
S2.C.50



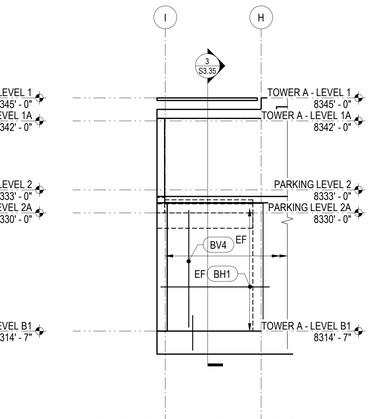
1 BASEMENT WALL ELEVATION
1/8" = 1'-0"



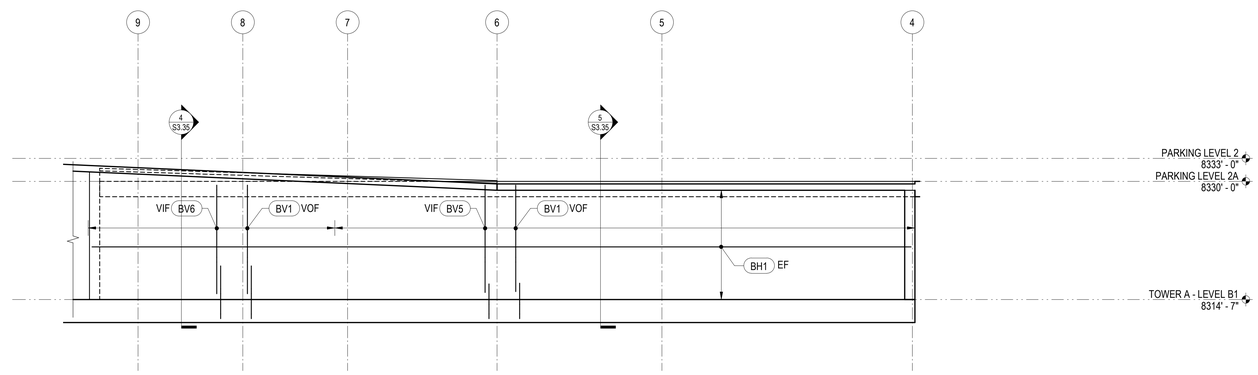
2 BASEMENT WALL ELEVATION
1/8" = 1'-0"



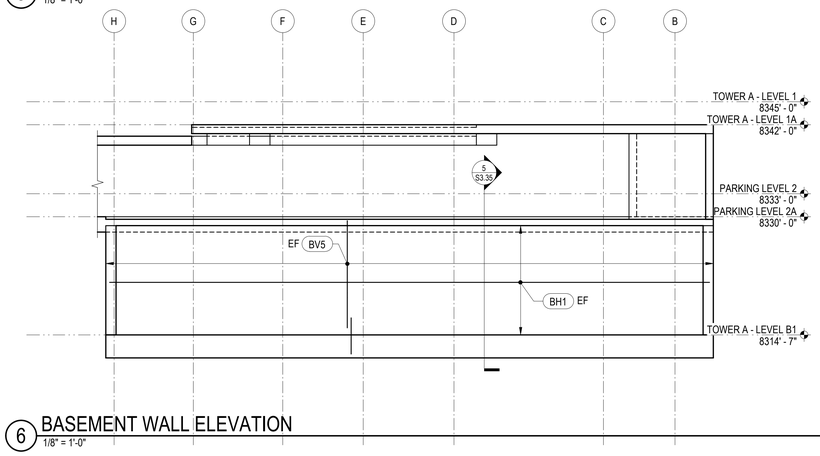
3 BASEMENT WALL ELEVATION
1/8" = 1'-0"



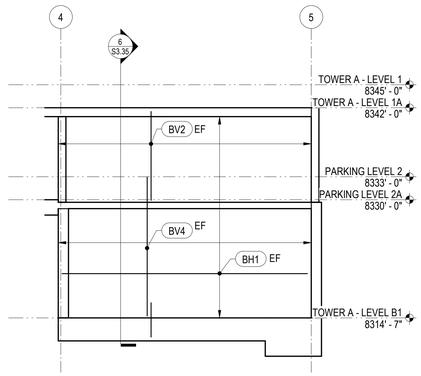
4 BASEMENT WALL ELEVATION
1/8" = 1'-0"



5 BASEMENT WALL ELEVATION
1/8" = 1'-0"



6 BASEMENT WALL ELEVATION
1/8" = 1'-0"



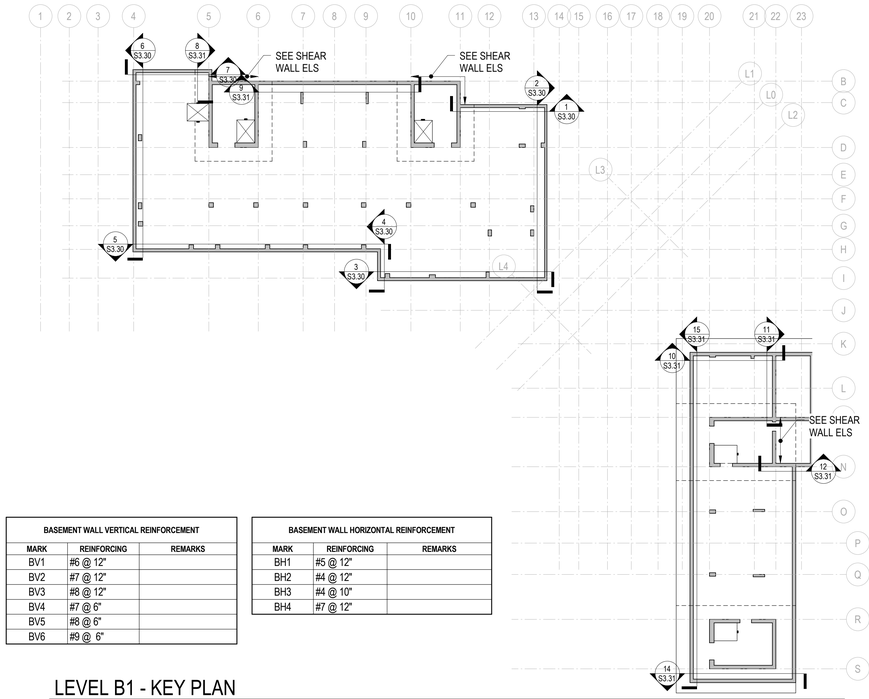
7 BASEMENT WALL ELEVATION
1/8" = 1'-0"

BASEMENT WALL VERTICAL REINFORCEMENT

MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV3	#8 @ 12"	
BV4	#7 @ 6"	
BV5	#8 @ 6"	
BV6	#9 @ 6"	

BASEMENT WALL HORIZONTAL REINFORCEMENT

MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	
BH3	#4 @ 10"	
BH4	#7 @ 12"	



LEVEL B1 - KEY PLAN

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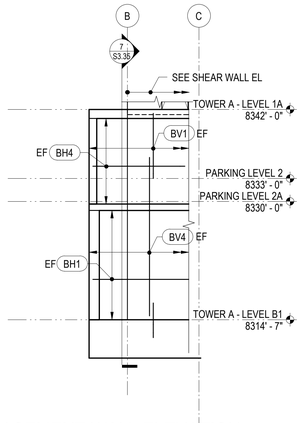
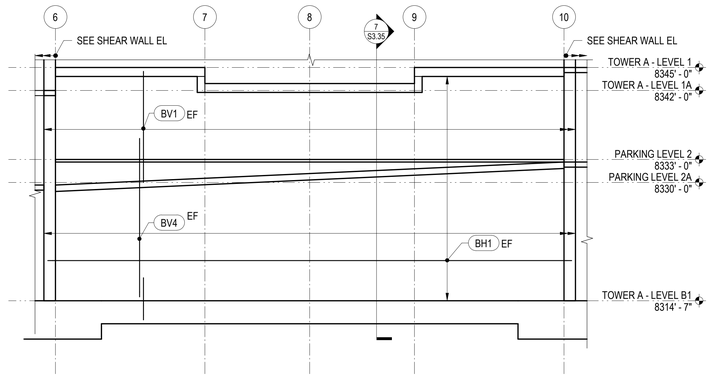
TOWER A & B
BASEMENT WALL
ELEVATIONS

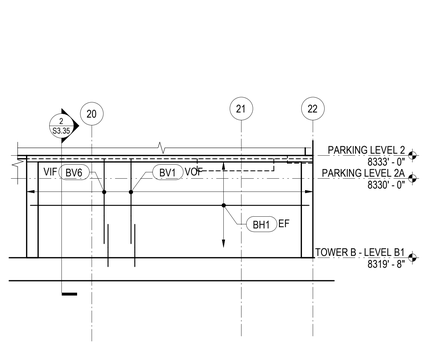
S3.30



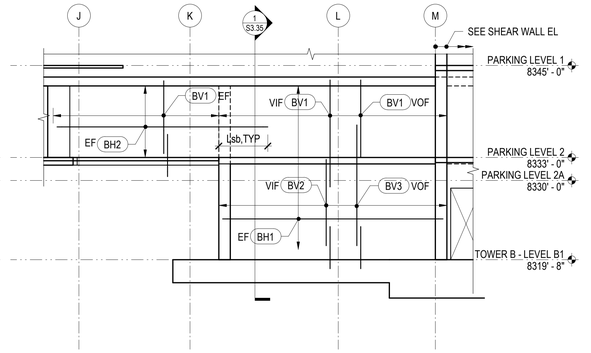
8 BASEMENT WALL ELEVATION
1/8" = 1'-0"



9 BASEMENT WALL ELEVATION
1/8" = 1'-0"



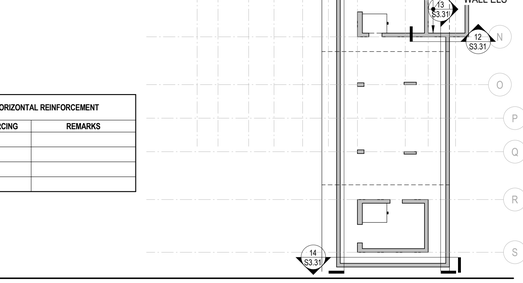
10 BASEMENT WALL ELEVATION
1/8" = 1'-0"



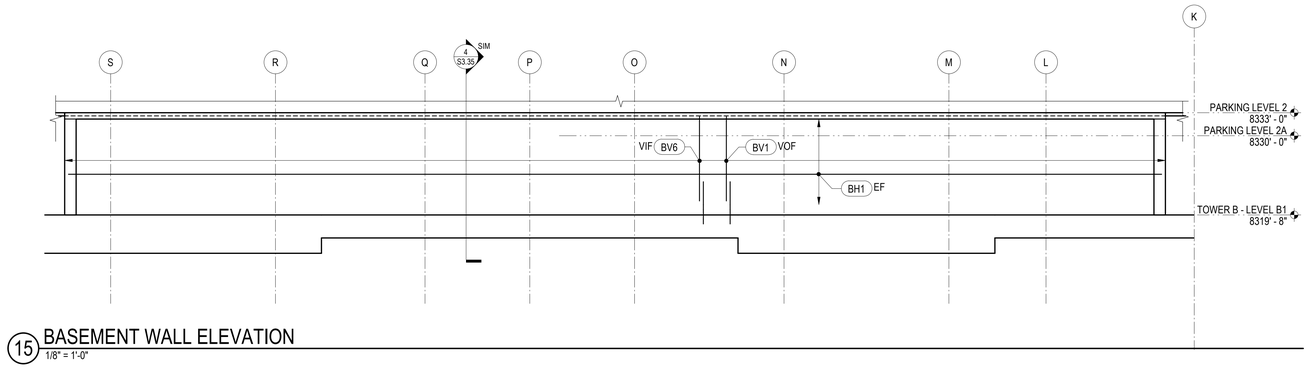
11 BASEMENT WALL ELEVATION
1/8" = 1'-0"

12 BASEMENT WALL ELEVATION
1/8" = 1'-0"

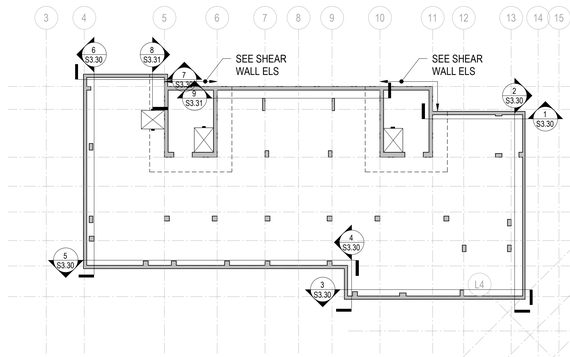
13 BASEMENT WALL ELEVATION
1/8" = 1'-0"



14 BASEMENT WALL ELEVATION
1/8" = 1'-0"



15 BASEMENT WALL ELEVATION
1/8" = 1'-0"



LEVEL B1 - KEY PLAN

BASEMENT WALL VERTICAL REINFORCEMENT

MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV3	#8 @ 12"	
BV4	#7 @ 6"	
BV5	#8 @ 6"	
BV6	#9 @ 6"	

BASEMENT WALL HORIZONTAL REINFORCEMENT

MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	
BH3	#4 @ 10"	
BH4	#7 @ 12"	

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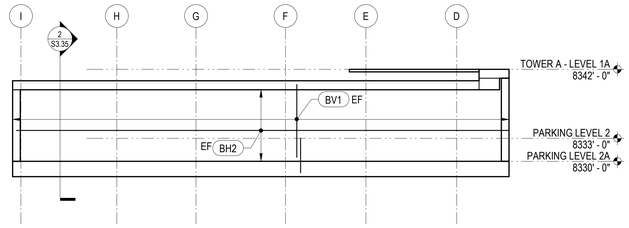
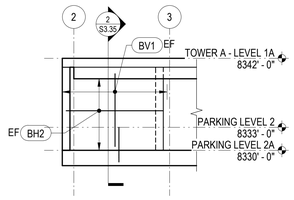
TOWER A & B
BASEMENT WALL
ELEVATIONS

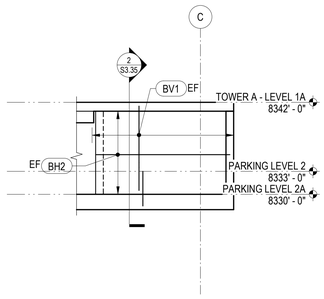
S3.31



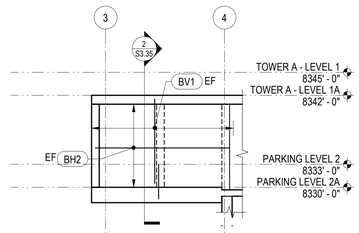
16 BASEMENT WALL ELEVATION
1/8" = 1'-0"



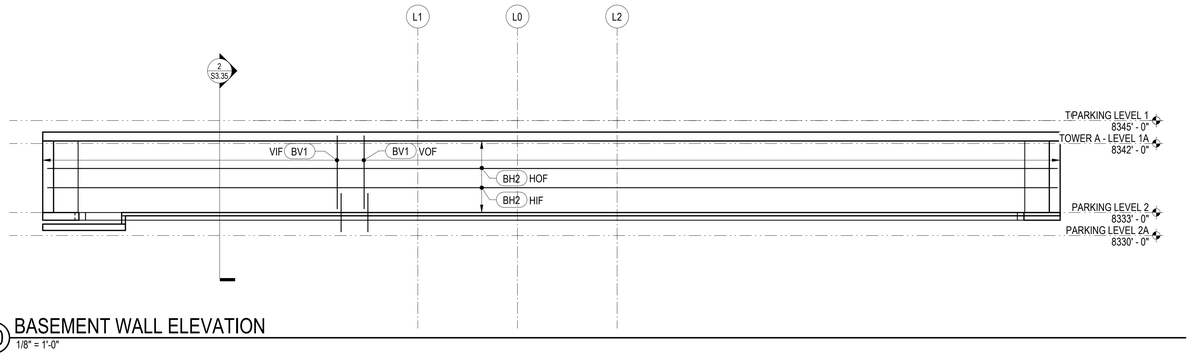
17 BASEMENT WALL ELEVATION
1/8" = 1'-0"



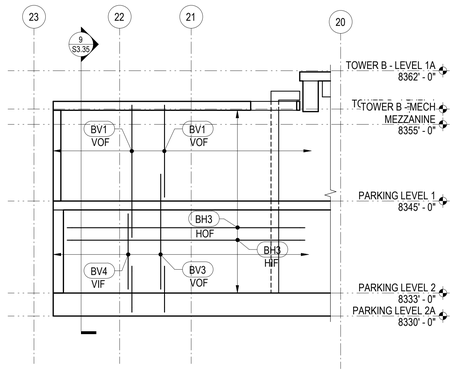
18 BASEMENT WALL ELEVATION
1/8" = 1'-0"



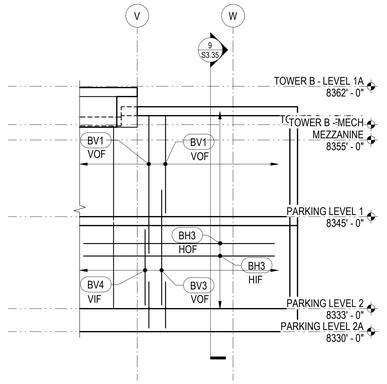
19 BASEMENT WALL ELEVATION
1/8" = 1'-0"



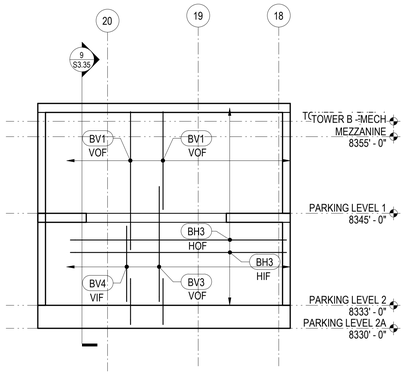
20 BASEMENT WALL ELEVATION
1/8" = 1'-0"



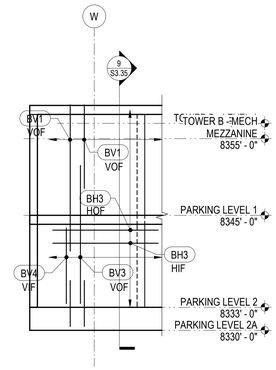
21 BASEMENT WALL ELEVATION
1/8" = 1'-0"



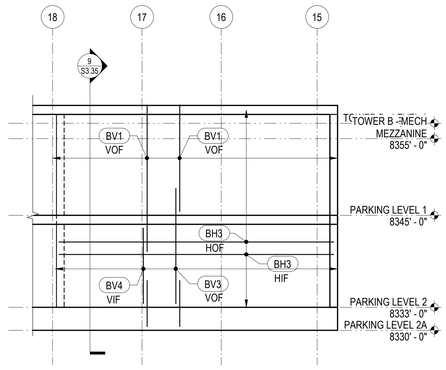
22 BASEMENT WALL ELEVATION
1/8" = 1'-0"



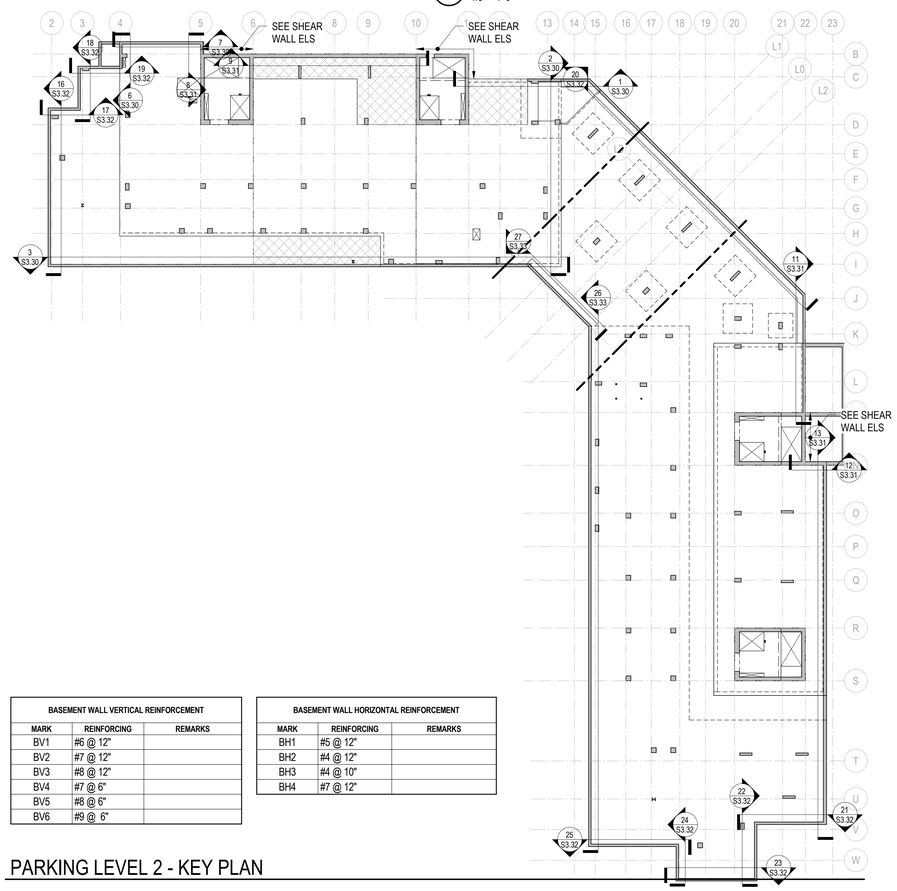
23 BASEMENT WALL ELEVATION
1/8" = 1'-0"



24 BASEMENT WALL ELEVATION
1/8" = 1'-0"



25 BASEMENT WALL ELEVATION
1/8" = 1'-0"



PARKING LEVEL 2 - KEY PLAN

BASEMENT WALL VERTICAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV3	#8 @ 12"	
BV4	#7 @ 6"	
BV5	#8 @ 6"	
BV6	#9 @ 6"	

BASEMENT WALL HORIZONTAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	
BH3	#4 @ 10"	
BH4	#7 @ 12"	

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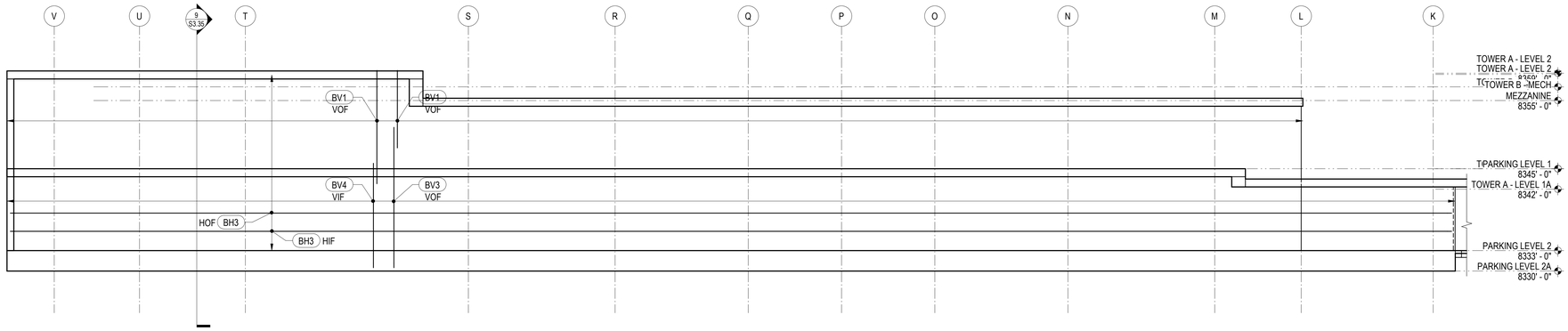
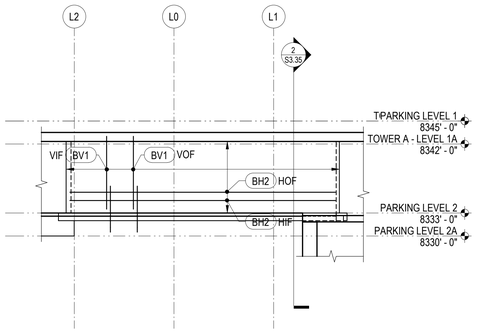
TOWER A & B
BASEMENT WALL
ELEVATIONS

S3.32



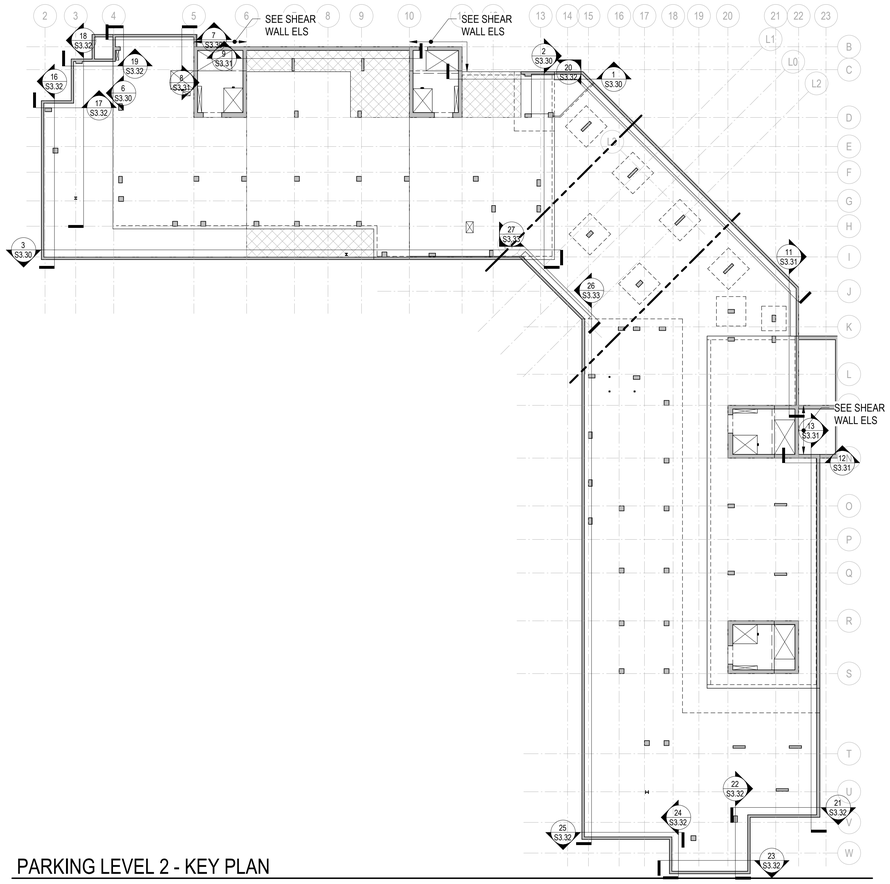
26 BASEMENT WALL ELEVATION

1/8" = 1'-0"



27 BASEMENT WALL ELEVATION

1/8" = 1'-0"



PARKING LEVEL 2 - KEY PLAN

BASEMENT WALL VERTICAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV3	#8 @ 12"	
BV4	#7 @ 6"	
BV5	#8 @ 6"	
BV6	#9 @ 6"	

BASEMENT WALL HORIZONTAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	
BH3	#4 @ 10"	
BH4	#7 @ 12"	

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principal architect
project manager
drawn by Author
checked by Checker
job no. 20052
date 7/15/2022

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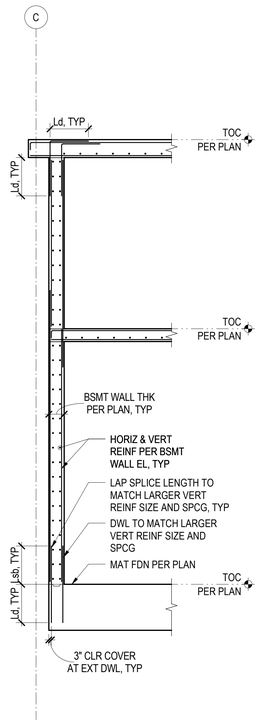
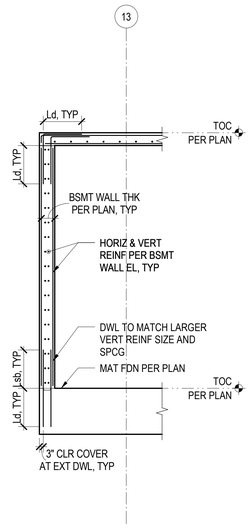
TOWER A & B
BASEMENT WALL
ELEVATIONS

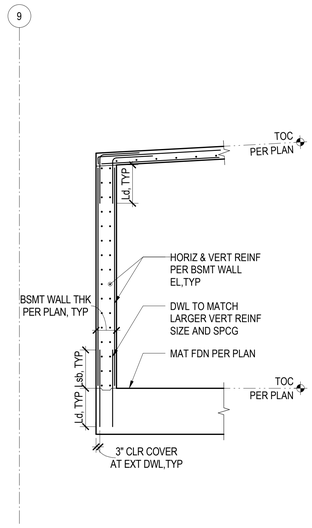
S3.33



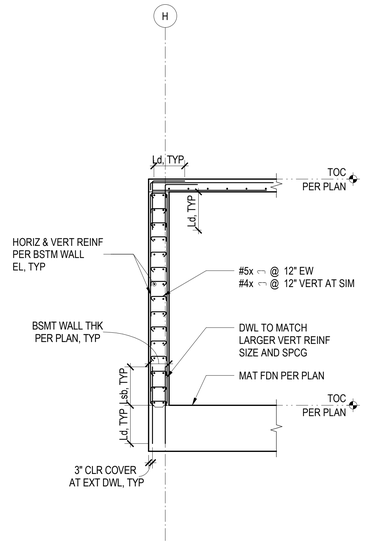
1 BASEMENT WALL SECTION
1/4" = 1'-0"



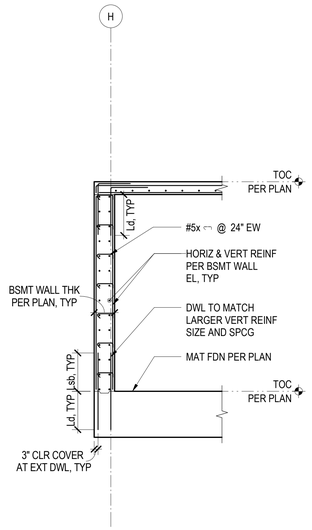
2 BASEMENT WALL SECTION
1/4" = 1'-0"



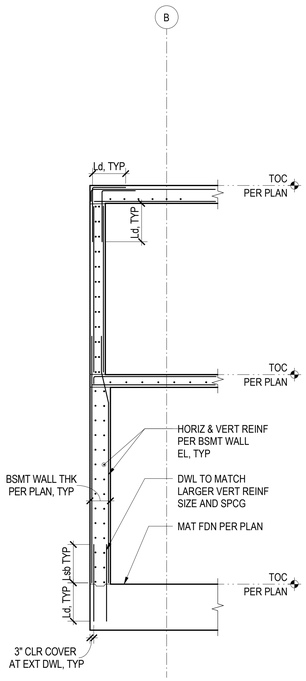
3 BASEMENT WALL SECTION
1/4" = 1'-0"



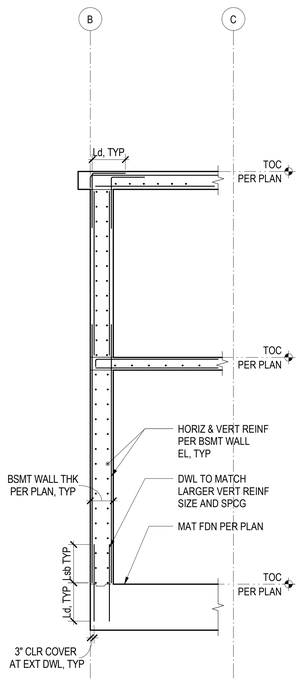
4 BASEMENT WALL SECTION
1/4" = 1'-0"



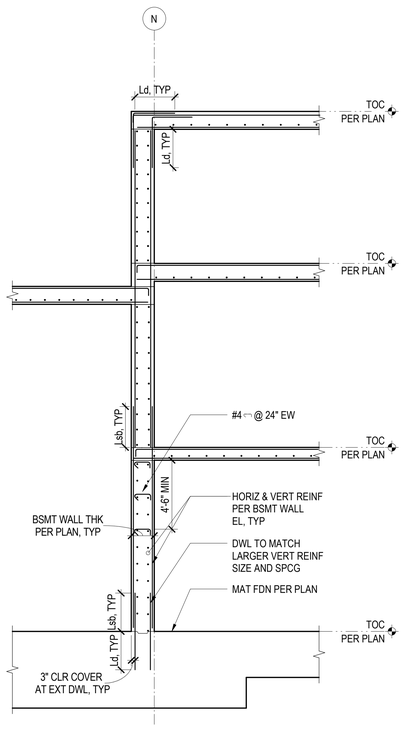
5 BASEMENT WALL SECTION
1/4" = 1'-0"



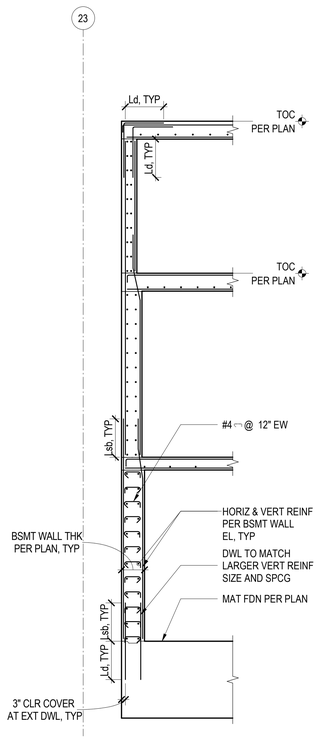
6 BASEMENT WALL SECTION
1/4" = 1'-0"



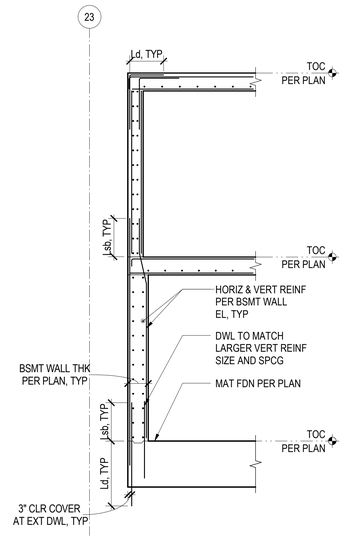
7 BASEMENT WALL SECTION
1/4" = 1'-0"



8 BASEMENT WALL SECTION
1/4" = 1'-0"



9 BASEMENT WALL SECTION
1/4" = 1'-0"



10 BASEMENT WALL SECTION
1/4" = 1'-0"

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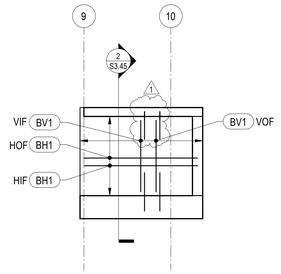
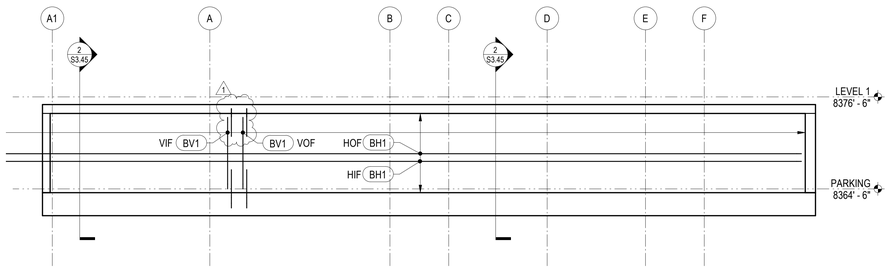
TOWER A & B
BASEMENT WALL
SECTIONS

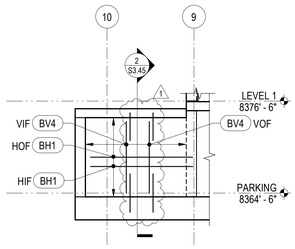
S3.35



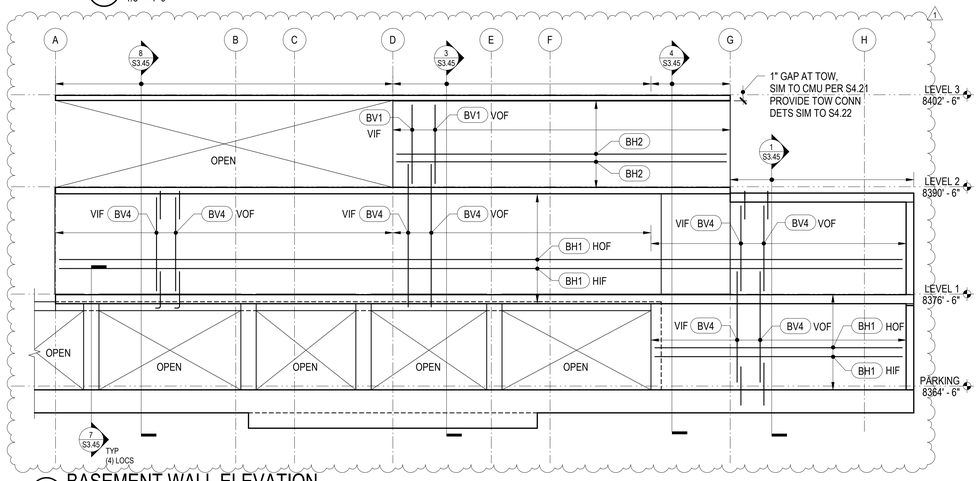
1 BASEMENT WALL ELEVATION
1/8" = 1'-0"



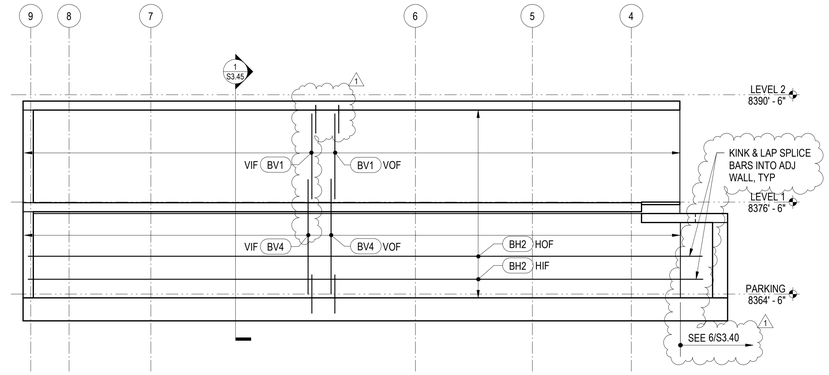
2 BASEMENT WALL ELEVATION
1/8" = 1'-0"



3 BASEMENT WALL ELEVATION
1/8" = 1'-0"



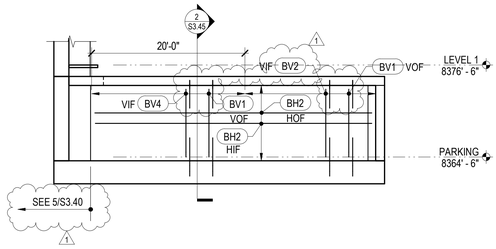
4 BASEMENT WALL ELEVATION
1/8" = 1'-0"



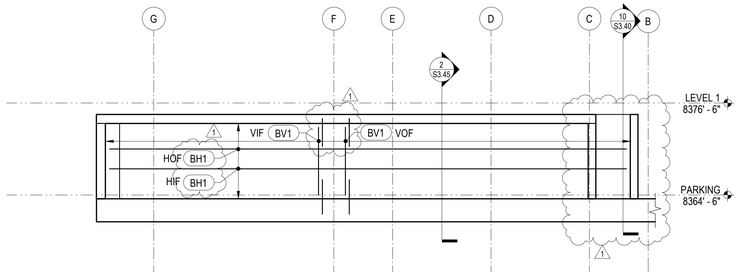
5 BASEMENT WALL ELEVATION
1/8" = 1'-0"

BASEMENT WALL HORIZONTAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	

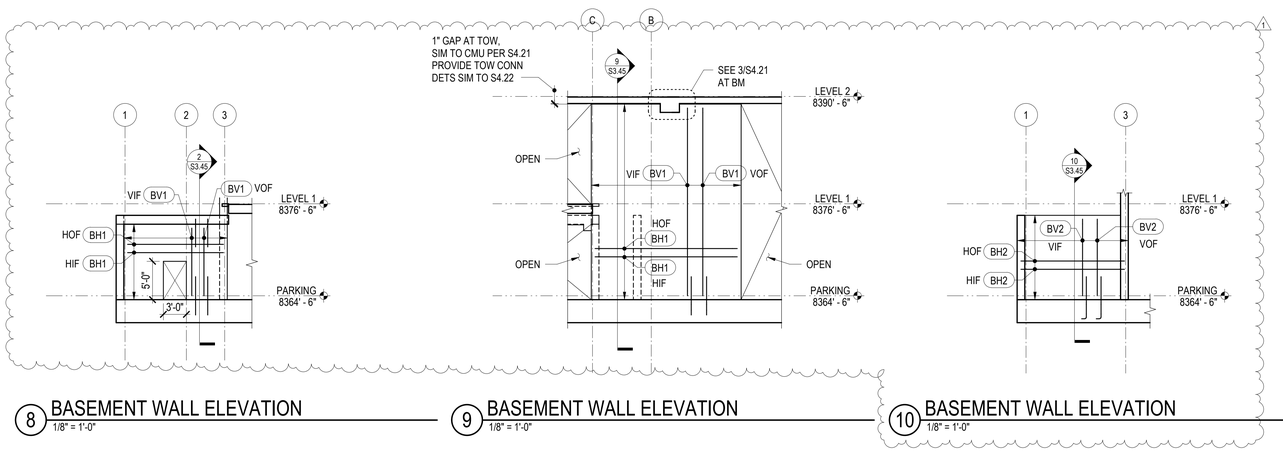
BASEMENT WALL VERTICAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV4	#7 @ 6"	



6 BASEMENT WALL ELEVATION
1/8" = 1'-0"



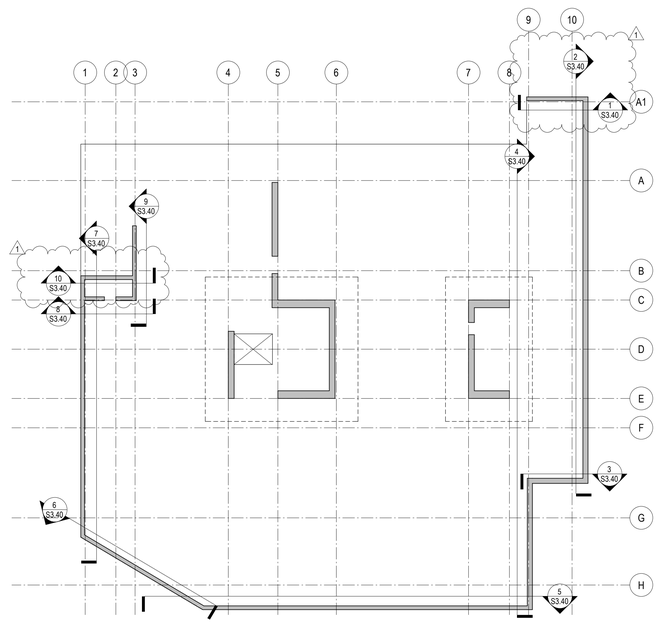
7 BASEMENT WALL ELEVATION
1/8" = 1'-0"



8 BASEMENT WALL ELEVATION
1/8" = 1'-0"

9 BASEMENT WALL ELEVATION
1/8" = 1'-0"

10 BASEMENT WALL ELEVATION
1/8" = 1'-0"



TOWER C - BASEMENT WALL ELEVATION KEY PLAN

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principal architect
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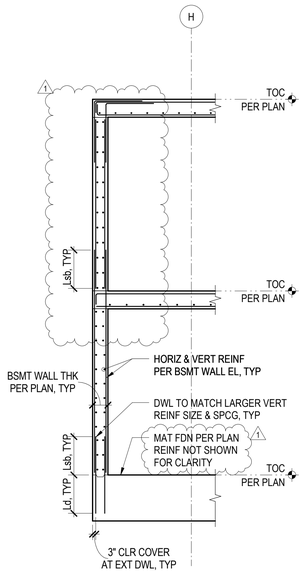
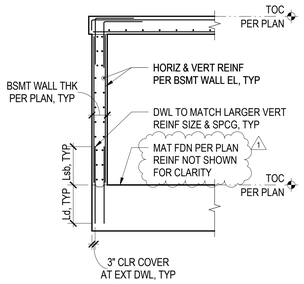
TOWER C
BASEMENT WALL
ELEVATIONS

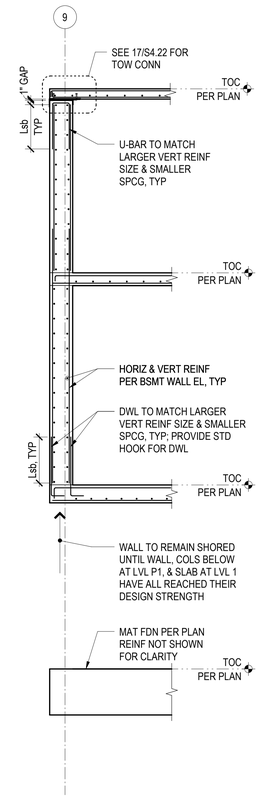
S3.40



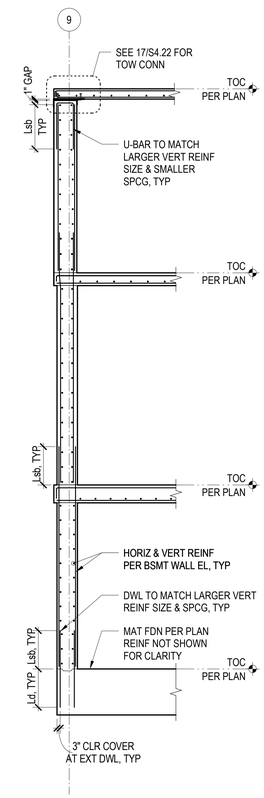
1 BASEMENT WALL SECTION
1/4" = 1'-0"



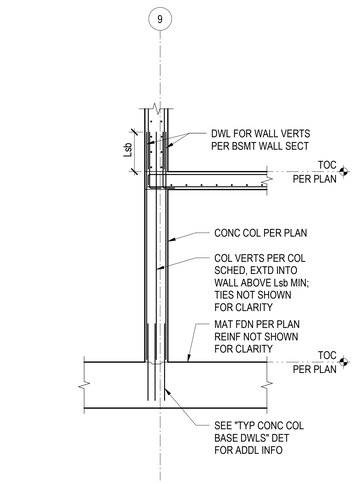
2 BASEMENT WALL SECTION
1/4" = 1'-0"



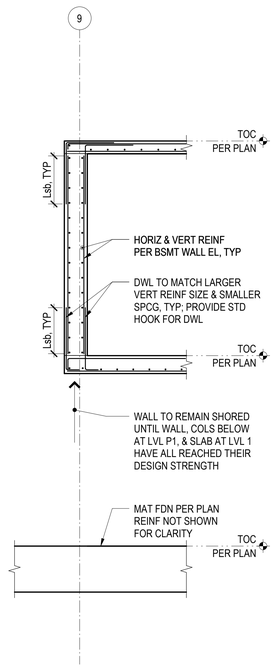
3 BASEMENT WALL SECTION
1/4" = 1'-0"



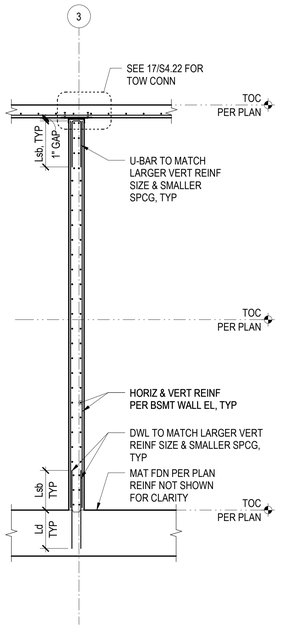
4 BASEMENT WALL SECTION
1/4" = 1'-0"



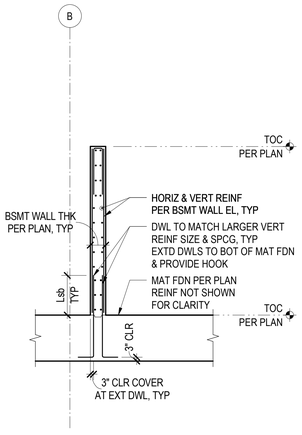
7 SECTION AT COLUMNS SUPPORTING WALL
1/4" = 1'-0"



8 BASEMENT WALL SECTION
1/4" = 1'-0"



9 BASEMENT WALL SECTION
1/4" = 1'-0"



10 BASEMENT WALL SECTION
1/4" = 1'-0"

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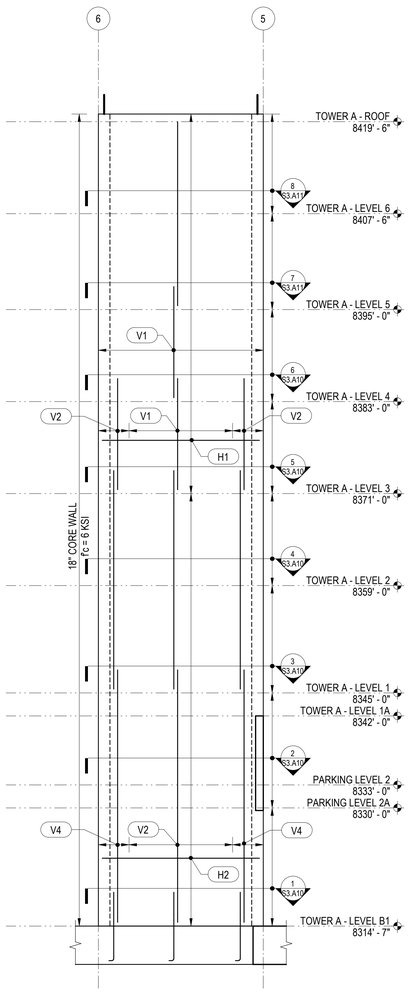
TOWER C
BASEMENT WALL
SECTIONS

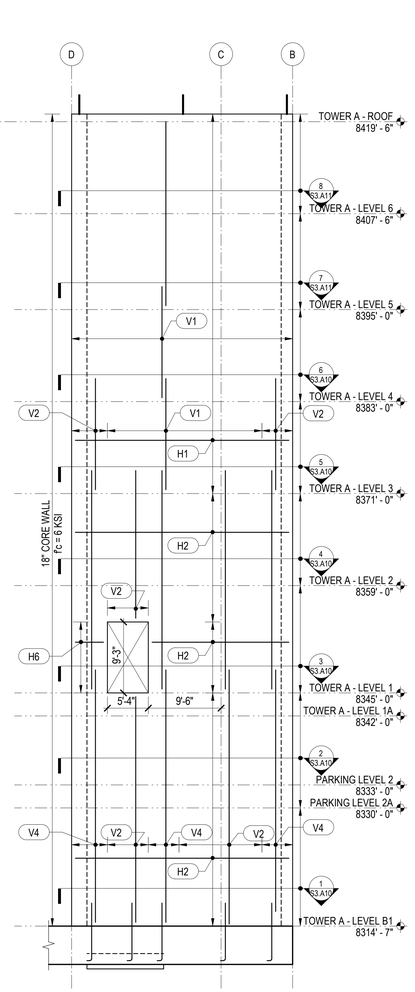
S3.45

SHEAR WALL VERTICAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
V1	#6 @ 12"	EF
V2	#7 @ 12"	EF
V3	#8 @ 12"	EF
V4	#7 @ 6"	EF

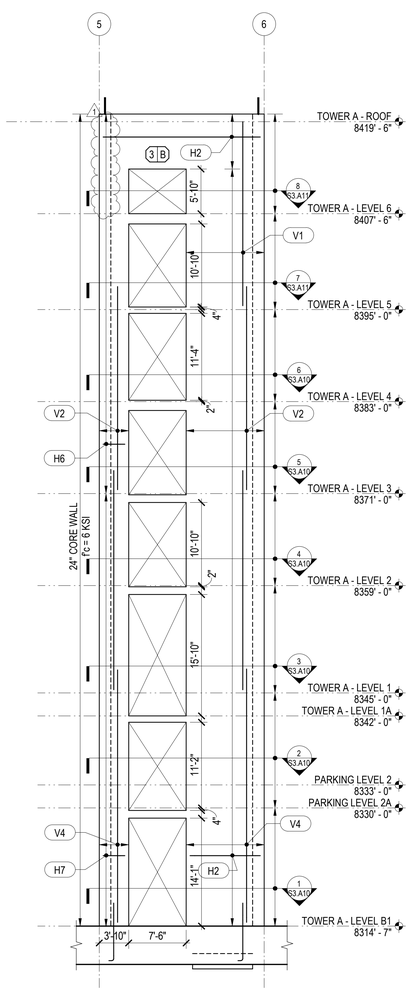
SHEAR WALL HORIZONTAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
H1	#5 @ 12"	EF
H2	#6 @ 12"	EF
H3	#7 @ 12"	EF
H6	#5 @ 6"	(3) LAYERS
H7	#5 @ 6"	(4) LAYERS



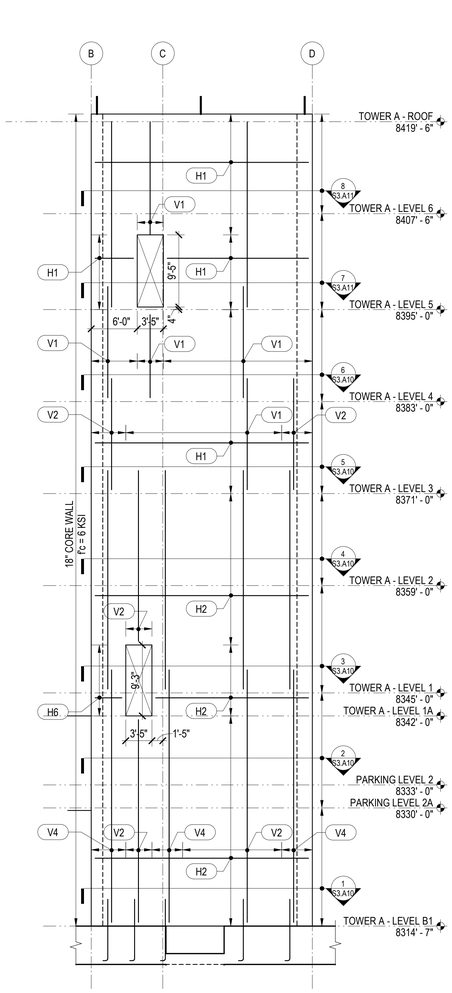
1 NORTH SHEAR WALL ELEVATION
1/8" = 1'-0"



2 EAST SHEAR WALL ELEVATION
1/8" = 1'-0"



3 SOUTH SHEAR WALL ELEVATION
1/8" = 1'-0"



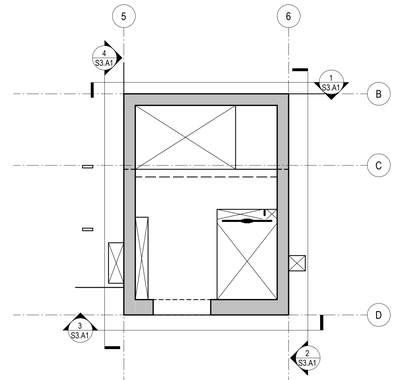
4 WEST SHEAR WALL ELEVATION
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. CORE WALL OPENINGS REMAIN THE SAME SIZE AS THE DIMENSIONED OPENING BELOW, UNLESS NOTED OTHERWISE.
2. WALL OPENINGS AND SLEEVE PENETRATIONS NOT SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.
3. FOUNDATION DOWELS SHALL MATCH SIZE AND SPACING OF SHEAR WALL VERTICAL BARS ABOVE. EXTEND DOWELS TO BOTTOM OF FOUNDATION WITH 90 DEGREE HOOK.
4. VERTICAL REINFORCEMENT RETAINS THE SAME MARK AS FOR THE STORY BELOW, UNLESS NOTED OTHERWISE.
5. LAP VERTICAL REINFORCEMENT Lsb, UNLESS NOTED OTHERWISE. AT THE CONTRACTOR'S OPTION VERTICAL REINFORCEMENT MAY BE SPLICED AT INTERMEDIATE LEVELS, UNLESS NOTED OTHERWISE.
6. LAP HORIZONTAL REINFORCEMENT Lsbt, UNLESS NOTED OTHERWISE. WHERE LAPS NOT EXPLICITLY SHOWN IN SHEAR WALL SECTIONS, CONTRACTOR SHALL SUBMIT ANY ADDITIONAL LAP LOCATIONS FOR REVIEW BY THE ENGINEER.
7. SEE SHEAR WALL SECTIONS FOR ADDITIONAL REINFORCEMENT NOT SHOWN ON SHEAR WALL ELEVATIONS.
8. (1 A) INDICATES A CONCRETE COUPLING BEAM. SEE "TYPICAL CONCRETE COUPLING BEAM DETAILS AND SCHEDULES" FOR REINFORCEMENT.
9. CONCRETE PLACED IN THE COUPLING BEAMS, INCLUDING THROUGHOUT THE SLAB DEPTH, SHALL MEET THE SAME SPECIFICATIONS AS FOR THE SHEAR WALLS. SEE GENERAL NOTES FOR CRITERIA.
10. SEE GENERAL NOTES FOR REINFORCEMENT THAT SHALL MEET SPECIAL DUCTILE QUALITY CRITERIA.



TOWER A WEST CORE WALL ELEVATION - KEY PLAN

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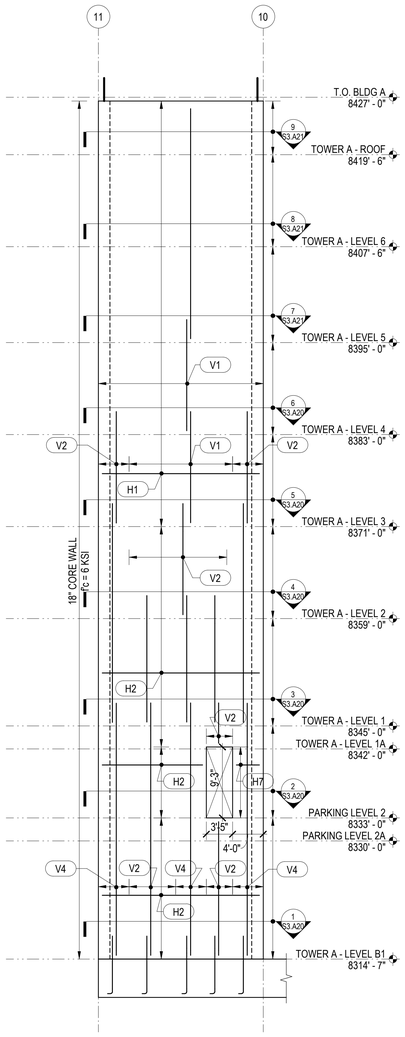
TOWER A WEST CORE WALL ELEVATIONS

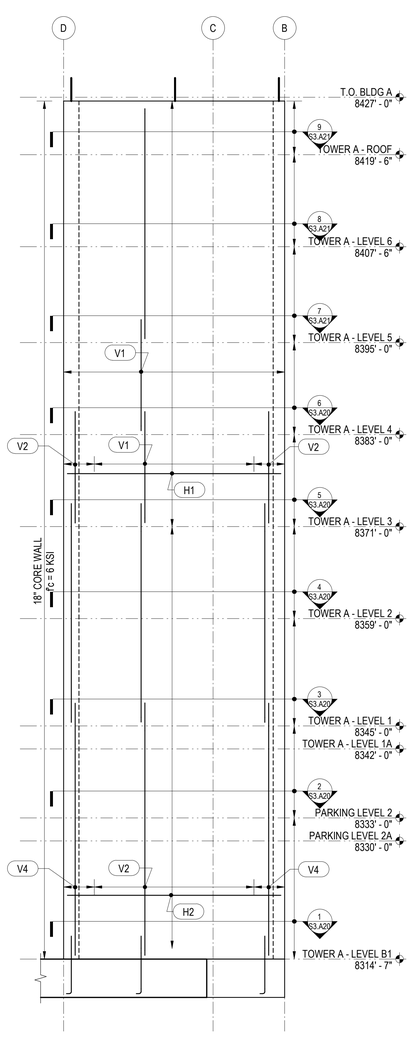
S3.A1

SHEAR WALL VERTICAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
V1	#6 @ 12"	EF
V2	#7 @ 12"	EF
V3	#8 @ 12"	EF
V4	#7 @ 6"	EF

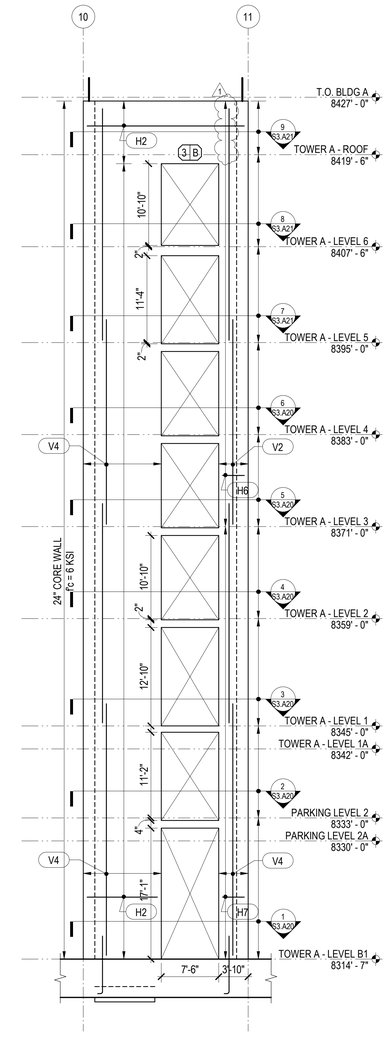
SHEAR WALL HORIZONTAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
H1	#5 @ 12"	EF
H2	#6 @ 12"	EF
H3	#7 @ 12"	EF
H6	#5 @ 6"	(3) LAYERS
H7	#5 @ 6"	(4) LAYERS



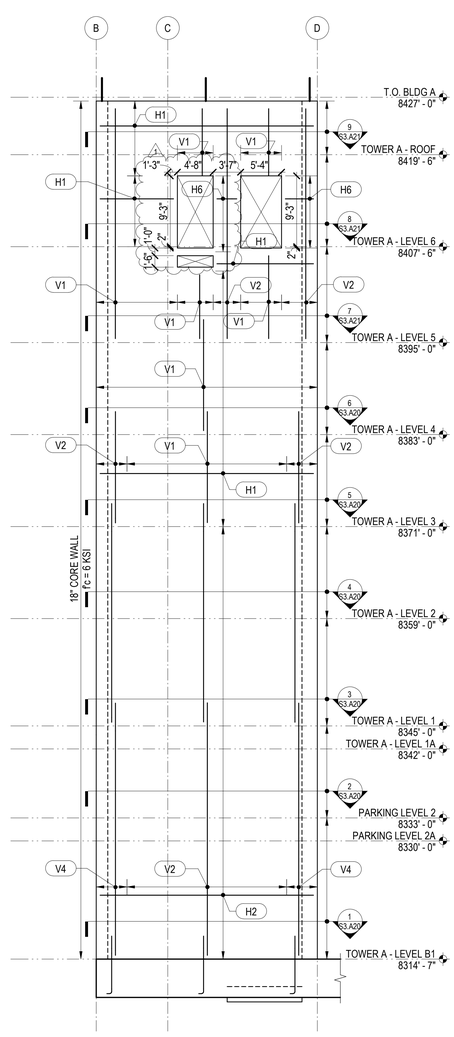
1 NORTH SHEAR WALL ELEVATION
1/8" = 1'-0"



2 EAST SHEAR WALL ELEVATION
1/8" = 1'-0"



3 SOUTH SHEAR WALL ELEVATION
1/8" = 1'-0"



4 WEST SHEAR WALL ELEVATION
1/8" = 1'-0"

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S3.XX ELEVATIONS
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S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

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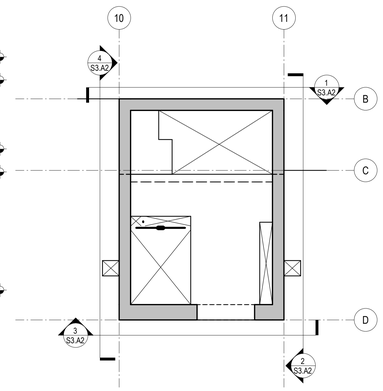
3. FOUNDATION DOWELS SHALL MATCH SIZE AND SPACING OF SHEAR WALL VERTICAL BARS ABOVE. EXTEND DOWELS TO BOTTOM OF FOUNDATION WITH 90 DEGREE HOOK.
4. VERTICAL REINFORCEMENT RETAINS THE SAME MARK AS FOR THE STORY BELOW, UNLESS NOTED OTHERWISE.
5. LAP VERTICAL REINFORCEMENT Lsb, UNLESS NOTED OTHERWISE. AT THE CONTRACTOR'S OPTION VERTICAL REINFORCEMENT MAY BE SPLICED AT INTERMEDIATE LEVELS, UNLESS NOTED OTHERWISE.
6. LAP HORIZONTAL REINFORCEMENT Lsbt, UNLESS NOTED OTHERWISE. WHERE LAPS NOT EXPLICITLY SHOWN IN SHEAR WALL SECTIONS, CONTRACTOR SHALL SUBMIT ANY ADDITIONAL LAP LOCATIONS FOR REVIEW BY THE ENGINEER.
7. SEE SHEAR WALL SECTIONS FOR ADDITIONAL REINFORCEMENT NOT SHOWN ON SHEAR WALL ELEVATIONS.
8. (1 A) INDICATES A CONCRETE COUPLING BEAM. SEE "TYPICAL CONCRETE COUPLING BEAM DETAILS AND SCHEDULES" FOR REINFORCEMENT.
9. CONCRETE PLACED IN THE COUPLING BEAMS, INCLUDING THROUGHOUT THE SLAB DEPTH, SHALL MEET THE SAME SPECIFICATIONS AS FOR THE SHEAR WALLS. SEE GENERAL NOTES FOR CRITERIA.
10. SEE GENERAL NOTES FOR REINFORCEMENT THAT SHALL MEET SPECIAL DUCTILE QUALITY CRITERIA.



TOWER A EAST CORE WALL ELEVATION - KEY PLAN

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principal architect
project manager
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checked by Checker
job no. 20052
date 7/15/2022

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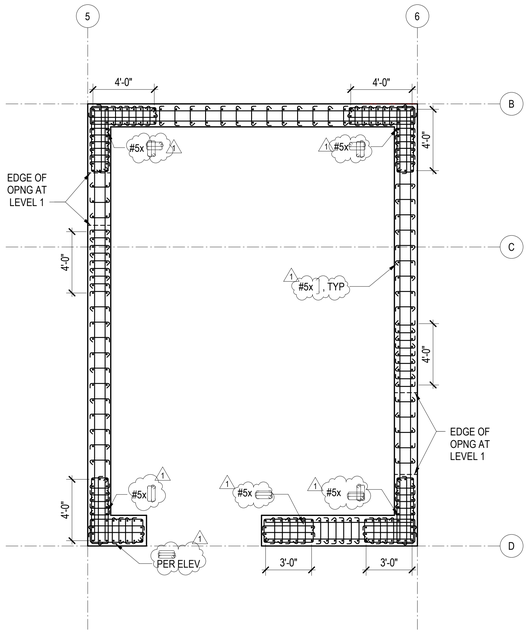
TOWER A EAST CORE WALL ELEVATIONS

S3.A2

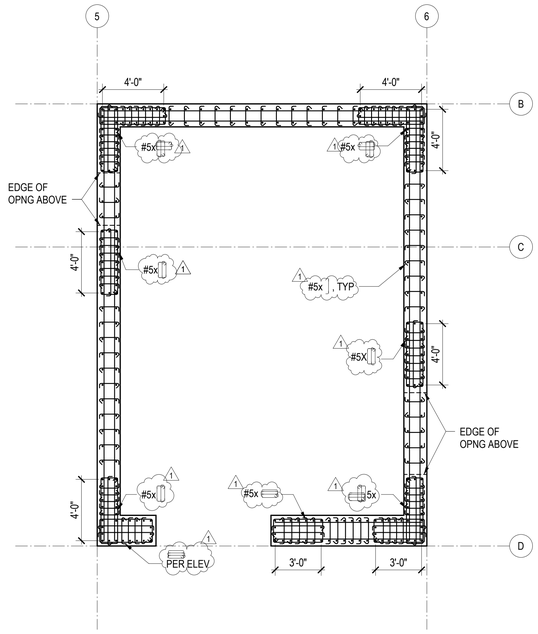
NOTES:

1. ALL HOOPS AND CROSSTIES SHALL BE #5 A615 GRADE 80 PLACED AT 6-INCH VERTICAL SPACING, UNLESS NOTED OTHERWISE. CROSSTIES WITH 90 DEGREE HOOKS SHALL HAVE THE CONSECUTIVE CROSSTIES ALTERNATED END-FOR-END ALONG THE LONGITUDINAL REINFORCEMENT.
2. CONCRETE COVER TO REINFORCEMENT IS 1-INCH.
3. BAR SIZE OF ADDED VERTICAL BARS TO MATCH SIZE AND GRADE OF ADJACENT REINFORCEMENT, UNLESS NOTED OTHERWISE. AT CORNERS, MATCH REINFORCEMENT SIZE OF THE LARGER OF THE TWO WALLS.

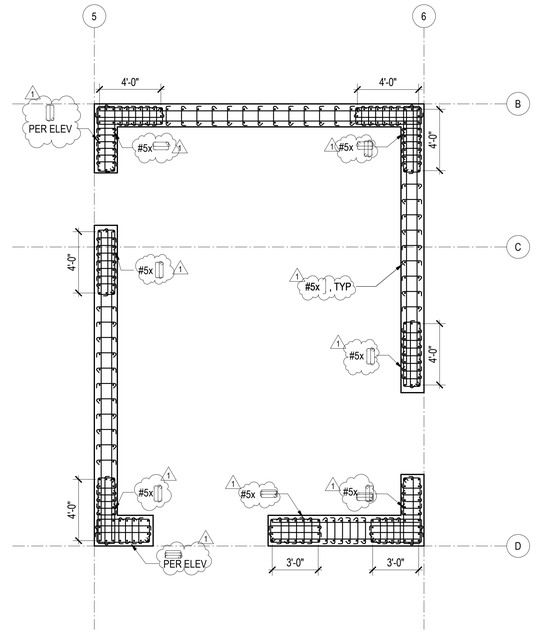
SHEAR WALL NOTES



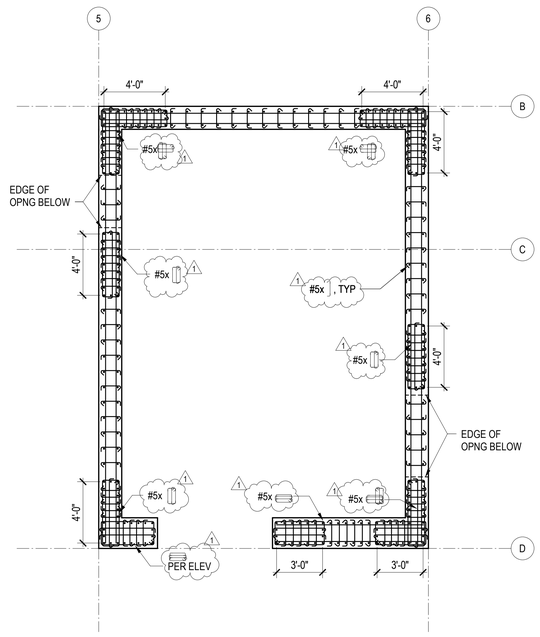
1 SHEAR WALL SECTION - FOUNDATION TO PARKING LEVEL 1
1/4" = 1'-0"



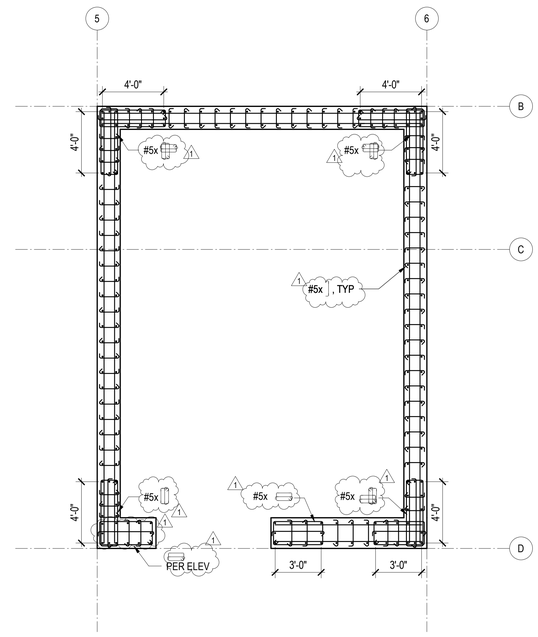
2 SHEAR WALL SECTION - PARKING LEVEL 1 TO LEVEL 1
1/4" = 1'-0"



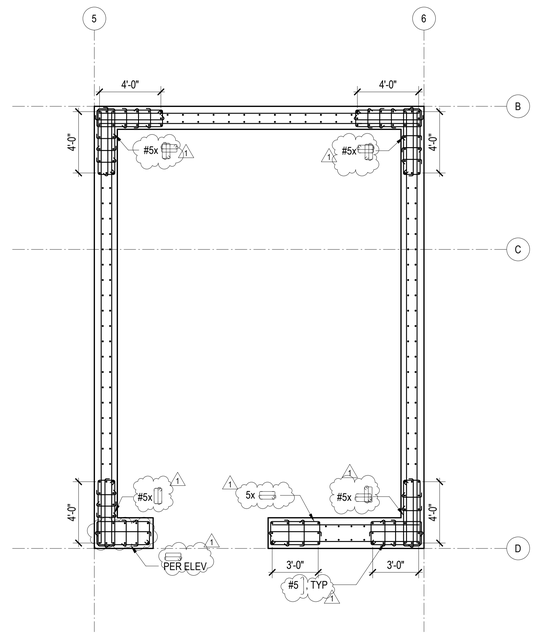
3 SHEAR WALL SECTION - LEVEL 1 TO LEVEL 2
1/4" = 1'-0"



4 SHEAR WALL SECTION - LEVEL 2 TO LEVEL 3
1/4" = 1'-0"



5 SHEAR WALL SECTION - LEVEL 3 TO LEVEL 4
1/4" = 1'-0"



6 SHEAR WALL SECTION - LEVEL 4 TO LEVEL 5
1/4" = 1'-0"

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principal architect
project manager
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TOWER A WEST
CORE WALL
SECTIONS

S3.A10

NOTES:

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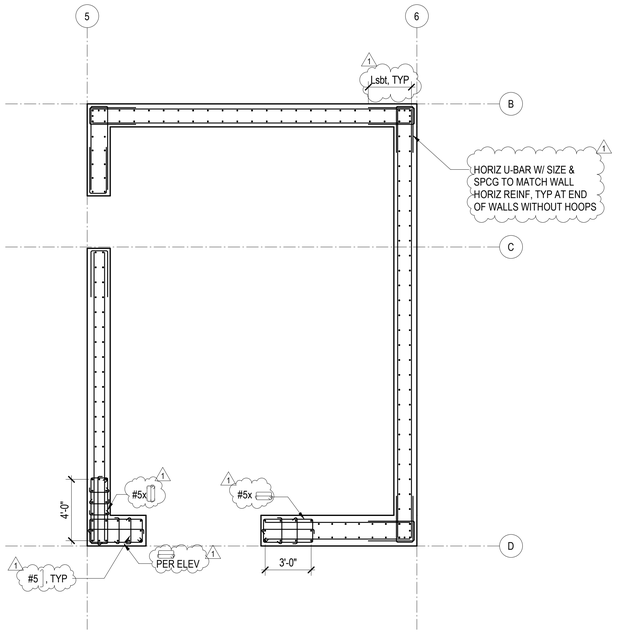
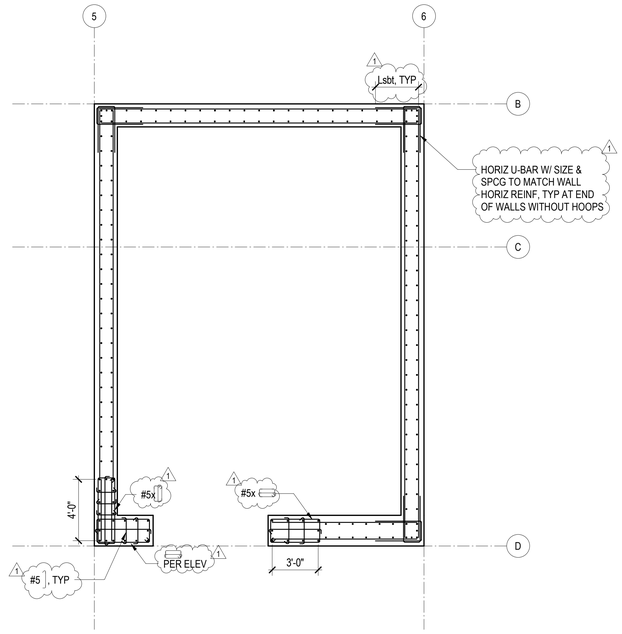
SHEAR WALL NOTES



7 SHEAR WALL SECTION - LEVEL 5 TO LEVEL 6
1/4" = 1'-0"



8 SHEAR WALL SECTION - LEVEL 6 TO TOP OF CORE
1/4" = 1'-0"

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TOWER A WEST
CORE WALL
SECTIONS

S3.A11

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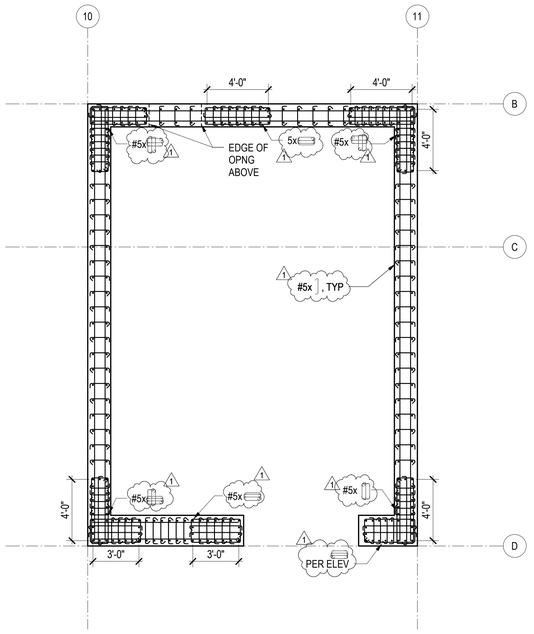
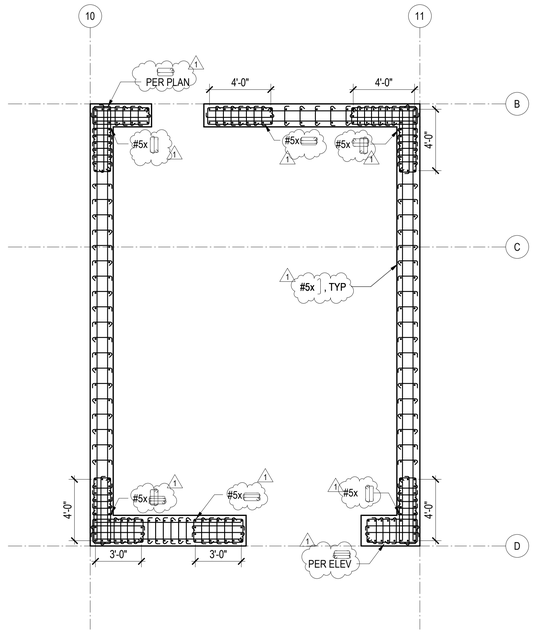
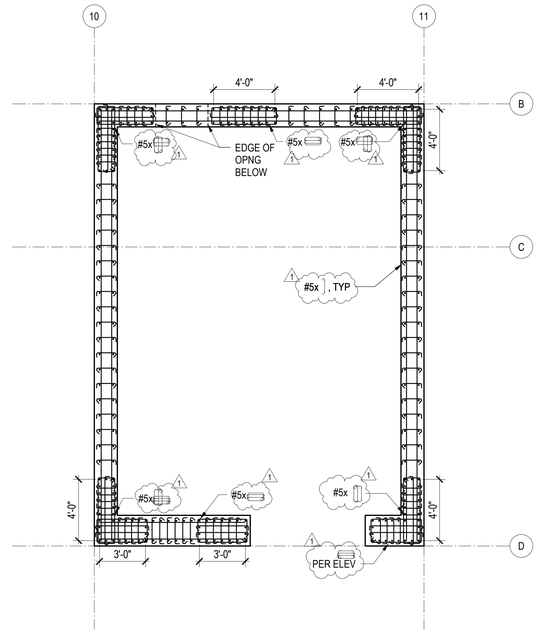
SHEAR WALL NOTES



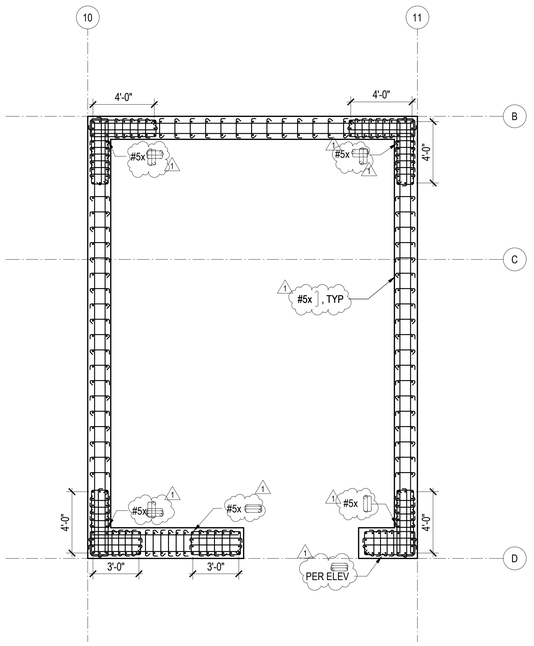
1 SHEAR WALL SECTION - FOUNDATION TO PARKING LEVEL 2
1/4" = 1'-0"



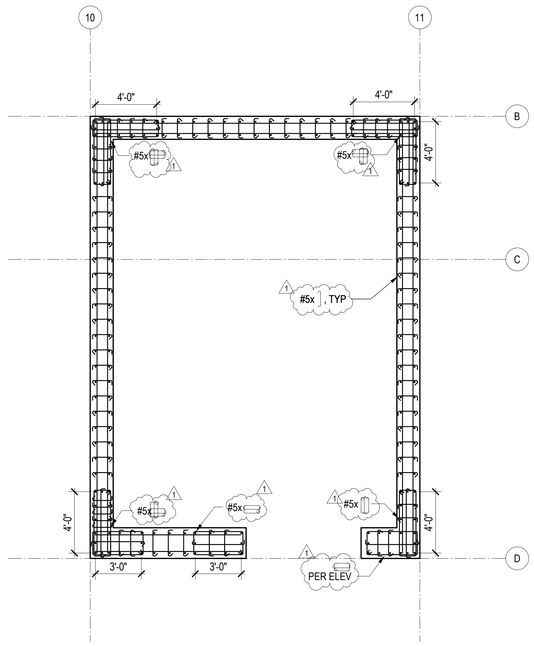
2 SHEAR WALL SECTION - PARKING LEVEL 2 TO LEVEL 1
1/4" = 1'-0"



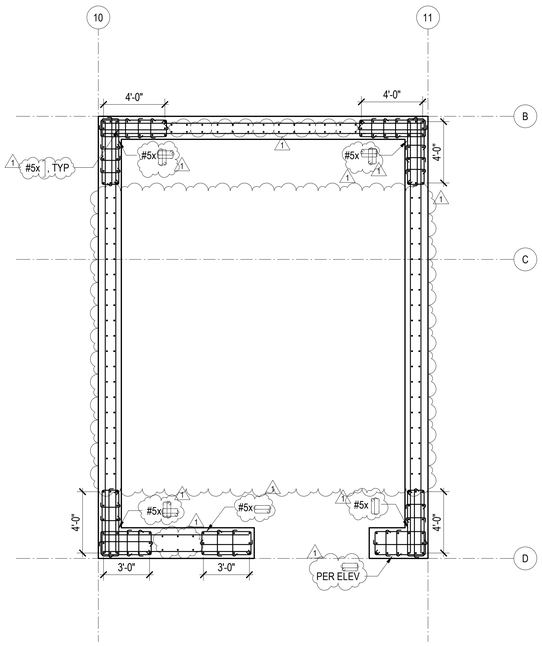
3 SHEAR WALL SECTION - LEVEL 1 TO LEVEL 2
1/4" = 1'-0"



4 SHEAR WALL SECTION - LEVEL 2 TO LEVEL 3
1/4" = 1'-0"



5 SHEAR WALL SECTION - LEVEL 3 TO LEVEL 6
1/4" = 1'-0"



6 SHEAR WALL SECTION - LEVEL 4 TO LEVEL 5
1/4" = 1'-0"

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TOWER A EAST
CORE WALL
SECTIONS

S3.A20

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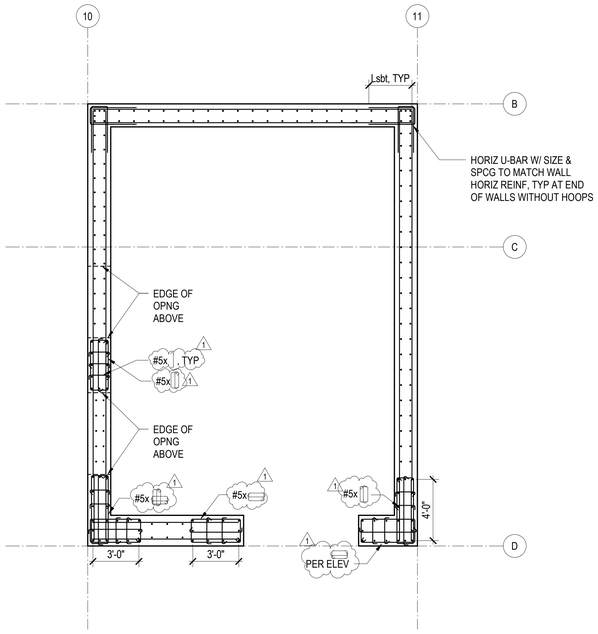
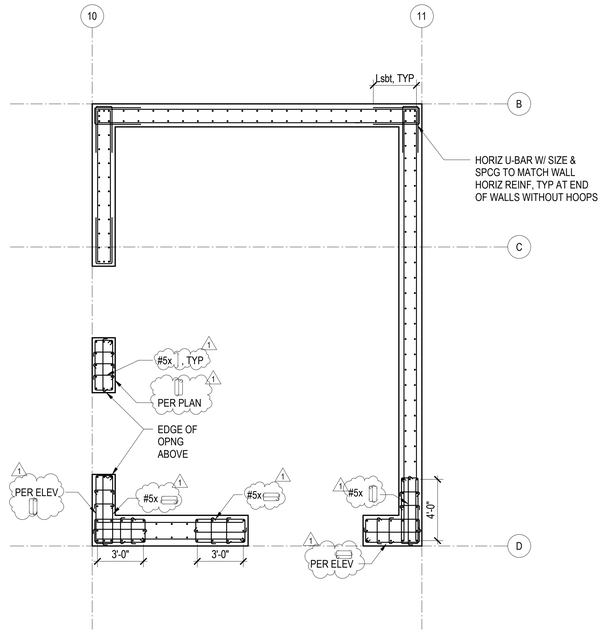
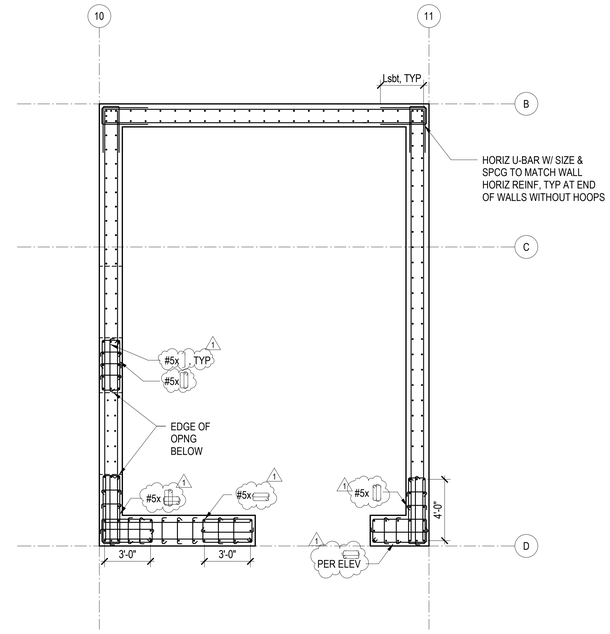
SHEAR WALL NOTES



7 SHEAR WALL SECTION - LEVEL 5 TO LEVEL 6
1/4" = 1'-0"



8 SHEAR WALL ELEVATION - LEVEL 6 TO ROOF
1/4" = 1'-0"



9 SHEAR WALL ELEVATION - ROOF TO TOP OF CORE
1/4" = 1'-0"

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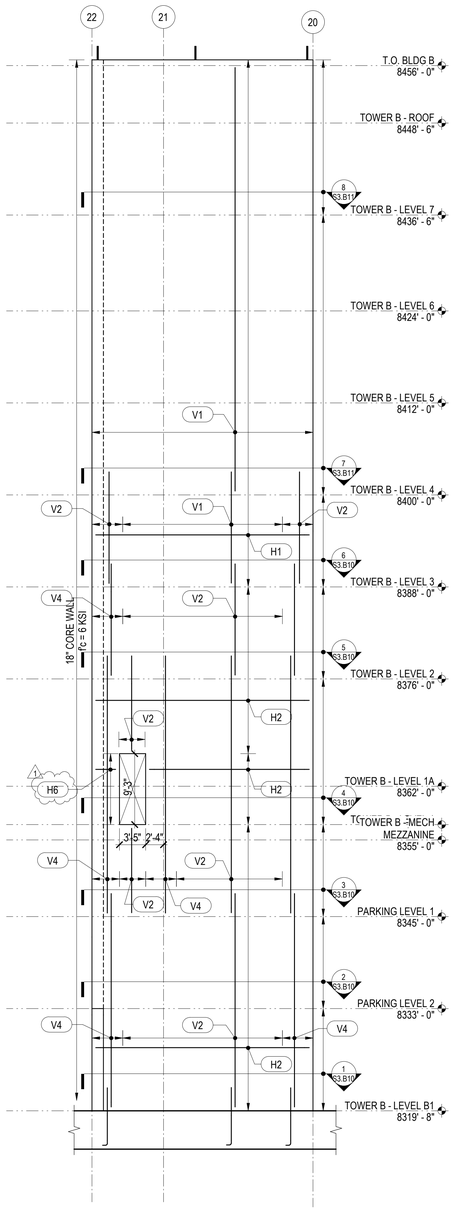
TOWER A EAST
CORE WALL
SECTIONS

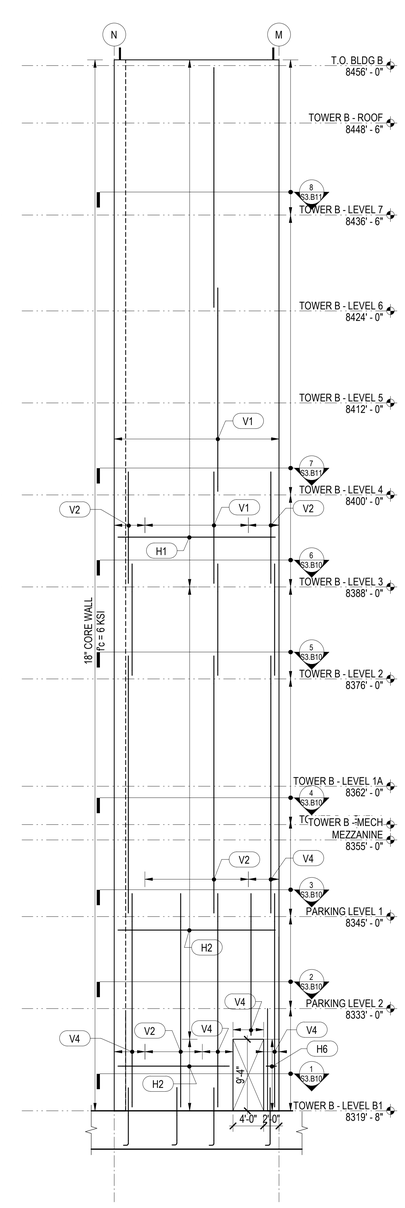
S3.A21

SHEAR WALL VERTICAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
V1	#6 @ 12"	EF
V2	#7 @ 12"	EF
V3	#8 @ 12"	EF
V4	#7 @ 6"	EF

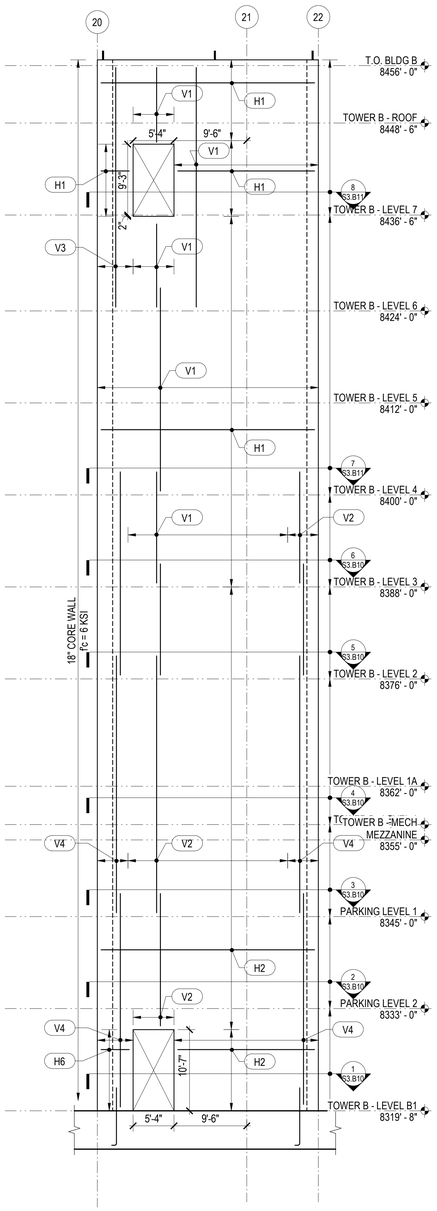
SHEAR WALL HORIZONTAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
H1	#5 @ 12"	EF
H2	#6 @ 12"	EF
H3	#7 @ 12"	EF
H6	#5 @ 6"	(3) LAYERS
H7	#5 @ 6"	(4) LAYERS



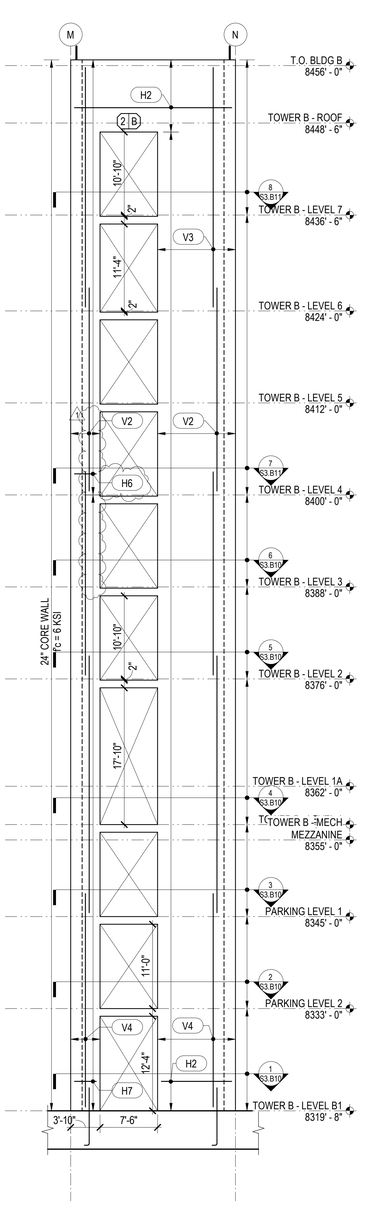
1 NORTH SHEAR WALL ELEVATION
1/8" = 1'-0"



2 EAST SHEAR WALL ELEVATION
1/8" = 1'-0"



3 SOUTH SHEAR WALL ELEVATION
1/8" = 1'-0"



4 WEST SHEAR WALL ELEVATION
1/8" = 1'-0"

REFERENCE DRAWINGS

S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
S1.XX LOAD DIAGRAMS
S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

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1. CORE WALL OPENINGS REMAIN THE SAME SIZE AS THE DIMENSIONED OPENING BELOW, UNLESS NOTED OTHERWISE.
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5. LAP VERTICAL REINFORCEMENT Lsb, UNLESS NOTED OTHERWISE. AT THE CONTRACTOR'S OPTION VERTICAL REINFORCEMENT MAY BE SPLICED AT INTERMEDIATE LEVELS, UNLESS NOTED OTHERWISE.
6. LAP HORIZONTAL REINFORCEMENT Lsbt, UNLESS NOTED OTHERWISE. WHERE LAPS NOT EXPLICITLY SHOWN IN SHEAR WALL SECTIONS, CONTRACTOR SHALL SUBMIT ANY ADDITIONAL LAP LOCATIONS FOR REVIEW BY THE ENGINEER.
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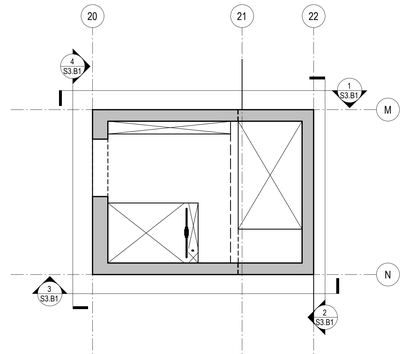

TOWER B NORTH CORE WALL ELEVATION - KEY PLAN

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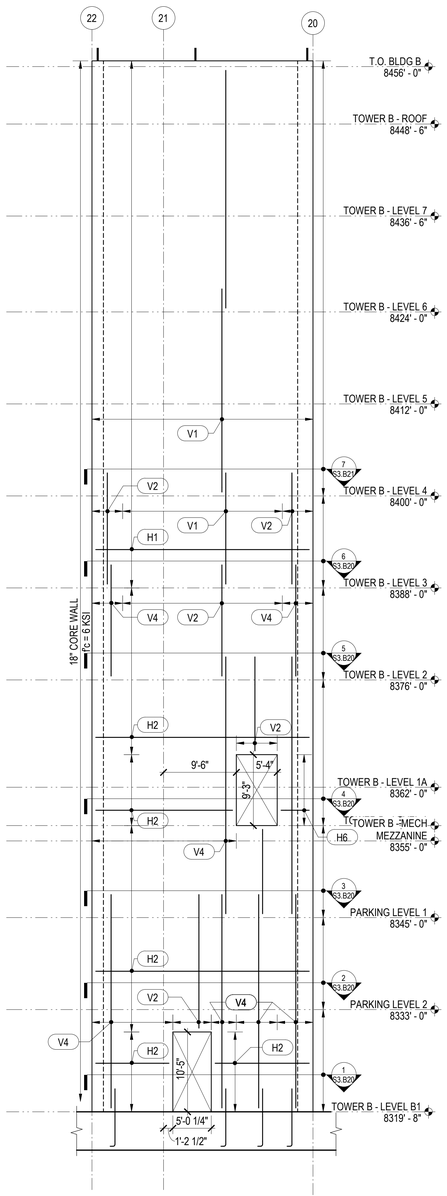
TOWER B NORTH CORE WALL ELEVATIONS

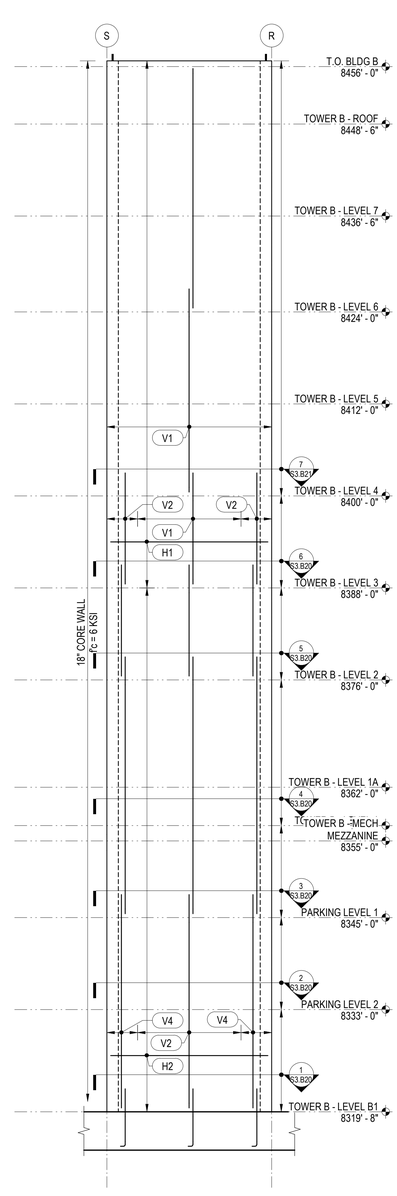
S3.B1

SHEAR WALL VERTICAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
V1	#6 @ 12"	EF
V2	#7 @ 12"	EF
V3	#8 @ 12"	EF
V4	#7 @ 6"	EF

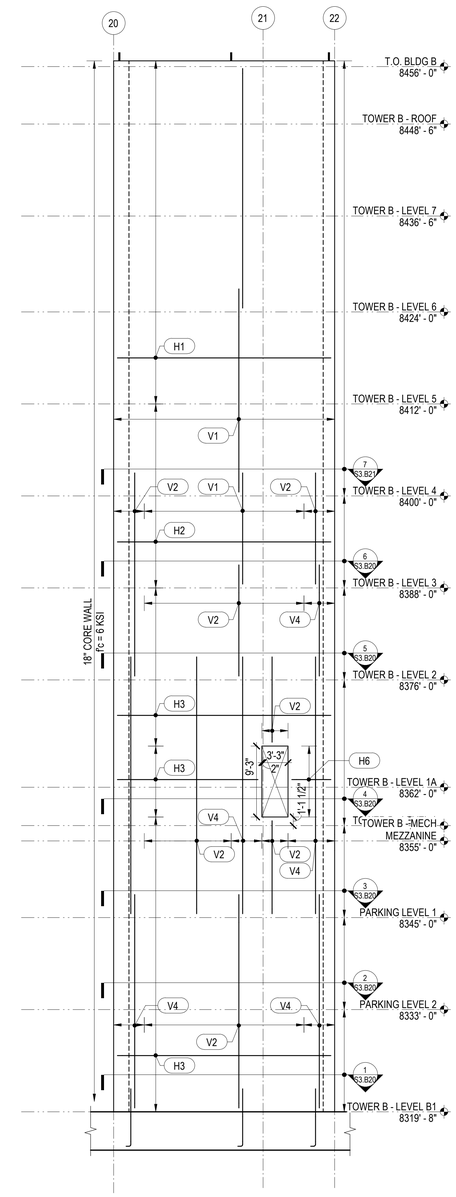
SHEAR WALL HORIZONTAL REINFORCING SCHEDULE		
MARK	REINFORCING	REMARKS
H1	#5 @ 12"	EF
H2	#6 @ 12"	EF
H3	#7 @ 12"	EF
H6	#5 @ 6"	(3) LAYERS
H7	#5 @ 6"	(4) LAYERS



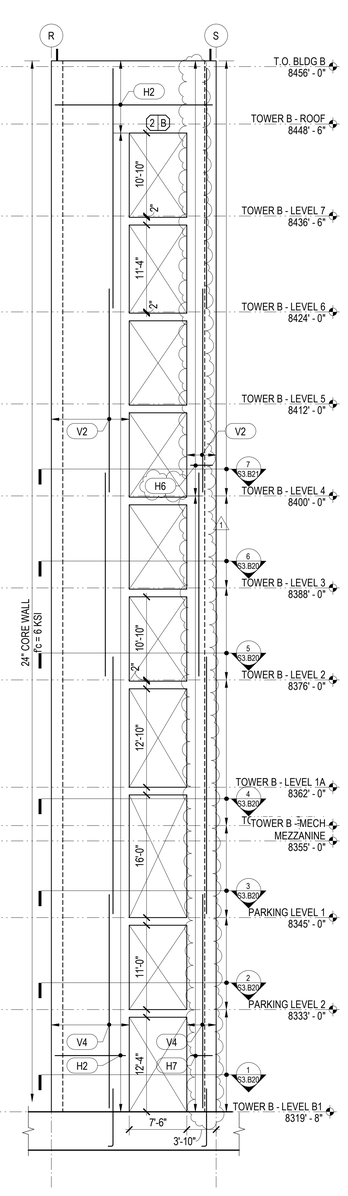
1 NORTH SHEAR WALL ELEVATION
1/8" = 1'-0"



2 EAST SHEAR WALL ELEVATION
1/8" = 1'-0"



3 SOUTH SHEAR WALL ELEVATION
1/8" = 1'-0"



4 WEST SHEAR WALL ELEVATION
1/8" = 1'-0"

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S5.XX CONCRETE SECTIONS AND DETAILS
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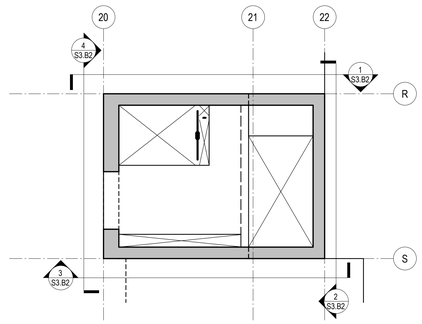
6. LAP HORIZONTAL REINFORCEMENT Lsbt, UNLESS NOTED OTHERWISE. WHERE LAPS NOT EXPLICITLY SHOWN IN SHEAR WALL SECTIONS, CONTRACTOR SHALL SUBMIT ANY ADDITIONAL LAP LOCATIONS FOR REVIEW BY THE ENGINEER.
7. SEE SHEAR WALL SECTIONS FOR ADDITIONAL REINFORCEMENT NOT SHOWN ON SHEAR WALL ELEVATIONS.
8. (1 A) INDICATES A CONCRETE COUPLING BEAM. SEE "TYPICAL CONCRETE COUPLING BEAM DETAILS AND SCHEDULES" FOR REINFORCEMENT.
9. CONCRETE PLACED IN THE COUPLING BEAMS, INCLUDING THROUGHOUT THE SLAB DEPTH, SHALL MEET THE SAME SPECIFICATIONS AS FOR THE SHEAR WALLS. SEE GENERAL NOTES FOR CRITERIA.
10. SEE GENERAL NOTES FOR REINFORCEMENT THAT SHALL MEET SPECIAL DUCTILE QUALITY CRITERIA.



TOWER B SOUTH CORE WALL ELEVATION - KEY PLAN

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project manager
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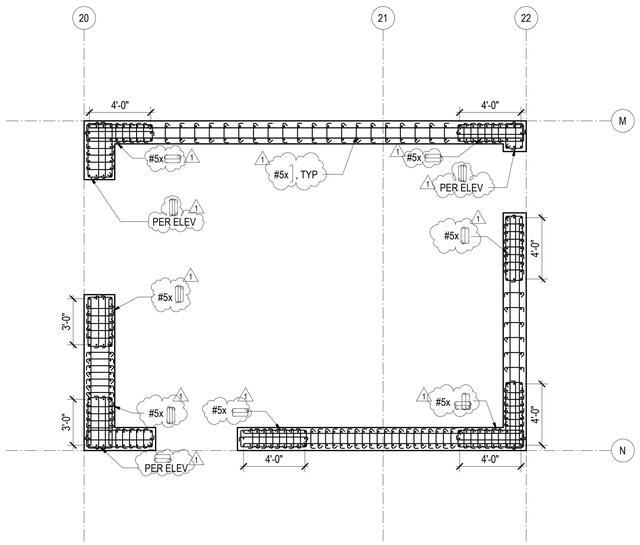
TOWER B SOUTH CORE WALL ELEVATIONS

S3.B2

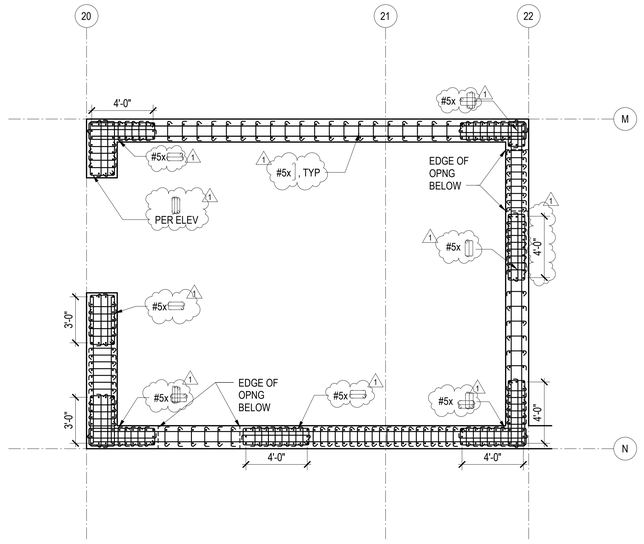
NOTES:

1. ALL HOOPS AND CROSSTIES SHALL BE #5 A615 GRADE 80 PLACED AT 6-INCH VERTICAL SPACING, UNLESS NOTED OTHERWISE. CROSSTIES WITH 90 DEGREE HOOKS SHALL HAVE THE CONSECUTIVE CROSSTIES ALTERNATED END-FOR-END ALONG THE LONGITUDINAL REINFORCEMENT.
2. CONCRETE COVER TO REINFORCEMENT IS 1-INCH.
3. BAR SIZE OF ADDED VERTICAL BARS TO MATCH SIZE AND GRADE OF ADJACENT REINFORCEMENT, UNLESS NOTED OTHERWISE. AT CORNERS, MATCH REINFORCEMENT SIZE OF THE LARGER OF THE TWO WALLS.

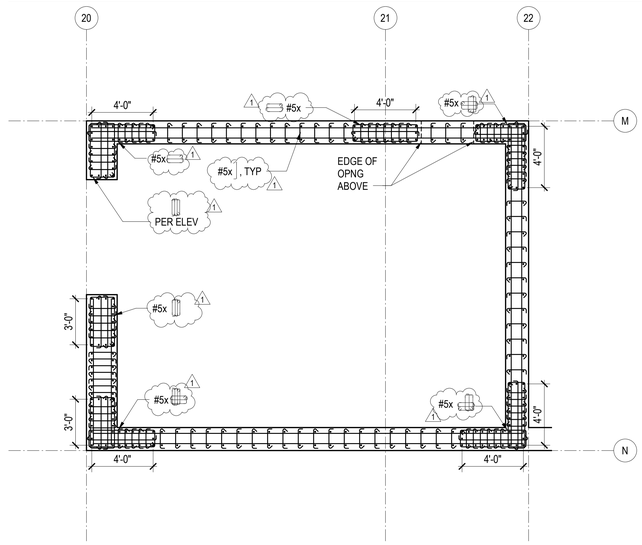
SHEAR WALL NOTES



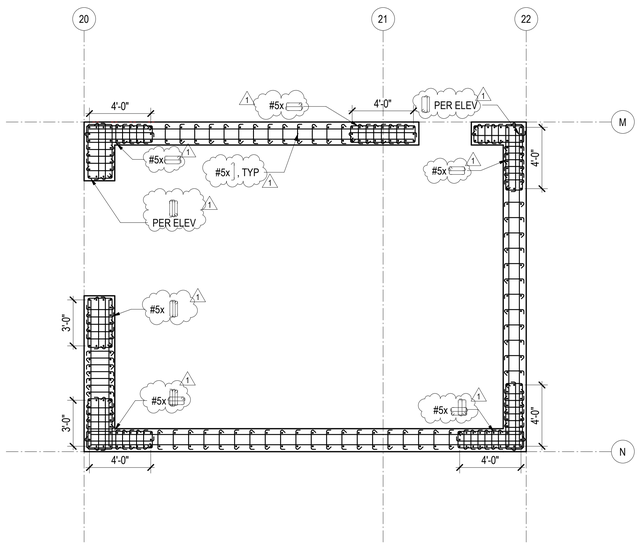
1 SHEAR WALL SECTION - FOUNDATION TO PARKING LEVEL 2
1/4" = 1'-0"



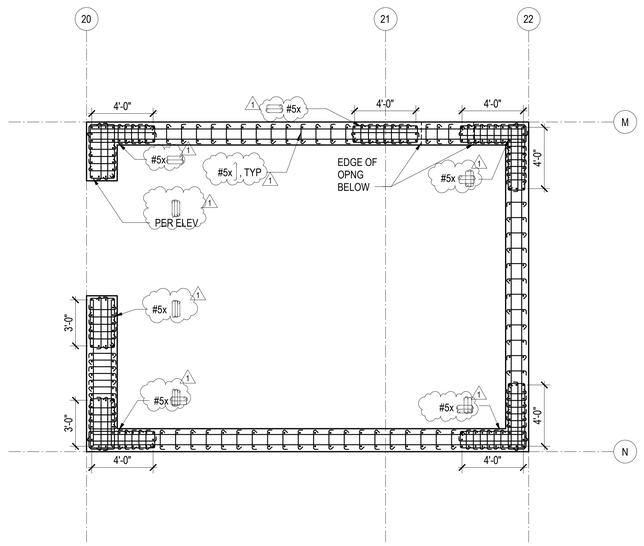
2 SHEAR WALL SECTION - PARKING LEVEL 2 TO PARKING LEVEL 1
1/4" = 1'-0"



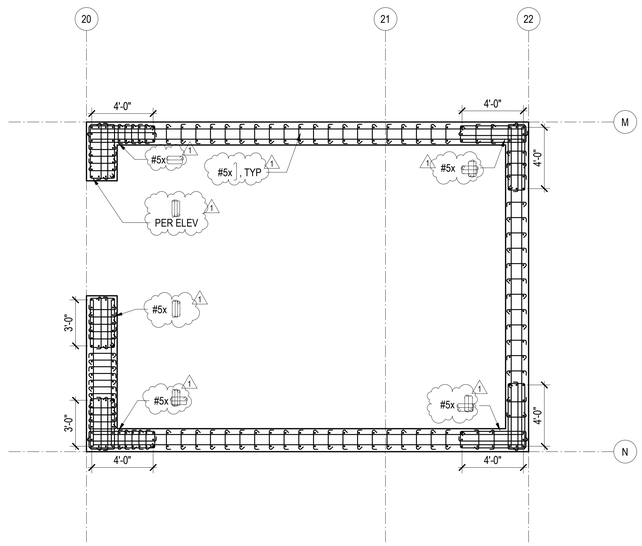
3 SHEAR WALL SECTION - PARKING LEVEL 1 TO LEVEL 1
1/4" = 1'-0"



4 SHEAR WALL SECTION - LEVEL 1 TO LEVEL 2
1/4" = 1'-0"



5 SHEAR WALL SECTION - LEVEL 2 TO LEVEL 3
1/4" = 1'-0"



6 SHEAR WALL SECTION - LEVEL 3 TO LEVEL 4
1/4" = 1'-0"

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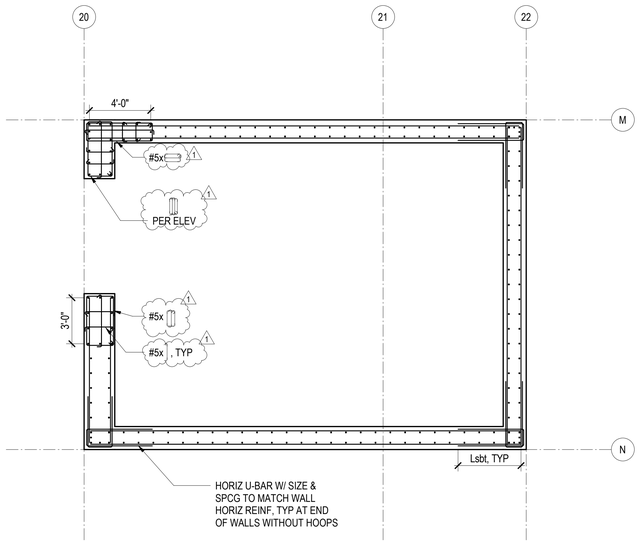
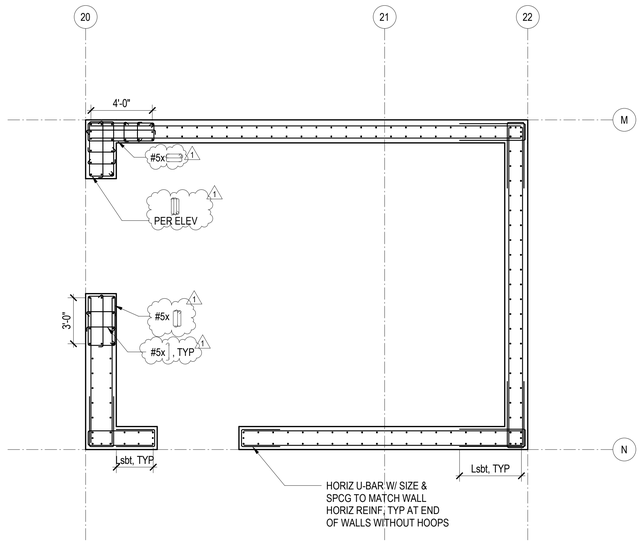
TOWER B NORTH
CORE WALL
SECTIONS

S3.B10



7 SHEAR WALL SECTION - LEVEL 4 TO LEVEL 7
1/4" = 1'-0"



8 SHEAR WALL SECTION - LEVEL 7 TO ROOF
1/4" = 1'-0"

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SHEAR WALL NOTES

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TOWER B NORTH
CORE WALL
SECTIONS

S3.B11

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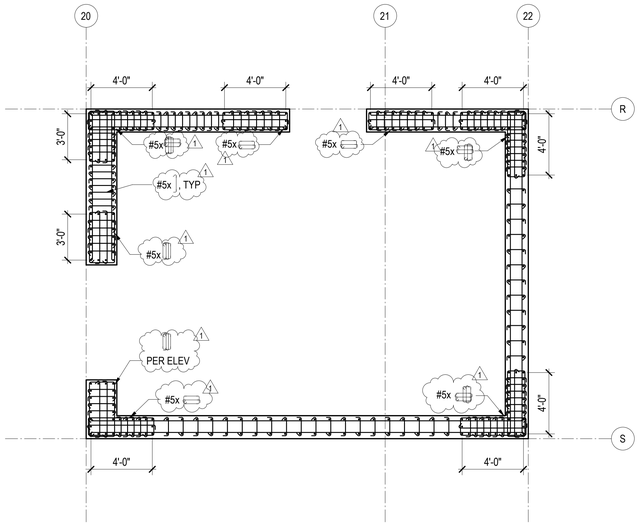
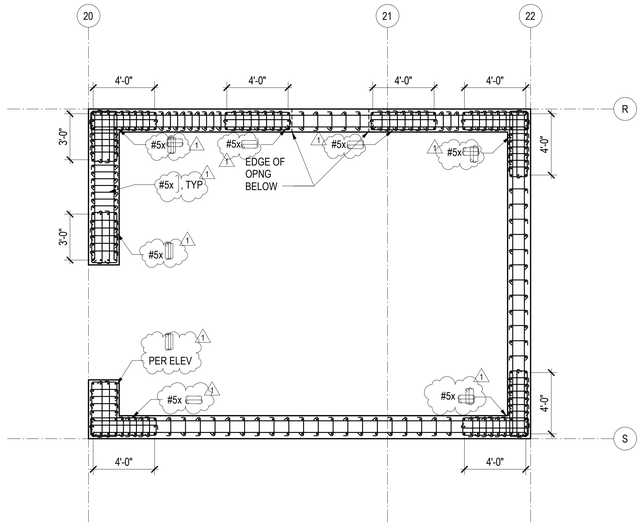
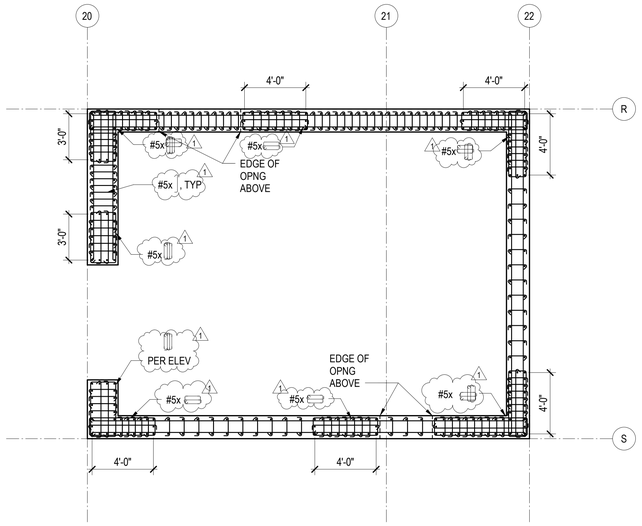
SHEAR WALL NOTES



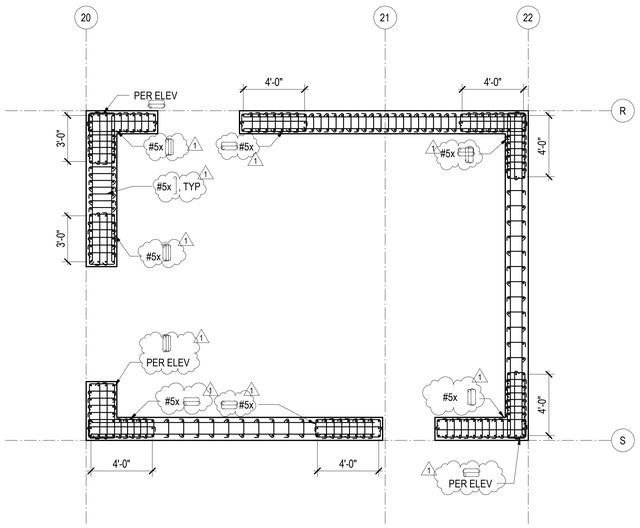
1 SHEAR WALL SECTION - FOUNDATION TO PARKING LEVEL 2
1/4" = 1'-0"



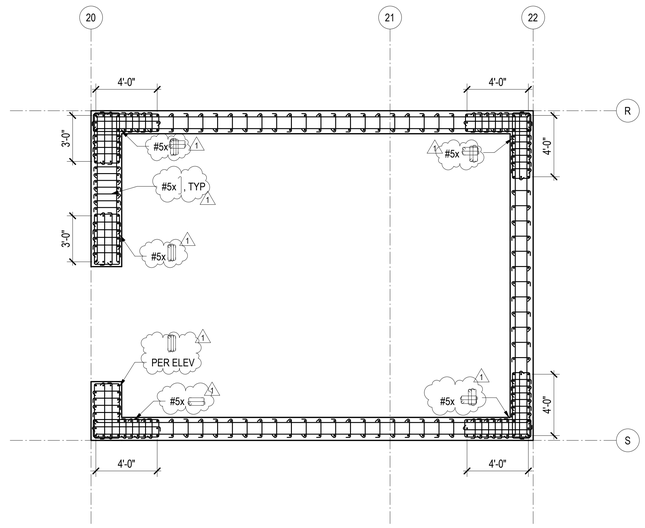
2 SHEAR WALL SECTION - PARKING LEVEL 2 TO PARKING LEVEL 1
1/4" = 1'-0"



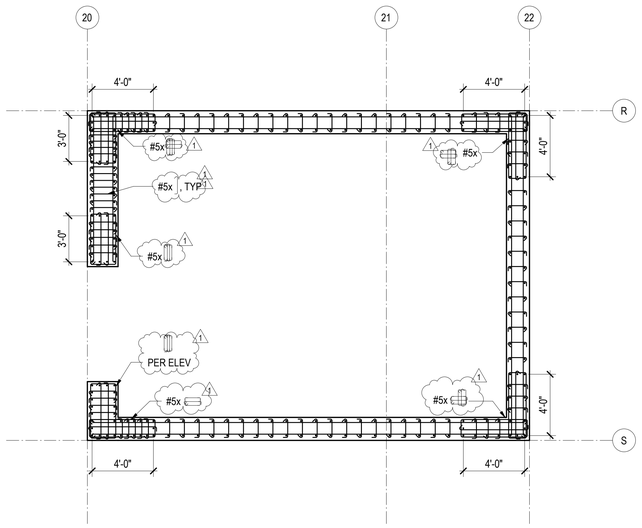
3 SHEAR WALL SECTION - PARKING LEVEL 1 TO LEVEL 1
1/4" = 1'-0"



4 SHEAR WALL SECTION - LEVEL 1 TO LEVEL 2
1/4" = 1'-0"



5 SHEAR WALL SECTION - LEVEL 2 TO LEVEL 3
1/4" = 1'-0"



6 SHEAR WALL SECTION - LEVEL 3 TO LEVEL 4
1/4" = 1'-0"

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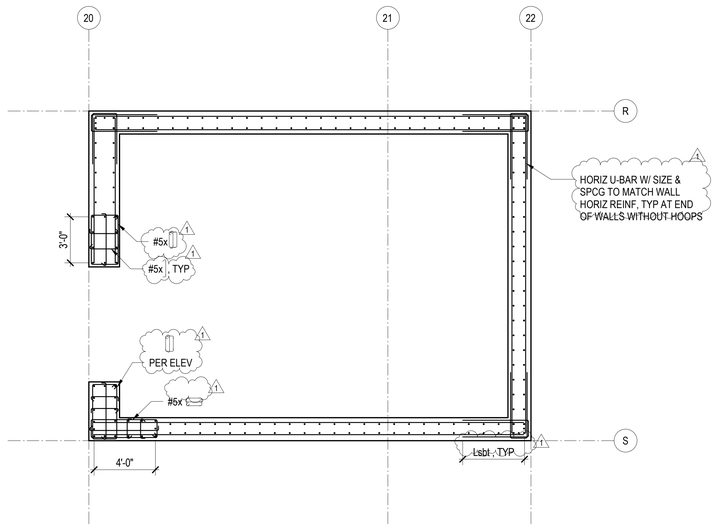
TOWER B SOUTH CORE WALL SECTIONS

S3.B20



7 SHEAR WALL SECTION - LEVEL 4 TO ROOF
1/4" = 1'-0"

NOTES:

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SHEAR WALL NOTES

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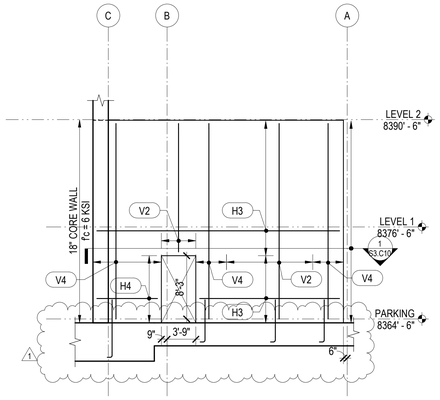
TOWER B SOUTH CORE WALL SECTIONS

S3.B21



5 SHEAR WALL ELEVATION
1/8" = 1'-0"

SHEAR WALL VERTICAL REINFORCEMENT SCHEDULE		
MARK	REINFORCING	REMARKS
V1	#6 @ 12"	EF
V2	#7 @ 12"	EF
V4	#7 @ 6"	EF
V5	#8 @ 6"	EF
V6	#9 @ 6"	EF
V7	#10 @ 6"	EF

SHEAR WALL HORIZONTAL REINFORCEMENT SCHEDULE		
MARK	REINFORCING	REMARKS
H1	#5 @ 12"	EF
H2	#6 @ 12"	EF
H3	#7 @ 12"	EF
H4	#6 @ 6"	EF
H5	#7 @ 6"	EF
H6	#5 @ 6"	(3) LAYERS

REFERENCE DRAWINGS

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S2.XX PLANS
S3.XX ELEVATIONS
S4.XX TYPICAL DETAILS AND SCHEDULES
S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

NOTES:

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2. WALL OPENINGS AND SLEEVE PENETRATIONS NOT SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.
3. FOUNDATION DOWELS SHALL MATCH SIZE AND SPACING OF SHEAR WALL VERTICAL BARS ABOVE. EXTEND DOWELS TO BOTTOM OF FOUNDATION WITH 90 DEGREE HOOK.
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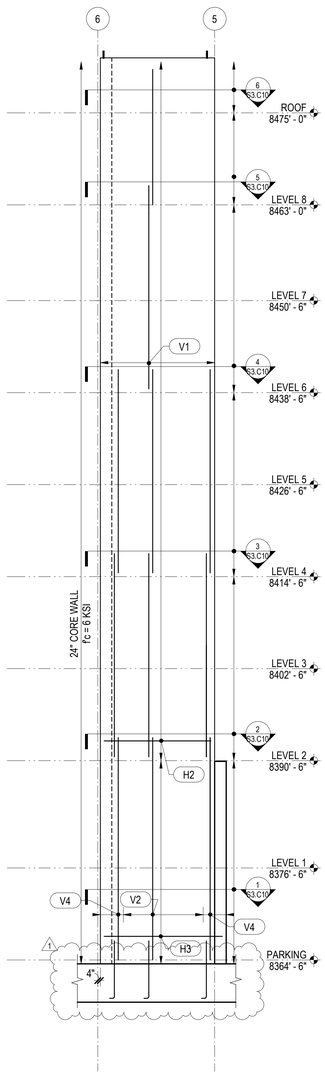
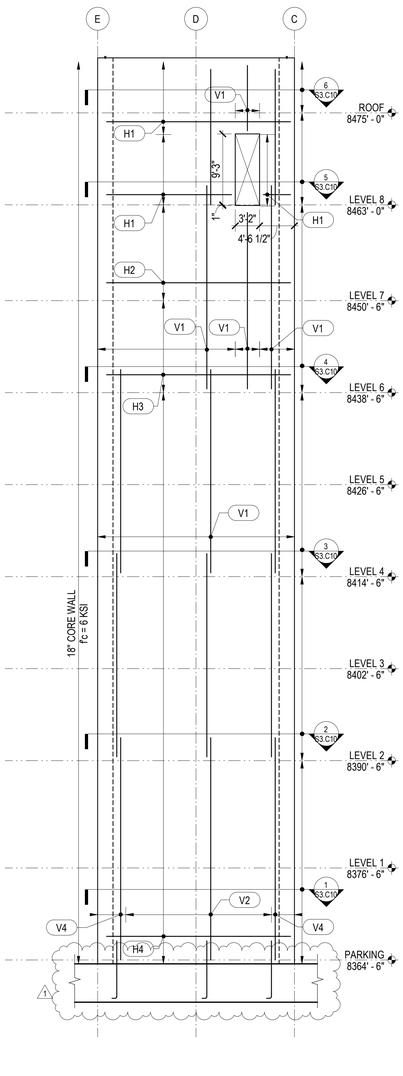
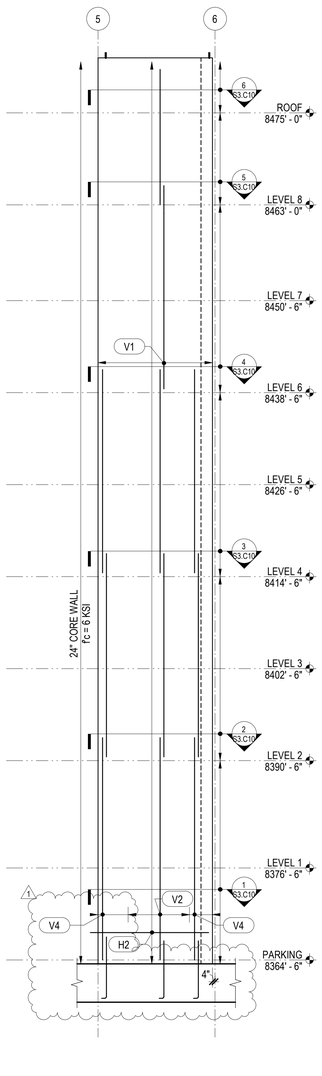


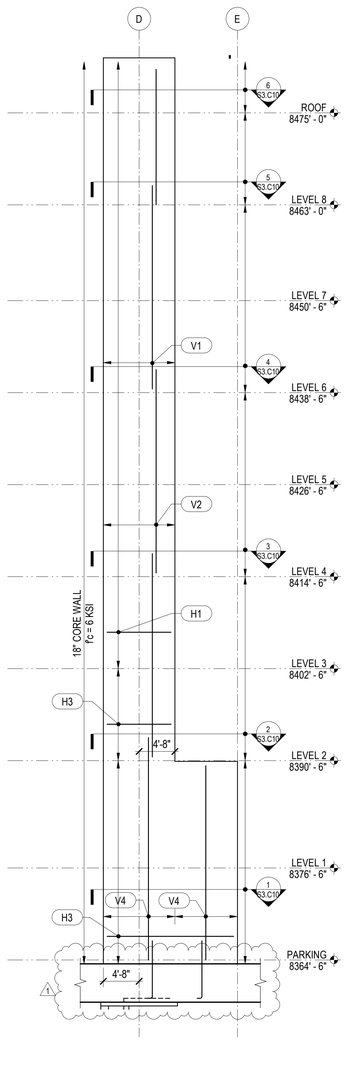
1 NORTH SHEAR WALL ELEVATION
1/8" = 1'-0"



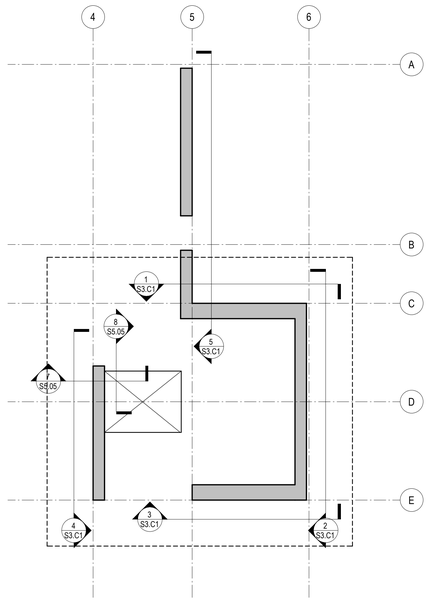
2 EAST SHEAR WALL ELEVATION
1/8" = 1'-0"



3 SOUTH SHEAR WALL ELEVATION
1/8" = 1'-0"



4 WEST BLADE WALL ELEVATION
1/8" = 1'-0"



TOWER C SHEAR WALL ELEVATION - KEY PLAN

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TOWER C SHEAR WALL ELEVATIONS

S3.C1

SHEAR WALL VERTICAL REINFORCEMENT SCHEDULE		
MARK	REINFORCING	REMARKS
V1	#6 @ 12"	EF
V2	#7 @ 12"	EF
V4	#7 @ 6"	EF
V5	#8 @ 6"	EF
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H5	#7 @ 6"	EF
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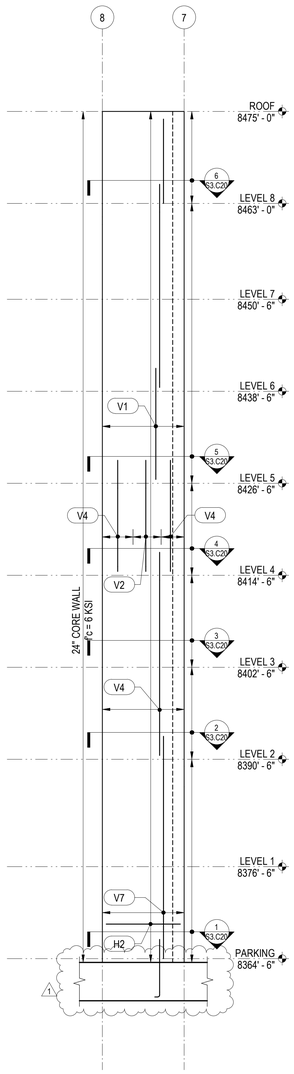
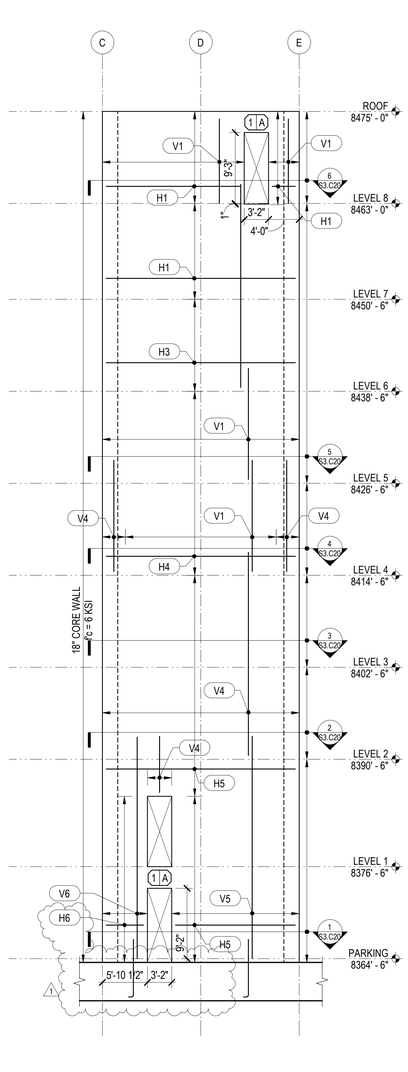
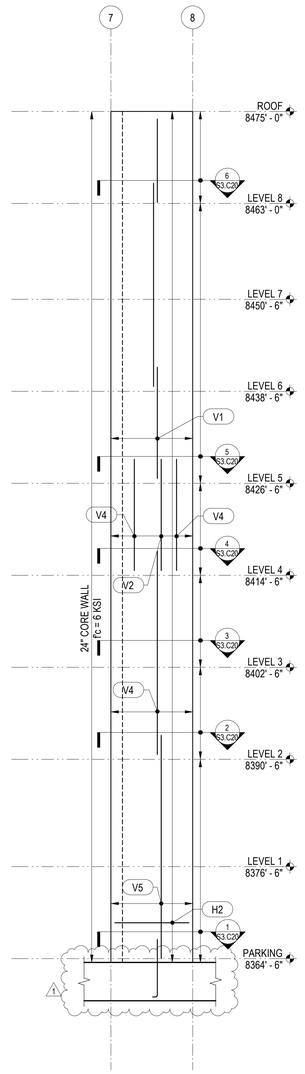


1 NORTH SHEAR WALL ELEVATION
1/8" = 1'-0"



2 WEST SHEAR WALL ELEVATION
1/8" = 1'-0"



3 SOUTH SHEAR WALL ELEVATION
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S5.XX CONCRETE SECTIONS AND DETAILS
S6.XX STEEL SECTIONS AND DETAILS

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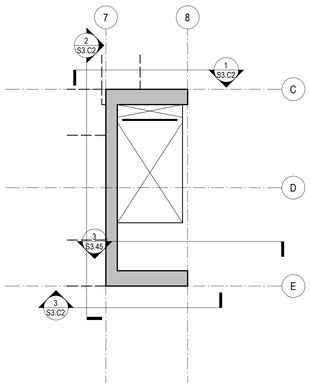

TOWER C SHEAR WALL ELEVATION - KEY PLAN

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Olson Kundig
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project:
SOMMET BLANC - ABC
DEER VALLEY, UTAH

MAGNUSSON KLEMENCIC ASSOCIATES
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Seattle Chicago
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206 292 1200

principal architect
project manager
drawn by Author
checked by Checker
job no. 20052
date 7/15/2022

revisions:

no.	date	by
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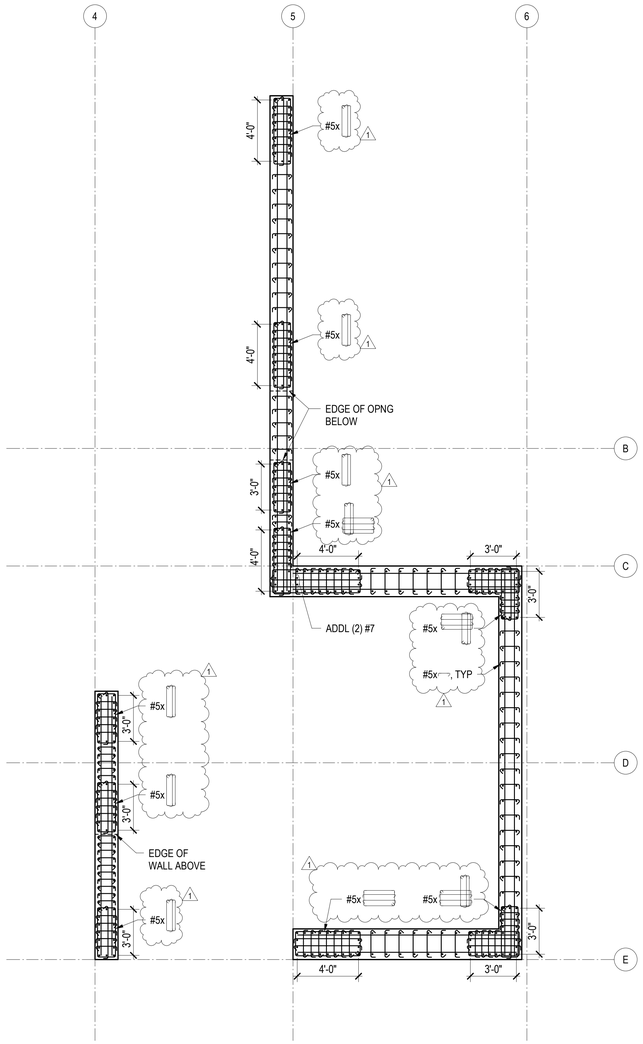
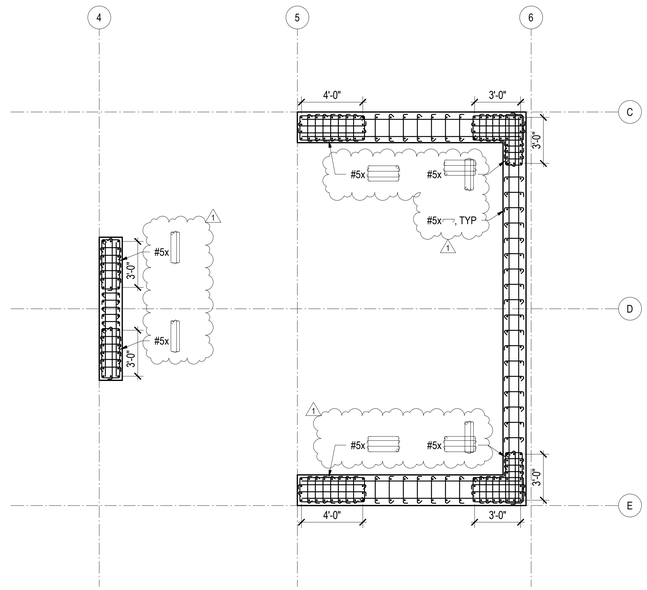
TOWER C SHEAR WALL ELEVATIONS

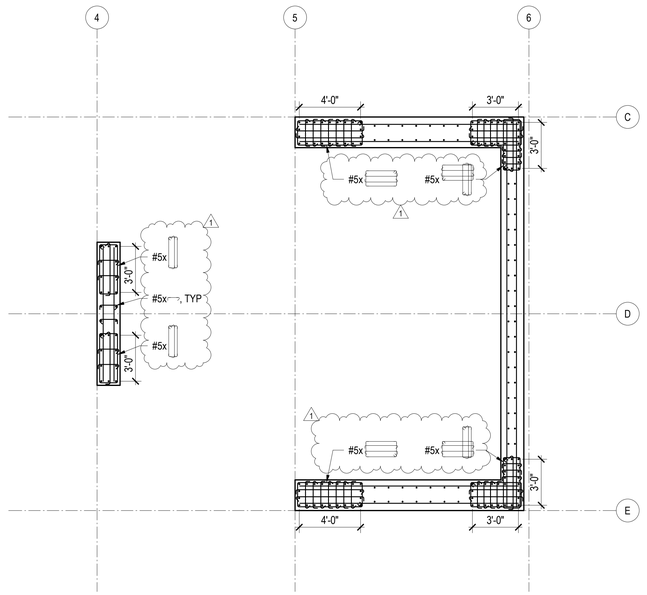
S3.C2



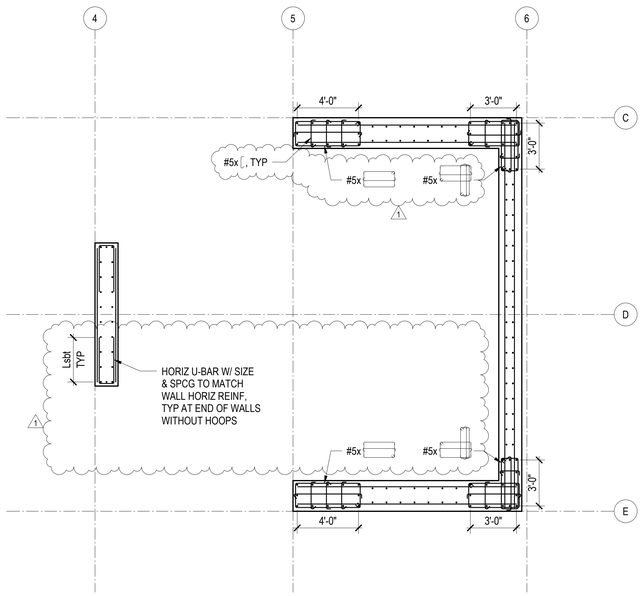
1 SHEAR WALL SECTION - FOUNDATION TO LEVEL 2
1/4" = 1'-0"



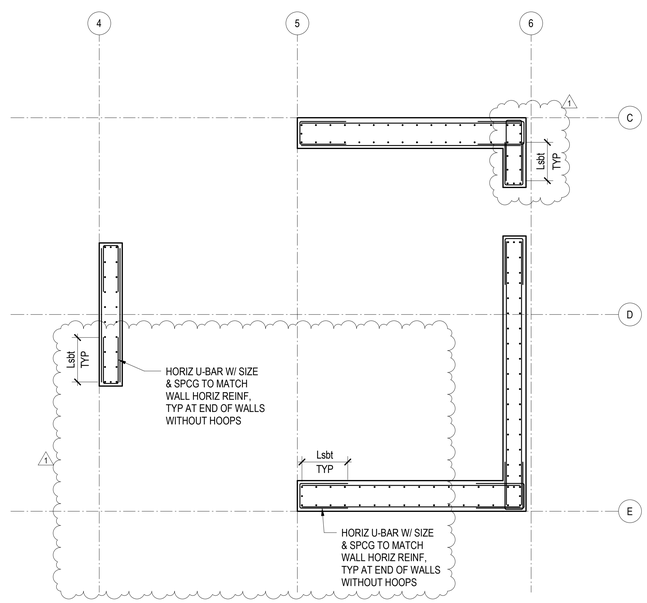
2 SHEAR WALL SECTION - LEVEL 2 TO LEVEL 4
1/4" = 1'-0"



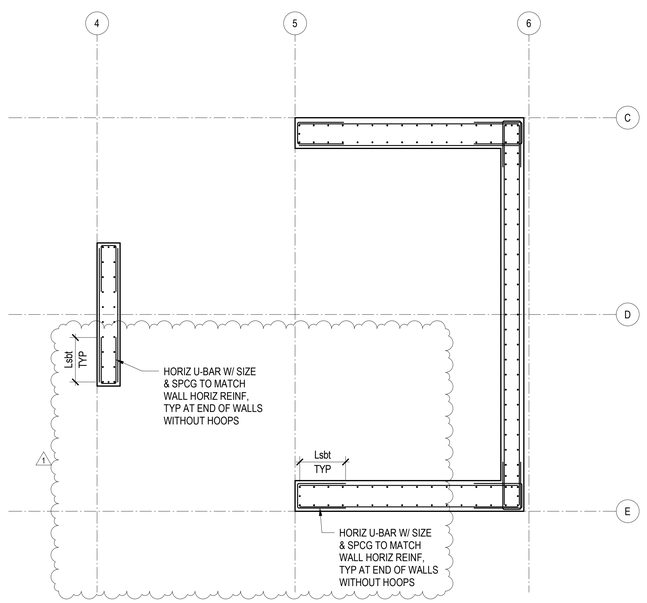
3 SHEAR WALL SECTION - LEVEL 4 TO LEVEL 6
1/4" = 1'-0"



4 SHEAR WALL SECTION - LEVEL 6 TO LEVEL 8
1/4" = 1'-0"



5 SHEAR WALL SECTION - LEVEL 8 TO ROOF
1/4" = 1'-0"



6 SHEAR WALL SECTION - ROOF TO TOP OF CORE
1/4" = 1'-0"

NOTES:

1. ALL HOOPS AND CROSSTIES SHALL BE #5 A615 GRADE 80 PLACED AT 6-INCH VERTICAL SPACING, UNLESS NOTED OTHERWISE. CROSSTIES WITH 90 DEGREE HOOKS SHALL HAVE THE CONSECUTIVE CROSSTIES ALTERNATED END-FOR-END ALONG THE LONGITUDINAL REINFORCEMENT.
2. CONCRETE COVER TO REINFORCEMENT IS 1-INCH.
3. BAR SIZE OF ADDED VERTICAL BARS TO MATCH SIZE AND GRADE OF ADJACENT REINFORCEMENT, UNLESS NOTED OTHERWISE. AT CORNERS, MATCH REINFORCEMENT SIZE OF THE LARGER OF THE TWO WALLS.

SHEAR WALL SECTION NOTES

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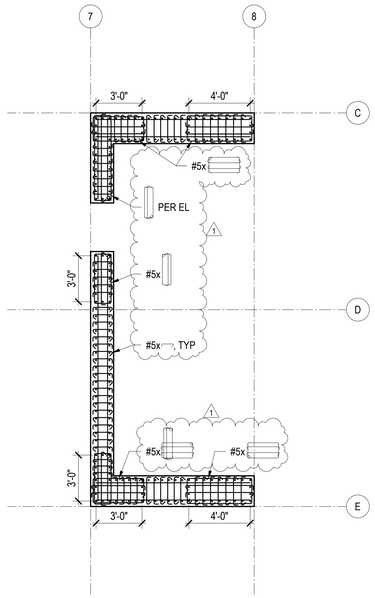
TOWER C SHEAR WALL SECTIONS

S3.C10

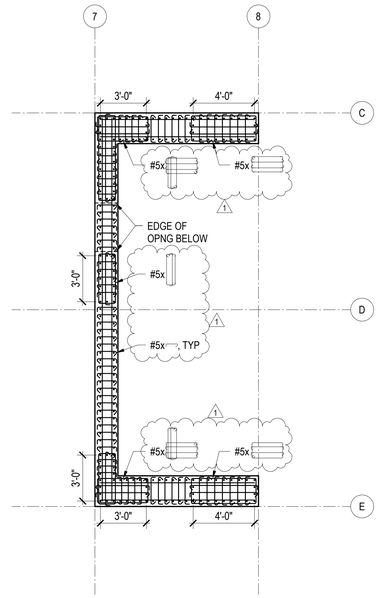
NOTES:

1. ALL HOOPS AND CROSSTIES SHALL BE #5 A615 GRADE 80 PLACED AT 6-INCH VERTICAL SPACING, UNLESS NOTED OTHERWISE. CROSSTIES WITH 90 DEGREE HOOKS SHALL HAVE THE CONSECUTIVE CROSSTIES ALTERNATED END-FOR-END ALONG THE LONGITUDINAL REINFORCEMENT.
2. CONCRETE COVER TO REINFORCEMENT IS 1-INCH.
3. BAR SIZE OF ADDED VERTICAL BARS TO MATCH SIZE AND GRADE OF ADJACENT REINFORCEMENT, UNLESS NOTED OTHERWISE. AT CORNERS, MATCH REINFORCEMENT SIZE OF THE LARGER OF THE TWO WALLS.

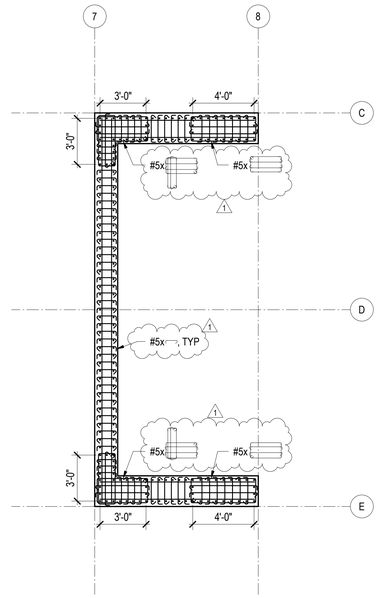
SHEAR WALL SECTION NOTES



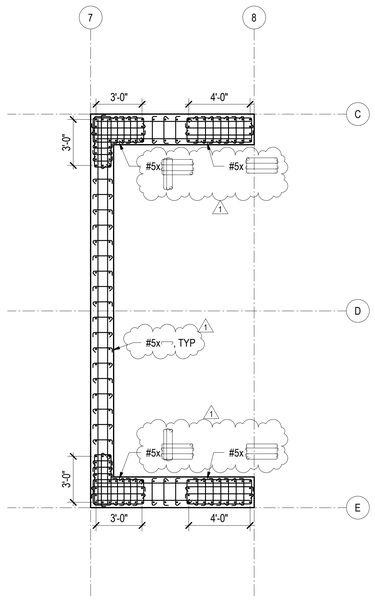
1 SHEAR WALL SECTION - FOUNDATION TO LEVEL 2
1/4" = 1'-0"



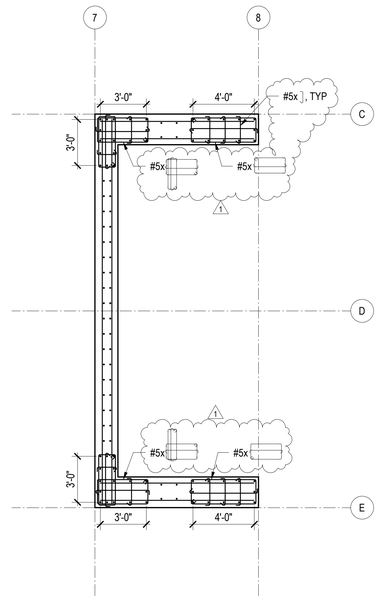
2 SHEAR WALL SECTION - LEVEL 2 TO LEVEL 3
1/4" = 1'-0"



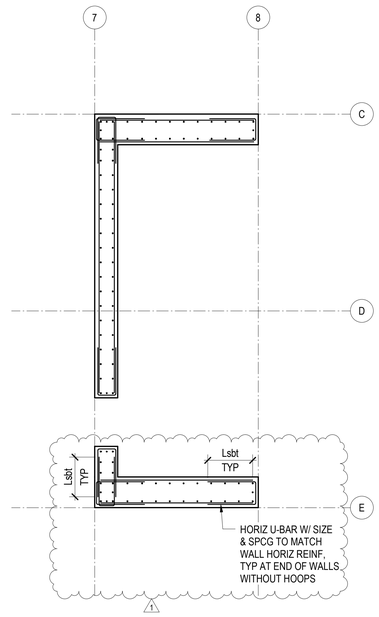
3 SHEAR WALL SECTION - LEVEL 3 TO LEVEL 4
1/4" = 1'-0"



4 SHEAR WALL SECTION - LEVEL 4 TO LEVEL 5
1/4" = 1'-0"



5 SHEAR WALL SECTION - LEVEL 5 TO LEVEL 8
1/4" = 1'-0"



6 SHEAR WALL SECTION - LEVEL 8 TO TOP OF CORE
1/4" = 1'-0"

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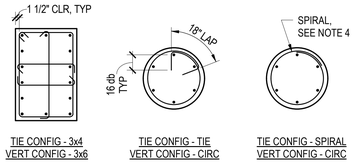
TOWER C SHEAR WALL SECTIONS

S3.C20

CONCRETE COLUMN TYPE SCHEDULE							
TYPE	LONGIT Fy	TRANSV Fy	VERT REINF	TIE CONFIG	VERT CONFIG	LOC 1 TIES	LOC 2 TIES
1	60	80	(12) #9	3x5 (s)	3x5	#4 @ 4 1/2"	#5 @ 4"
2	60	80	(16) #8	5x5	5x5	#4 @ 6"	#5 @ 5 1/2"
3	60	80	(8) #8	3x3	3x3	#4 @ 5 1/2"	#5 @ 4 1/2"
4	60	80	(12) #7	3x5 (s)	3x5	#4 @ 4 1/2"	#5 @ 4 1/2"
5	60	80	(10) #7	3x4	3x4	#4 @ 4 1/2"	#5 @ 4 1/2"
6	60	80	(12) #7	4x4 (s)	4x4	#4 @ 5"	#5 @ 5"
7	60	80	(14) #9	3x6 (s)	3x6	#4 @ 5"	#5 @ 5"
8	60	80	(12) #7	2x6 (s)	2x6	#4 @ 3"	#5 @ 3"
10	60	80	(14) #9	4x5	4x5	#4 @ 5 1/2"	#5 @ 5 1/2"
11	60	80	(14) #10	3x6 (s)	3x6	#4 @ 4 1/2"	#5 @ 4"
12	60	80	(16) #8	2x8 (s)	2x8	#4 @ 3"	#5 @ 3"
13	60	80	(20) #8	5x7	5x7	#4 @ 6"	#5 @ 6"
14	60	80	(14) #11	3x6 (s)	3x6	#4 @ 4 1/2"	#5 @ 4"

NOTES:

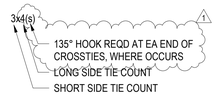
1. TYPICAL CROSSTIES SHALL HAVE A 135 DEGREE HOOK AT ONE END AND A 90 DEGREE HOOK AT THE OTHER END UNLESS NOTED OTHERWISE. AT CONTRACTOR'S OPTION, 135 DEGREE HOOKS MAY BE REPLACED WITH 180 DEGREE HOOKS AND 90 DEGREE HOOKS MAY BE REPLACED WITH 135 OR 180 DEGREE HOOKS.
2. CROSSTIES WITH 90 DEGREE HOOKS SHALL HAVE THE CONSECUTIVE CROSSTIES ALTERNATED END FOR END ALONG THE LONGITUDINAL REINFORCEMENT.
3. CIRCULAR TIES SHALL ALTERNATE POSITION OF LAPS 180 DEGREES EVERY OTHER HOOP.
4. REFER TO "TYPICAL CONCRETE COLUMN SPIRAL REINFORCING" FOR ADDITIONAL DETAILING REQUIRMENTS.



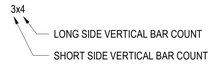
TIE CONFIG - 3x4 VERT CONFIG - 3x6 | TIE CONFIG - TIE VERT CONFIG - CIRC | TIE CONFIG - SPIRAL VERT CONFIG - CIRC

SAMPLE CONFIGURATIONS

TIE CONFIGURATION KEY:

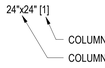


VERTICAL REINF CONFIGURATION KEY:



1 CONCRETE COLUMN TYPES

CONCRETE COLUMN SCHEDULE KEY:



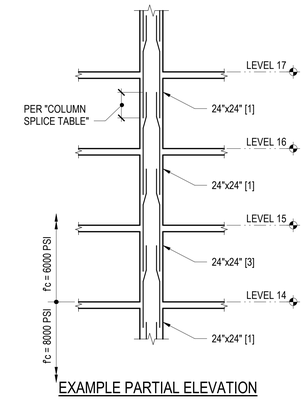
NOTES:

1. SEE THE FOLLOWING ACCOMPANYING DETAILS:
   "TYPICAL CONCRETE COLUMN"
   "TYPICAL CONCRETE COLUMN BASE DOWELS"
   "CONCRETE COLUMN TYPES"
2. VERTICAL REINFORCEMENT SPLICE LENGTHS ARE PER THE "TYPICAL COLUMN SPLICE TABLE" .

CONCRETE COLUMN SCHEDULE			
	COLUMN MARK	C1	C2
f'c = 6,000 PSI	LEVEL 20		
	LEVEL 19		
	LEVEL 18		
	LEVEL 17		24"x24" [1]
	LEVEL 16	24"x24" [1]	
	LEVEL 15	24"x24" [3]	
	LEVEL 14		
f'c = 8,000 PSI	LEVEL 13		
	LEVEL 12	24"x24" [1]	
	LEVEL 11		
	LEVEL 10		
	LEVEL 9		
	LEVEL 8	24"x30" [1]	30"x30" [2]

COLUMN SIZE, TYPE & SPLICE LENGTH

EXAMPLE PARTIAL CONCRETE COLUMN SCHEDULE



EXAMPLE PARTIAL ELEVATION

3 CONCRETE COLUMN SCHEDULE NOTES AND SAMPLE COLUMN SPECIFICATIONS

TOWER A CONCRETE COLUMN SCHEDULE																		
f'c = 6,000 PSI	LEVEL 6		18"x32" [1]															
	LEVEL 5								18"x32" [4]	18"x32" [1]								
	LEVEL 4																	
	LEVEL 3			18"x32" [4]		18"x32" [5]	24"x24" [3]	18"x32" [5]			18"x32" [5]							
	LEVEL 2	24"x24" [3]	18"x32" [5]	32"x32" [2]		32"x32" [2]	24"x54" [7]	32"x32" [2]					18"x32" [2]					24"x24" [3]
	LEVEL 1								18"x32" [5]			24"x24" [3]	18"x32" [2]	18"x32" [5]	18"x32" [5]	18"x32" [5]	24"x24" [3]	
	LEVEL P2	24"x24" [6]	18"x32" [4]	18"x32" [1]	12"x60" [8]	18"x32" [4]	24"x24" [6]	18"x32" [5]		18"x32" [5]		24"x24" [6]	18"x32" [4]	24"x24" [3]				12"x60" [8]
	FOUNDATION																	
	COLUMN MARK	AC1	AC2	AC3	AC4	AC5	AC6	AC7	AC8	AC9	AC10	AC11	AC12	AC13	AC14	AC15	AC16	AC17

6 TOWER A CONCRETE COLUMN SCHEDULE

TOWER B CONCRETE COLUMN SCHEDULE																				
f'c = 6,000 PSI	LEVEL 7					18"x32" [1]														
	LEVEL 6																			
	LEVEL 5																			
	LEVEL 4			18"x32" [5]																
	LEVEL 3		18"x32" [5]	32"x40" [10]	24"x24" [3]		24"x24" [3]		24"x24" [3]		18"x32" [5]	18"x32" [5]	18"x32" [5]	18"x32" [5]	18"x32" [5]					
	LEVEL 2		32"x32" [2]			18"x32" [5]	24"x54" [7]		32"x32" [2]		32"x41" [10]	18"x32" [4]	32"x32" [2]	32"x42" [10]	32"x32" [2]					
	LEVEL 1									18"x32" [5]			12"x60" [12]	12"x60" [12]	12"x60" [12]					24"x24" [3]
	LEVEL P1	18"x32" [5]		18"x32" [4]	24"x24" [6]	18"x32" [4]	24"x24" [6]		24"x24" [6]							12"x60" [8]				
	LEVEL P2		18"x32" [4]					12"x60" [8]		18"x32" [4]	18"x32" [4]						18"x32" [5]	24"x24" [3]	24"x24" [6]	18"x32" [5]	
	FOUNDATION																			
	COLUMN MARK	BC1	BC2	BC3	BC4	BC5	BC6	BC7	BC8	BC9	BC10	BC11	BC12	BC13	BC14	BC15	BC16	BC17	BC18	BC19	BC20

11 TOWER B CONCRETE COLUMN SCHEDULE

TOWER C CONCRETE COLUMN SCHEDULE												
f'c = 6,000 PSI	LEVEL 8	18"x32" [11]			18"x32" [11]			18"x32" [11]			18"x32" [14]	
	LEVEL 7											
	LEVEL 6											
	LEVEL 5											
	LEVEL 4							18"x32" [5]		18"x32" [5]		
	LEVEL 3		12"x48" [12]		18"x32" [5]	12"x48" [12]		32"x32" [2]		33"x34" [2]		
	LEVEL 2		30"x48" [13]		32"x32" [2]							18"x32" [5]
	LEVEL 1	18"x32" [5]	18"x32" [4]	24"x24" [6]	18"x32" [1]	18"x36" [4]	18"x24" [4]	18"x32" [4]	18"x32" [5]	18"x32" [4]	18"x32" [5]	
	FOUNDATION											
	COLUMN MARK	CC1	CC2	CC3	CC4	CC5	CC6	CC7	CC8	CC9	CC10	CC11

16 TOWER C CONCRETE COLUMN SCHEDULE

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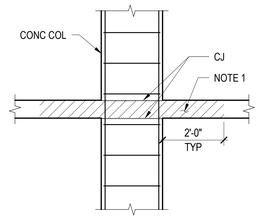
COLUMN SCHEDULES

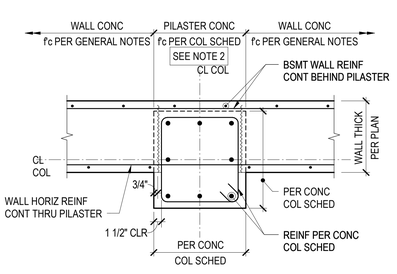
S4.00



NOTES:

1. WHERE THE COLUMN CONCRETE f'c IS GREATER THAN 1.4 TIMES THE SLAB CONCRETE f'c, PUDDLE CONCRETE WITH f'c EQUAL TO OR GREATER THAN THE COLUMN CONCRETE f'c WITHIN THE HATCHED AREA.

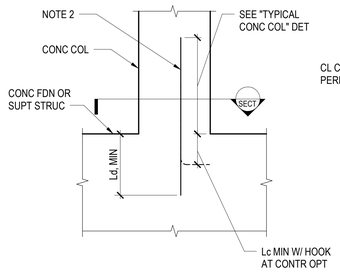
5 TYPICAL CONCRETE PLACEMENT AT COLUMN TO SLAB JOINT



NOTES:

1. CENTERLINES OF PILASTER AND COLUMN ABOVE COINCIDE UNLESS NOTED OTHERWISE ON PLAN.
2. CONCRETE AT PILASTER TO BE CAST IN PLACE. SHOTCRETE NOT PERMITTED AT PILASTER.

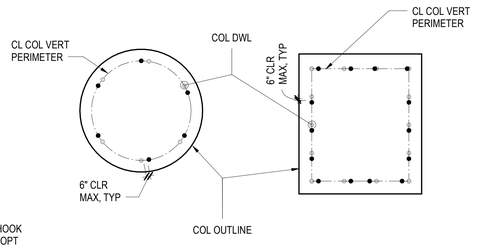
8 TYPICAL PILASTER



NOTES:

1. THIS DETAIL IS APPLICALBE AT ALL CONCRETE COLUMN BASE LOCATIONS UNLESS NOTED OTHERWISE.
2. DOWEL SIZE AND QUANTITY TO MATCH VERTICAL REINFORCEMENT.

9 TYPICAL CONCRETE COLUMN BASE DOWELS



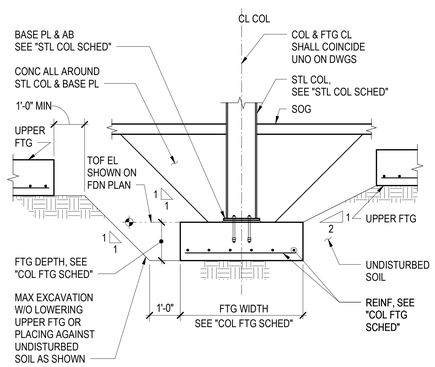
SECTION

COLUMN FOOTING SCHEDULE				
MARK	SIZE	DEPTH	REINFORCING EACH WAY BOTTOM	REMARKS
F1	6' - 0"x6' - 0"	2' - 6"	#8 @ 12"	#5 @ 6" EW TOP REINF
F2	10' - 0"x10' - 0"	2' - 0"	(10) #7	
F3	12' - 0"x12' - 0"	2' - 0"	(12) #8	

NOTES:

1. SEE TYPICAL COLUMN FOOTING DETAILS.

11 COLUMN FOOTING SCHEDULE

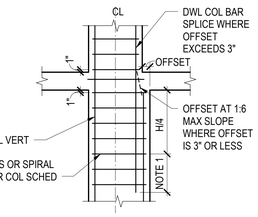


16 TYPICAL STEEL COLUMN FOOTING

TYPICAL COLUMN SPLICE TABLE (INCHES)						
BAR SIZE	fy = 60 ksi	fy = 60 ksi	fy = 60 ksi	fy = 80 ksi	fy = 80 ksi	fy = 80 ksi
	f'c = 6 ksi	f'c = 8 ksi	f'c = 10 ksi	f'c = 6 ksi	f'c = 8 ksi	f'c = 10 ksi
#7	27	27	27	42	42	42
#8	31	31	31	48	48	48
#9	35	35	35	55	55	55
#10	39	39	39	61	61	61
#11	43	43	43	68	68	68

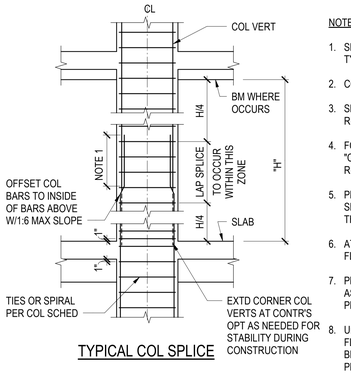
NOTES:

1. SEE "TYPICAL COLUMN SPLICE TABLE" FOR SPLICE LENGTH UNLESS NOTED OTHERWISE.
2. CLEAR DISTANCE BETWEEN THE TWO LAP SPLICE BARS AND ADJACENT BARS SHALL BE NOT LESS THAN 1.5 TIMES THE BAR DIA, NOR 1 1/2 INCHES
3. IF OFFSET IN VERTICAL BARS OCCURS OUTSIDE THE HORIZONTAL FRAMING, PROVIDE ADDITIONAL SET OF TIES AT OFFSET LOCATION.



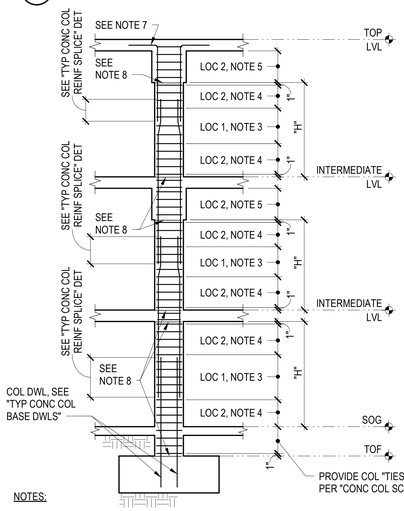
ABOVE SLAB BAR OFFSET

17 TYPICAL CONCRETE COLUMN REINFORCING SPLICE



TYPICAL COL SPLICE

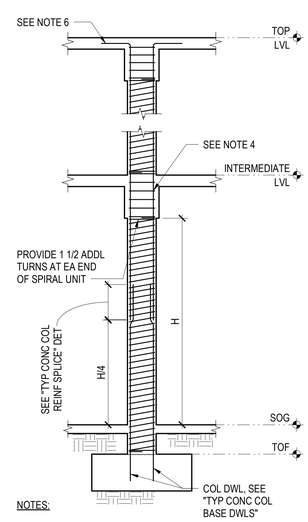
19 TYPICAL CONCRETE COLUMN



NOTES:

1. SEE "COLUMN COLUMN SCHEDULE" FOR COLUMN SIZE AND REINFORCING CONFIGURATION TYPE.
2. COLUMN VERTICAL REINFORCING SHALL BE SPECIAL DUCTILE QUALITY. SEE "GENERAL NOTES."
3. SEE "CONCRETE COLUMN TYPE SCHEDULE" UNDER "LOC 1 TIES" FOR TIE SIZE, SPACING, AND REINFORCING CONFIGURATION. REINFORCING CONFIGURATION TYPE IS NOTED IN [ ].
4. FOR H/6, MAXIMUM COLUMN DIMENSION, OR 18 INCHES (WHICHEVER IS GREATER), SEE "COLUMN TYPE SCHEDULE" UNDER "LOC 2 TIES" FOR TIE SIZE, SPACING, AND REINFORCING CONFIGURATION. REINFORCING CONFIGURATION TYPE IS NOTED IN [ ].
5. PROVIDE "LOC 2 TIES" THROUGHOUT THE BEAM AND SLAB DEPTH, EXCEPT THAT THE TIE SPACING MAY BE INCREASED TO 6" WHERE BEAMS FRAME INTO FOUR SIDES OF COLUMN FOR THE DEPTH OF THE MOST SHALLOW BEAM.
6. AT CONTRACTOR'S OPTION, COLUMN VERTICAL BARS MAY BE EXTENDED UP ADDITIONAL FLOOR LEVELS WITHOUT SPLICING BETWEEN FLOORS.
7. PLACE HORIZONTAL HOOKS DIRECTLY BELOW TOP BARS OF BEAMS OR SLABS. SPLAY HOOKS AS NECESSARY TO RELIEVE BAR CONGESTION. AT CONTRACTOR'S OPTION, HOOKS MAY BE PLACED TOWARD THE INSIDE OF THE COLUMN.
8. UNLESS NOTED OTHERWISE, COLUMN CONSTRUCTION JOINTS SHALL BE AT THE UNDERSIDE OF FLOOR SLABS, BEAMS, OR GIRDERS, AND AT THE TOPS OF FOOTINGS OR FLOOR SLABS. BEAMS, GIRDERS, BRACKETS, COLUMNS CAPITALS, HAUNCHES, AND DROP PANELS SHALL BE PLACED AT THE SAME TIME AS SLABS.

20 TYP CONCRETE COLUMN SPIRAL REINFORCING



NOTES:

1. USE THIS DETAIL WHEN NOTED "SPIRAL" IN THE "CONCRETE COLUMN TYPE SCHEDULE."
2. SEE "CONCRETE COLUMN SCHEDULE" FOR COLUMN SIZE AND REINFORCING CONFIGURATION TYPE.
3. SPIRALS SHALL BE CONTINUOUS FROM TOP OF SLAB OR FOOTING TO SLAB OR BEAM SOFFIT ABOVE.
4. EXTEND SPIRALS CONTINUOUSLY THROUGHOUT THE BEAM AND SLAB DEPTH OR PROVIDE CIRCULAR TIES AT SIZE AND SPACING TO MATCH SPIRAL SIZE AND PITCH. THE TIE SPACING MAY BE INCREASED TO 6" WHERE BEAMS FRAME INTO FOUR SIDES OF THE COLUMN FOR THE DEPTH OF THE MOST SHALLOW BEAM.
5. AT CONTRACTOR'S OPTION, COLUMN VERTICAL BARS MAY BE EXTENDED UP ADDITIONAL FLOOR LEVELS WITHOUT SPLICING BETWEEN FLOORS.
6. PLACE HORIZONTAL HOOKS DIRECTLY BELOW TOP BARS OF BEAMS OR SLABS. SPLAY HOOKS AS NECESSARY TO RELIEVE BAR CONGESTION. AT CONTRACTOR'S OPTION, HOOKS MAY BE PLACED TOWARD THE INSIDE OF THE COLUMN.
7. UNLESS NOTED OTHERWISE, COLUMN CONSTRUCTION JOINTS SHALL BE AT THE UNDERSIDE OF FLOOR SLABS, BEAMS, OR GIRDERS, AND AT THE TOPS OF FOOTINGS OR FLOOR SLABS. BEAMS, GIRDERS, BRACKETS, COLUMNS CAPITALS, HAUNCHES, AND DROP PANELS SHALL BE PLACED AT THE SAME TIME AS SLABS.

Reserved for permit stamp

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date 7/15/2022

revisions:

no.	date	by

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7/15/2022

TYPICAL CONCRETE COLUMN DETAILS

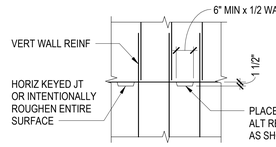
S4.01

NOTES:

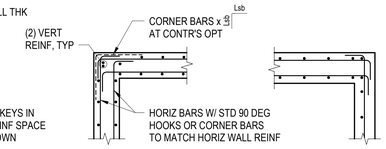
1. UNLESS NOTED OR SHOWN OTHERWISE, ALL WALLS ARE TO BE REINFORCED WITH MINIMUM REINFORCEMENT AS SHOWN IN THE FOLLOWING TABLE:

MINIMUM WALL REINFORCEMENT			
WALL THICKNESS	HORIZONTAL BARS	VERTICAL BARS	LOCATION
6" & UNDER	#4 @ 12"	#4 @ 12"	CENTERLINE
OVER 6-8"	#5 @ 15"	#5 @ 15"	CENTERLINE
OVER 8-10"	#5 @ 12"	#5 @ 12"	CENTERLINE
OVER 10-12"	#4 @ 12"	#4 @ 12"	EACH FACE
OVER 12-14"	#5 @ 18"	#5 @ 18"	EACH FACE
OVER 14-16"	#5 @ 15	#5 @ 15"	EACH FACE
OVER 16-20"	#5 @ 12"	#5 @ 12"	EACH FACE
OVER 20-24"	#5 @ 10"	#5 @ 10"	EACH FACE

2. LAP VERTICAL WALL REINFORCING Lsb AND HORIZONTAL WALL REINFORCING Lsbt AT SPLICES.
3. ALL VERTICAL REINFORCING IN CONCRETE WALLS SHALL BE CONTINUOUS FROM STRUCTURAL FLOOR TO STRUCTURAL FLOOR, OR FROM FOOTING TO FIRST STRUCTURAL FLOOR ABOVE UNLESS NOTED OTHERWISE.
4. START HORIZONTAL AND VERTICAL BARS 1 INCH CLEAR OF EDGE OF OPENINGS. SPACE REINFORCING BARS AT EQUAL SPACES NOT TO EXCEED REQUIRED SPACING.
5. SMALL WALL SECTION DETAILS APPLY BOTH IN HORIZONTAL AND VERTICAL DIRECTIONS.
6. WALL DETAILS APPLY TO FOUNDATION AND MISC CONCRETE WALLS SHOWN ON THE STRUCTURAL DRAWINGS. FOR APPLICATION TO OTHER WALLS, CONTACT THE ENGINEER FOR APPROVAL.

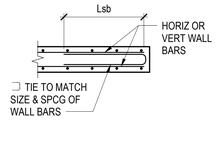


HORIZONTAL JOINT ELEVATION

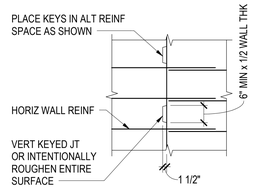


WALL CORNER

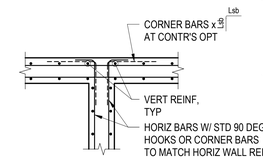
WALL CORNER OF SINGLE & DOUBLE LAYER REINFORCING



WALL END

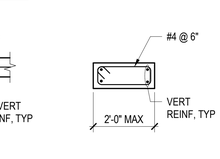


VERTICAL JOINT ELEVATION



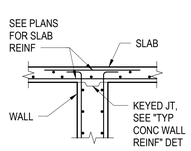
WALL INTERSECTION

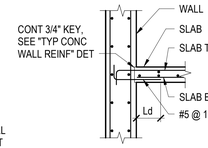
WALL INTERSECTION OF SINGLE & DOUBLE LAYER REINFORCING

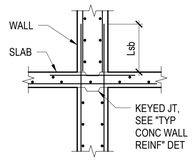


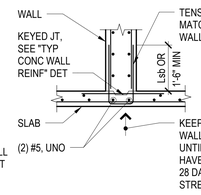
SMALL WALL SECTION
SEE NOTE 5

3 TYPICAL CONCRETE WALL REINFORCING







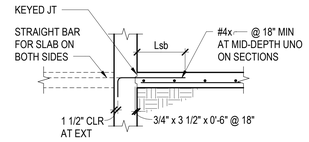


NOTES:

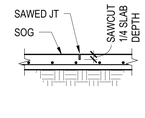
1. SLAB ON GRADE IS 5 INCHES THICK UNLESS NOTED OTHERWISE.
2. LOCATE CONSTRUCTION JOINTS UNDER PARTITIONS OR ON COLUMN LINES. PROVIDE CONTROL JOINTS ON ALL COLUMN LINES AND AT A MAXIMUM SPACING OF 30 x SLAB THICKNESS EACH WAY IN BETWEEN. PROVIDE CONTROL JOINTS AT ALL RE-ENTRANT CORNERS. CONTRACTOR SHALL SUBMIT A JOINTING PLAN TO ARCHITECT FOR REVIEW.
3. SAWED JOINTS SHALL BE MADE AS SOON AS THE CONTROL JOINT CAN BE CUT WITHOUT EDGES RAVELING AND WITHIN 24 HOURS OF SLAB PLACEMENT. SAWED JOINTS SHALL BE FILLED WITH SEALANT AS COORDINATED WITH THE ARCHITECT.
4. LOCATE REINFORCING AT MID-DEPTH OF SLAB UNO.
5. TYPICAL SLAB REINFORCING:
   4" - 6" SLAB: #4 @ 18" EACH WAY
   GREATER THAN 6" SLAB: #5 @ 18" EACH WAY BOT
   #4 @ 18" EACH WAY TOP

TYPICAL CONCRETE REINFORCING AT INTERSECTION OF SLABS AND WALLS

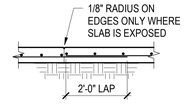
8 TYPICAL SLAB ON GRADE



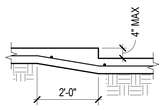
ADJACENT TO WALL



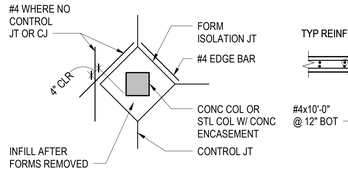
CONTROL JOINT



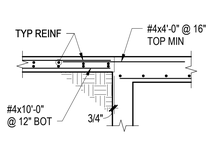
CONSTRUCTION JOINT



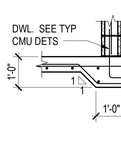
MINOR STEP



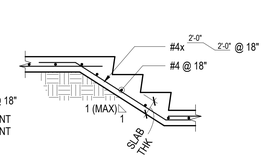
ISOLATION JOINT AT COLUMN



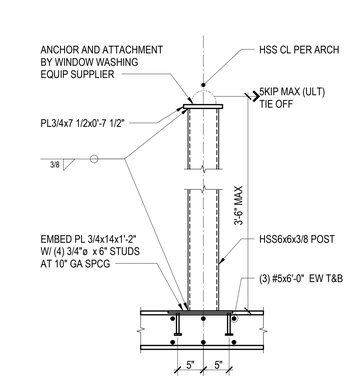
ADJACENT TO STRUCTURAL SLAB



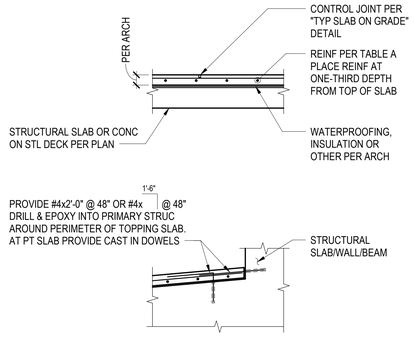
SUPPORTING CMU



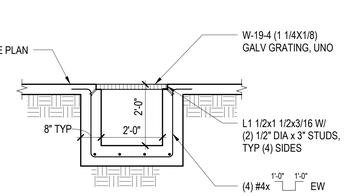
STEPS ON GRADE



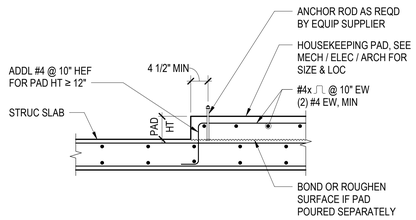
11 TYPICAL FALL ARREST POST



PERIMETER



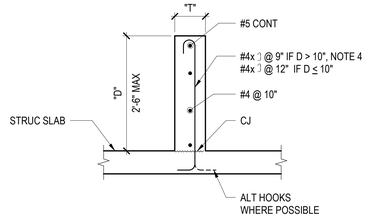
13 TYPICAL SUMP PIT



NOTES:

1. HOUSEKEEPING PADS ON SLAB-ON-GRADE AND COMPOSITE SLABS SIMILAR.

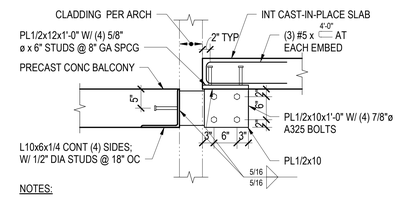
14 TYPICAL HOUSEKEEPING PAD



NOTES:

1. T = 6" MINIMUM OR 10" MAXIMUM. IF T > 10", SEE "TYPICAL HOUSEKEEPING PAD" DETAIL.
2. SEE ARCHITECTURAL FOR T AND D DIMENSIONS AND CURB LOCATIONS.
3. CONCRETE CURBS ON SLAB-ON-GRADE AND FORMED SLABS ARE SIMILAR.

15 TYPICAL CONCRETE CURB



NOTES:

1. CONNECTION TO BE PROVIDED AT 4'-0" MAX SPACING. MINIMUM OF (2) CONNECTIONS PER BALCONY EDGE. FIRST CONNECTION TO OCCUR NO MORE THAN 12" FROM ANY EDGE OF BALCONY SLAB.

16 TYPICAL PRECAST BALCONY CONNECTION

TABLE A	
THICKNESS (IN)	MINIMUM REINFORCING
2	#3 @ 12" EW OR WWR 6X6 - W2.5XW2.5
3	#3 @ 12" EW OR WWR 6X6 - W3.5XW3.5
4	#3 @ 12" EW OR WWR 6X6 - W4.5XW4.5
5	#3 @ 12" EW OR WWR 6X6 - W5.5XW5.5
6	#4 @ 12" EW OR WWR 6X6 - W6.5XW6.5
7	#4 @ 12" EW OR WWR 6X6 - W8XW8
8	#4 @ 12" EW OR WWR 6X6 - W9XW9

NOTES:

1. SEE "TYPICAL SLAB ON GRADE" DETAILS FOR ADDITIONAL INFORMATION.
2. AT CONTRACTORS OPTION, AT DEPRESSED AREAS, ACCEPTABLE TO PROVIDE CONCRETE TOPPING OF MINIMUM THICKNESS REQUIRED PER ARCHITECTURAL (4" MIN) ON POLYSTYRENE PER "TYPICAL BUILT-UP SLAB ON POLYSTYRENE.

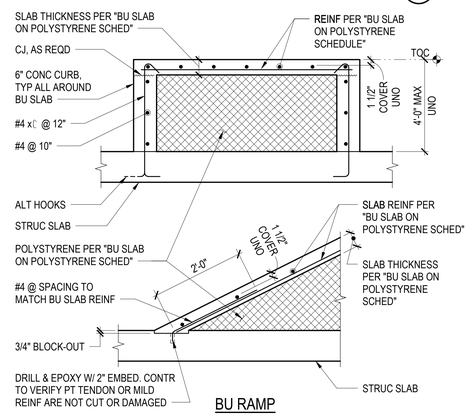
17 TYPICAL TOPPING SLAB

BUILT-UP SLAB ON POLYSTYRENE SCHEDULE				
TYPE	SLAB THICKNESS (INCH)	REINFORCEMENT	POLYSTYRENE STRENGTH - NOTE 1	REMARKS
1A	4	#4 @ 12" EW	3.6 PSI @ 1%	TYP UNO
1V	6	#5 @ 12" EW	7.3 PSI @ 1%	PROVIDE AT ALL AREAS SUBJECT TO VEHICLE LOADS

NOTES:

1. RIGID CELLULAR POLYSTYRENE WITH MINIMUM COMPRESSIVE RESISTANCE INDICATED AT 1% DEFORMATION CONFORMING TO ASTM D6817 OR ASTM C578 OR APPROVED EQUIVALENT.
2. PROVIDE 3/4" DEEP SAWCUT CONTROL JOINTS AT MAXIMUM SPACING OF 30 TIMES THE SLAB THICKNESS ON CENTER EACH WAY. PROVIDE CONTROL JOINTS AT ALL RE-ENTRANT CORNERS. CONTRACTOR SHALL SUBMIT A JOINTING PLAN TO THE ARCHITECT FOR REVIEW.
3. FOR MECHANICAL EQUIPMENT EXCEEDING 1000 POUNDS SUPPORTED ON THE SLAB, CONTACT THE ENGINEER FOR APPROVAL.
4. BUILT-UP SLAB AND CURB MINIMUM f'c = 4000 PSI.

18 TYPICAL BUILT-UP SLAB ON POLYSTYRENE



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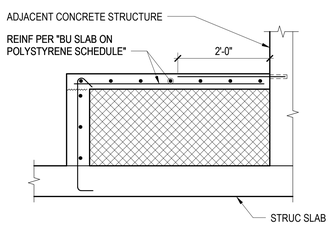

BU SLAB AT ADJACENT STRUCTURE

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checked by
job no. 20052
date 7/15/2022

revisions:

no.	date	by

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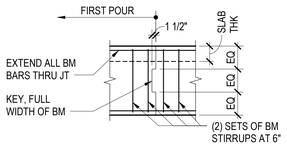
PROGRESS PRINT

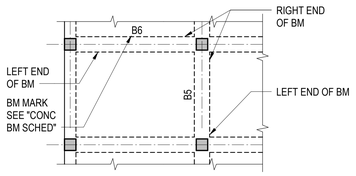
7/15/2022

TYPICAL CONCRETE DETAILS

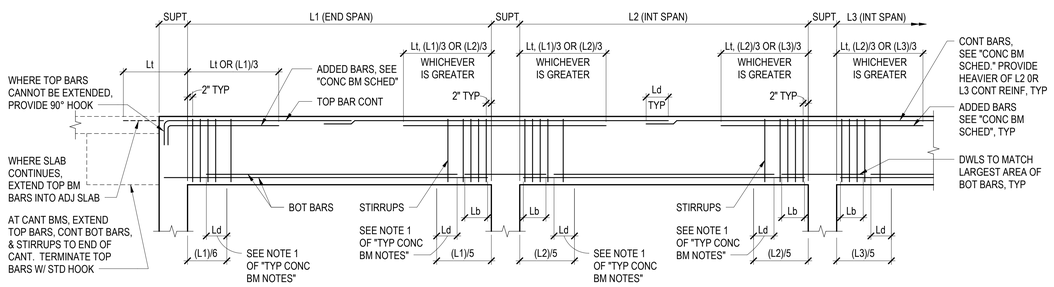
S4.02



BEAM CONSTRUCTION JOINT

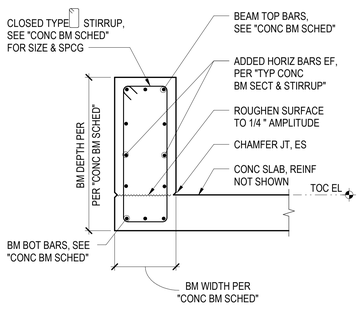


PLAN OF BEAMS

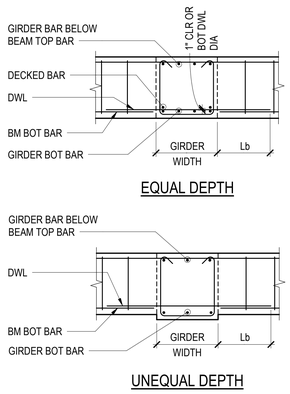


BEAM REINFORCING ELEVATION

11 TYPICAL CONCRETE BEAM



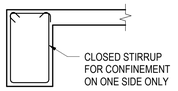
16 TYPICAL CONCRETE UPTURNED BEAM



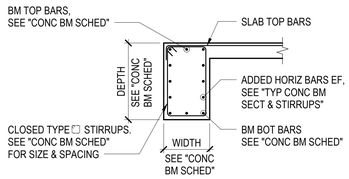
EQUAL DEPTH

UNEQUAL DEPTH

17 TYP CONC BM AND GIRDER INTERSECTION



ALTERNATE



18 TYPICAL CONCRETE EDGE BEAM

NOTES:

1. AT CONTRACTOR'S OPTION, WHERE REQUIRED TO RELIEVE BAR CONGESTION, NOT MORE THAN 50 PERCENT OF THE AREA OF THE STRAIGHT BOTTOM BARS MAY BE TERMINATED AS SHOWN UNLESS NOTED OTHERWISE.
2. BEAM SCHEDULES DO NOT INDICATE REQUIREMENTS FOR ARRANGING BARS. THE CONTRACTOR SHALL DETAIL AND PLACE REINFORCING STEEL IN A SINGLE LAYER WHENEVER POSSIBLE. A SECOND LAYER MAY BE USED ONLY WHERE REQUIRED TO PROVIDE PROPER CLEARANCES BETWEEN BARS IN A LAYER AND WHERE REQUIRED IN ORDER TO PROPERLY CLEAR COLUMN VERTICALS AND SIMILAR REINFORCING.
3. EITHER 90 OR 180 DEGREE STANDARD HOOK BARS MAY BE USED FOR LONGITUDINAL BARS.
4. WHERE TOP BARS ARE INDICATED AS CONTINUOUS AND RUN OVER 60 FEET IN LENGTH, BARS MAY BE LAPPED Ld IN THE MIDDLE THIRD OF THE BEAM SPAN UNLESS NOTED OTHERWI SE. CONTINUOUS TOP BARS SHALL NOT BE LAPPED IN THE SPAN ADJACENT TO A CANTILEVER, UNLESS NOTED OTHERWISE. WHERE BOTTOM BARS ARE SHOWN AS CONTINUOUS AND RUN IN EXCESS OF 60 FEET, A LAP SPLICE MAY BE USED EQUAL TO Lsb AND SHALL BE OUTSIDE THE MIDDLE THIRD OF THE BEAM SPAN. SIDE BAR SPLICES MAY BE MADE WHERE CONVENIENT.
5. LOCATE ALL CONSTRUCTION JOINTS WITHIN THE MIDDLE THIRD OF SPAN. JOINTS SHALL BE OFFSET AT A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS. SUBMIT LOCATION OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE BEFORE FORMING.
6. ALL BARS IN SAME LAYER UNLESS NOTED OTHERWISE.

3 TYPICAL CONCRETE BEAM NOTES

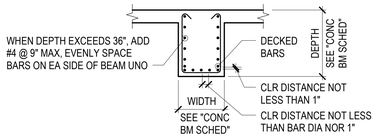
CONCRETE BEAM SCHEDULE

MARK	SIZE (WIDTHxDEPTH)	CAMBER	BOTTOM BARS	TOP BARS LEFT	TOP BARS CONTINUOUS	TOP BARS RIGHT	STIRRUPS LEFT	STIRRUPS CONTINUOUS	STIRRUPS RIGHT	REMARKS	S_ConcBmSched_MIN
B1	12"x18"		(2) #7	(2) #7	(2) #7	(2) #7		#4 @ 6" [2C]			
B2	24"x24"		(3) #8		(7) #8			#5 @ 9" [2C]			
B3	24"x24"		(4) #8	(4) #8	(3) #8	(4) #8		#5 @ 9" [2C]			
B4	24"x24"		(3) #5		(3) #7			#4 @ 10" [2C]			
B5	24"x24"		(3) #8		(9) #9			#5 @ 9" [2C]			
B6	24"x24"		(3) #8	(5) #9	(4) #9	(5) #9		#5 @ 9" [2C]			
B7	24"x24"		(3) #8		(10) #9			#5 @ 9" [3C]			
B8	24"x24"		(8) #10	(5) #10	(5) #10	(5) #10		#6 @ 5" [3C]			
B9	30"x24"		(4) #7		(7) #9			#5 @ 9" [3C]		ADD (2) #4 SIDE BAR EA SIDE	
B10	30"x24"	1	(4) #7		(7) #9			#5 @ 9" [3C]		ADD (3) #4 SIDE BAR EA SIDE	
B12	30"x24" MIN		(4) #7		(9) #10			#5 @ 9" [3C]		ADD (2) #4 SIDE BAR EA SIDE	MIN
B13	30"x25"		(5) #7		(10) #10			#5 @ 9" [3C]		ADD (2) #4 SIDE BAR EA SIDE	
B14	24"x24"		(5) #9		(4) #7			#5 @ 9" [3C]			
B16	30"x24"		(4) #7		(6) #8			#5 @ 6" [3C]		ADD (3) #5 SIDE BAR EA SIDE	
B17	30"x24"		(4) #7		(9) #8			#4 @ 9" [4C]		ADD (3) #4 SIDE BAR EA SIDE	
B21	30"x24" MIN	1	(4) #7		(10) #10			#6 @ 6" [3C]		ADD (2) #4 SIDE BAR EA SIDE	MIN
B22	30"x25"		(6) #8		(10) #10			#6 @ 6" [3C]		ADD (2) #4 SIDE BAR EA SIDE	
B23	30"x25"		(4) #7		(8) #9			#5 @ 8" [3C]		ADD (2) #4 SIDE BAR EA SIDE	
B25	24"x24"		(6) #9		(4) #7			#5 @ 9" [3C]			
B26	30"x30"	1	(3) #7		(4) #7			#5 @ 9" [3C]			
B27	30"x30"		(3) #7		(4) #7			#6 @ 6" [2C]			
B28	18"x24"		(3) #7		(3) #7			#5 @ 5" [2C]			
B29	30"x24"		(4) #7		(6) #8			#5 @ 9" [2C]			
B30	30"x24"		(4) #7		(9) #8			#5 @ 10" [2C]			
B33	24"x32"		(4) #8		(3) #7			#5 @ 14" [2C]			
B34	24"x24"		(4) #8		(4) #8			#5 @ 10" [2C]			
B35	24"x24"		(5) #8		(5) #8			#5 @ 10" [2C]			
B37	18"x32"		(3) #7		(3) #7			#5 @ 5" [2C]			
B38	24"x32"		(4) #7		(4) #8			#5 @ 14" [2C]			
B39	24"x32"		(4) #8		(4) #8			#5 @ 9" [3C]			
B40	24"x32"		(6) #9		(4) #8			#5 @ 9" [3C]			
B41	34"x30"		(6) #8		(4) #7			#5 @ 9" [3C]			
B42	32"x32"		(4) #9		(4) #9			#4 @ 14" [4C]			
B43	18"x32"		(4) #9		(4) #9			#5 @ 14" [4C]			
B44	24"x48"		(3) #7		(3) #7			#4 @ 14" [3C]			
B45	24"x48"		(3) #8		(3) #8			#4 @ 14" [3C]			
B46	24"x32"		(3) #8		(3) #8			#4 @ 14" [3C]			
B47	24"x32"		(3) #9		(5) #9			#4 @ 14" [3C]			
B48	24"x50"		(3) #8		(3) #9			#4 @ 14" [3C]			
B49	24"x32"		(3) #9		(3) #9			#4 @ 14" [3C]			
B50	12"x39"		(3) #7		(3) #7			#4 @ 14" [3C]			
B51	18"x32"		(4) #8		(4) #8			#4 @ 10 [4C]			
B52	32"x72"		(14) #11	(3) #10	(4) #10	(3) #10		#5 @ 6" [4C]			
B53	18"x32"		(4) #8	(2) #8	(3) #8	(2) #8		#5 @ 12" [2C]			
B54	24"x32"		(3) #7		(3) #7			#4 @ 14" [3C]			
B56	18"x37"		(3) #9		(3) #9			#4 @ 14" [3D]			
B57	24"x32"		(3) #9		(3) #9			#4 @ 14" [3C]			
B58	30"x36"		(4) #9	(3) #7	(4) #9	(3) #7	(13) #5 @ 6" [4C]	#5@ 14" [4C]	(13) #5 @ 6" [3C]		
B59	24"x62"		(4) #9		(4) #9			#4 @ 12" [4C]			
B61	24"x39"		(6) #10		(6) #8			#4 @ 14" [4C]			
B62	24"x74"		(3) #8		(3) #8			#4 @ 14" [3C]			
B63	24"x72"		(5) #11		(5) #11			#4 @ 14" [3C]			
B64	32"x72"		(8) #11		(8) #11			#5 @ 8" [4C]			
B65	24"x60"		(6) #11		(6) #9			#5 @ 14" [4C]			
B66	24"x74"		(5) #11		(5) #11			#4 @ 14" [3C]			
B67	24"x26"		(3) #8		(3) #8			#4 @ 14" [3C]			
B68	24"x62"		(4) #9		(4) #9			#4 @ 14" [4C]			
B69	24"x48"		(4) #11		(4) #11			#4 @ 14" [4C]			
B70	24"x24"		(4) #9		(4) #7			#4 @ 9" [3C]			
B71	40"x42"		(11) #18	(6) #11	(6) #11	(6) #11		#6 @ 4" [4C]			
B72	60"x42"		(14) #11		(8) #9			#5 @ 6" [7C]			

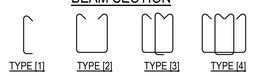
NOTES:

1. SEE "TYPICAL CONCRETE BEAM" DETAIL.
2. [ ] DENOTES TYPE OF REINFORCING CONFIGURATION. SEE "TYPICAL CONCRETE BEAM SECTION AND STIRRUPS" DETAIL FOR STIRRUP TYPE.

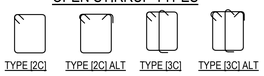
13 CONCRETE BEAM SCHEDULE



BEAM SECTION



OPEN STIRRUP TYPES



CLOSED STIRRUP TYPES

NOTES:

1. [ ] DENOTES TYPE OF STIRRUP REINFORCING CONFIGURATION. SEE "CONCRETE BEAM SCHEDULE."

19 TYP CONC BEAM SECTION AND STIRRUPS

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project manager
drawn by
checked by
job no. 20052
date 7/15/2022

revisions:

no.	date	by
1	7/15/2022	PROGRESS PRINT

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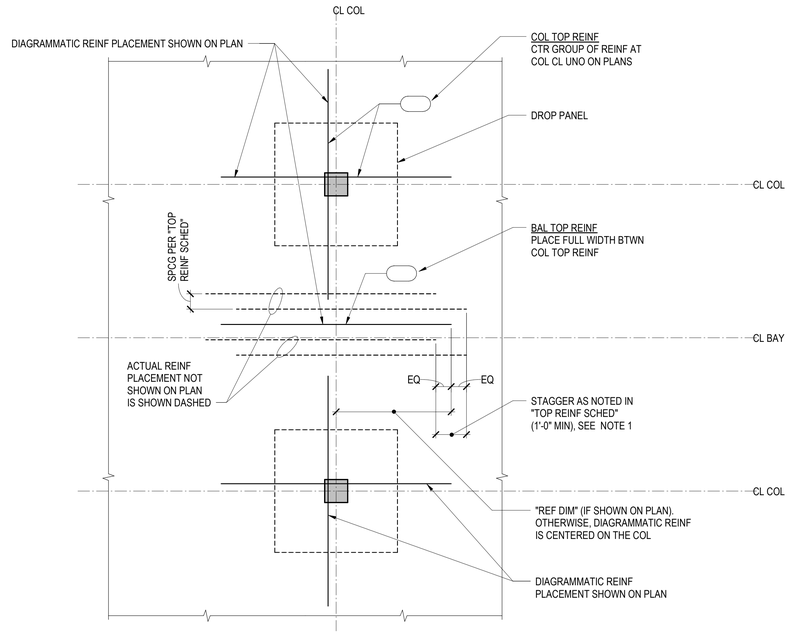
TYPICAL CONCRETE BEAM DETAILS AND SCHEDULE

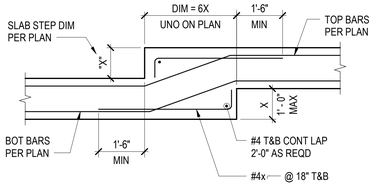
S4.03



NOTES:

1. STAGGERED CONDITION APPLIES TO ALL TOP BARS EXCEPT HOOKED BARS AT SLAB EDGES.

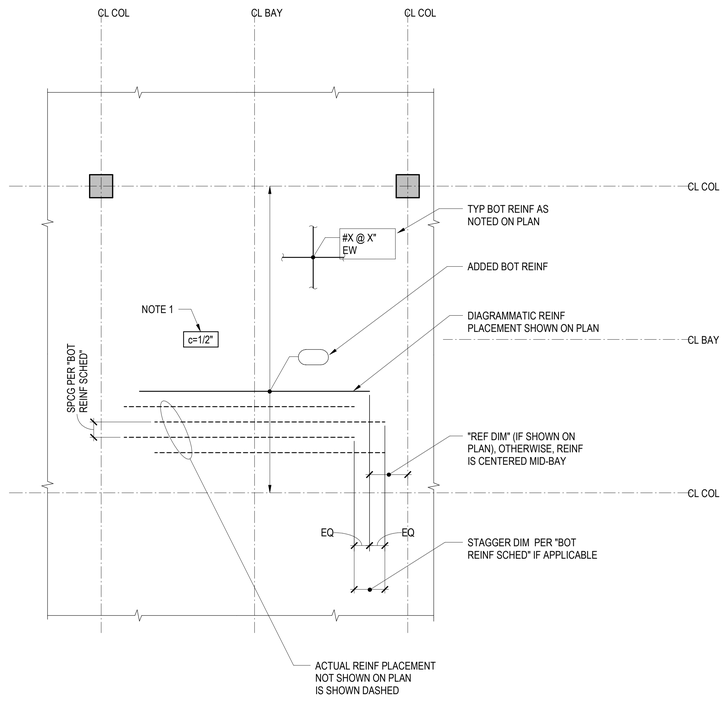
9 TYPICAL MILD SLAB TOP REINFORCING PLACEMENT



NOTES:

1. THIS DETAIL ONLY APPLIES TO MILD REINFORCED SLABS.

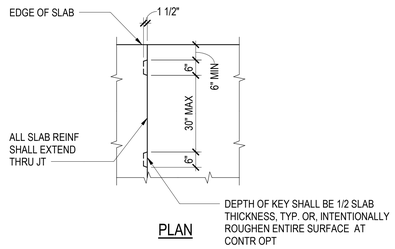
13 TYPICAL MILD SLAB STEP REINFORCING



NOTES:

1. WHEN NO EXTENT LINES EXIST AT MILD SLAB CAMBER, A SINGLE HIGH POINT AT MID-BAY IS INDICATED. THE SURROUNDING SLAB SLOPES AWAY TOWARD THE ADJACENT COLUMNS OR WALLS.

19 TYPICAL MILD SLAB BOTTOM REINFORCING PLACEMENT

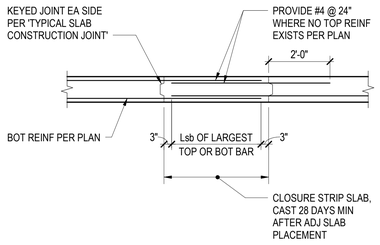


PLAN

NOTES:

1. LOCATE ALL CONSTRUCTION JOINTS WITHIN THE MIDDLE THIRD OF THE SPAN. SUBMIT LOCATIONS OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE PRIOR TO FORMING.
2. PROVIDE #4x5'-0" @ 24" CENTERED ACROSS CONSTRUCTION JOINT AT LOCATIONS WHERE TOP SLAB REINFORCING IS NOT SPECIFIED PER PLAN.

17 TYPICAL SLAB CONSTRUCTION JOINT



NOTES:

1. SHORE SLAB UNTIL CLOSURE STRIP REACHES 28 DAY DESIGN STRENGTH.
2. REINFORCING IN OPPOSITE DIRECTION IS PER PLANS.

18 TYPICAL MILD SLAB CLOSURE STRIP

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principal architect
project manager
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checked by
job no. 20052
date 7/15/2022

revisions:

no.	date	by

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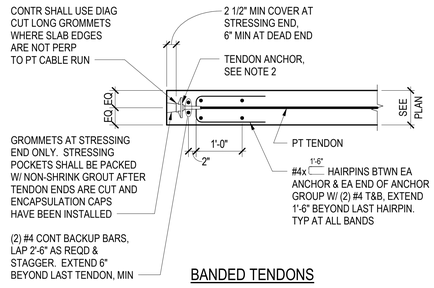
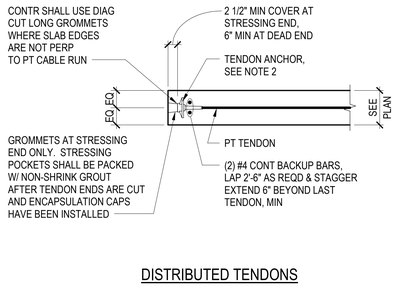
TYPICAL MILD SLAB DETAILS

S4.04



BANDED TENDONS

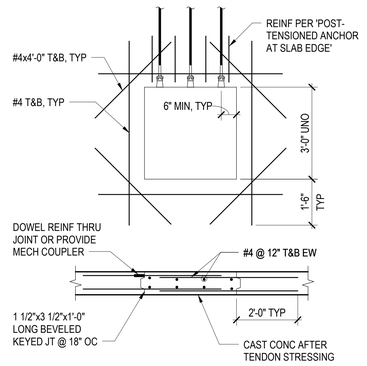


DISTRIBUTED TENDONS

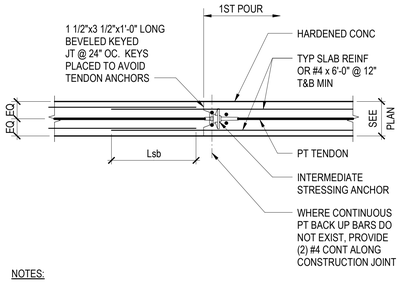
NOTES:

1. BANDED TENDON DETAIL APPLIES FOR GROUPS OF FOUR OR MORE TENDONS WHERE ANCHOR SPACING IS LESS THAN 12 INCHES.
2. PLACE ANCHORS VERTICALLY IN SLABS GREATER THAN OR EQUAL TO 7 INCHES THICK. PLACE ANCHORS HORIZONTALLY FOR SLABS LESS THAN 7 INCHES THICK.

1 TYPICAL POST-TENSIONED ANCHOR AT SLAB EDGE



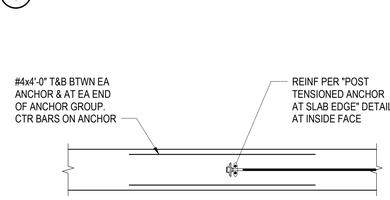
6 TYPICAL STRESSING BLOCKOUT



NOTES:

1. CONTRACTOR TO SUBMIT LOCATIONS OF CONSTRUCTION JOINTS TO ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.
2. SEE "TYPICAL POST-TENSION ANCHOR AT SLAB EDGE" DETAIL FOR ADDITIONAL REINFORCING AT ANCHOR.

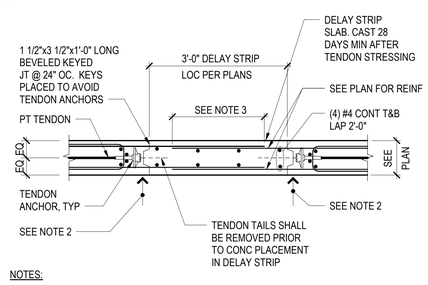
7 TYP POST-TENSIONED SLAB CONST JOINT



NOTES:

1. ACCEPTABLE FOR BAR PROVIDED ON PLAN TO SATISFY NOTED REINFORCEMENT REQUIREMENTS.

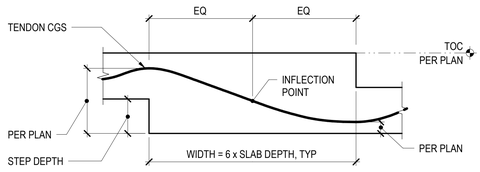
11 TYP PT ANCHOR AWAY FROM SLAB EDGE



NOTES:

1. SEE "TYPICAL POST-TENSION ANCHOR AT SLAB EDGE" DETAIL FOR ADDITIONAL REINFORCING AT ANCHOR.
2. SHORE SLAB UNTIL DELAY STRIP HAS REACHED 28 DAY DESIGN STRENGTH.
3. LAP SPLICE Lsb UNLESS NOTED OTHERWISE. WHERE LAP CANNOT BE ACHIEVED, PROVIDE BAR COUPLERS THAT REMAIN UNFASTENED UNTIL JUST PRIOR TO CASTING OF CLOSURE STRIP.

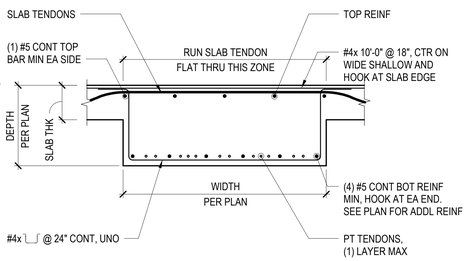
12 TYPICAL POST-TENSIONED DELAY STRIP



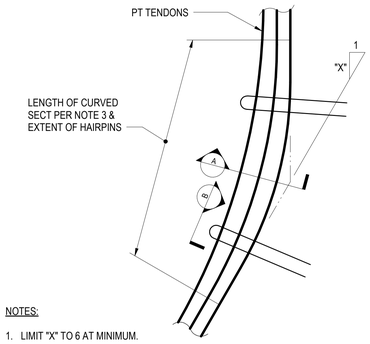
NOTES:

1. SEE "TYPICAL MILD SLAB STEP REINFORCING" FOR REINFORCEMENT AT STEP, NOT SHOWN HERE.

17 TYPICAL PT SLAB STEP



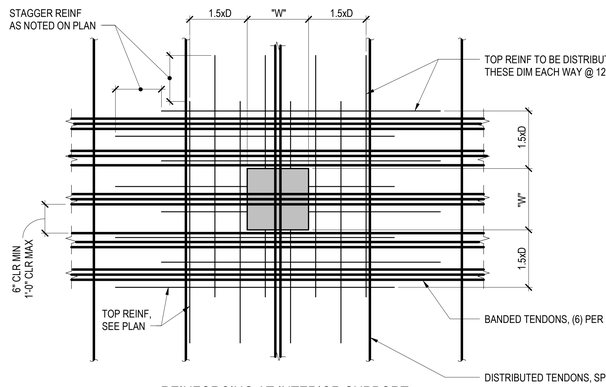
18 TYPICAL PT WIDE SHALLOW



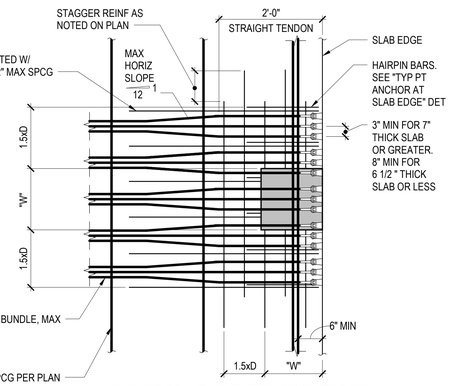
NOTES:

1. LIMIT "X" TO 6 AT MINIMUM.
2. WHERE "X" IS 12 OR GREATER, NO HAIRPINS ARE REQUIRED.
3. MINIMUM LENGTH OF CURVED SECTION IS EQUAL TO 20/X (IN FEET).

19 TYPICAL SKEWED TENDON REINFORCEMENT



REINFORCING AT INTERIOR SUPPORT

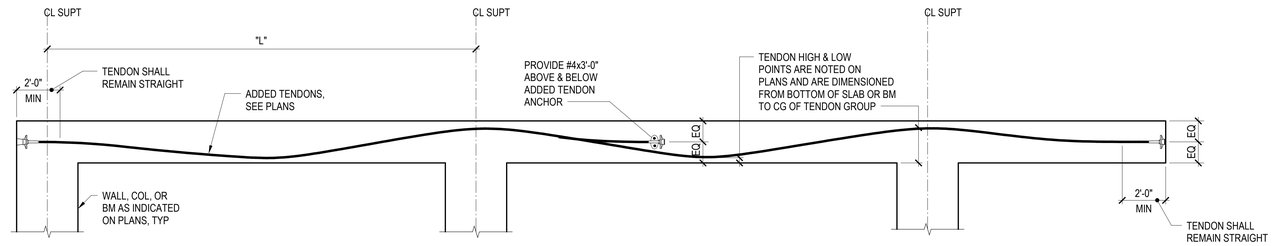


REINFORCING AT EXTERIOR SUPPORT

NOTES:

1. A MINIMUM OF TWO TENDONS ARE TO PASS DIRECTLY OVER COLUMN WITHIN COLUMN VERTICAL REINFORCING EACH WAY.
2. LEGEND:
   W =COLUMN WIDTH
   D = DEPTH OF SLAB OR DEPTH OF SLAB PLUS DROP PANEL.

3 TYPICAL POST-TENSIONED REINFORCING AT SUPPORT FOR TWO-WAY SLABS

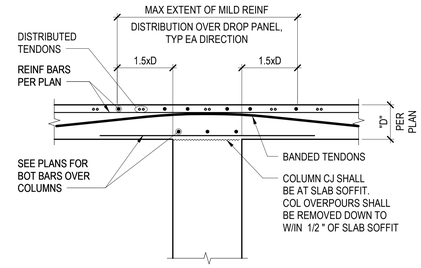


EXTERIOR SUPPORT | EXTERIOR SPAN | INTERIOR SUPPORT | INTERIOR SPAN | CANTILEVERED SLAB

NOTES:

1. SEE PLANS FOR TENDON LAYOUT.
2. TENDON LOW POINTS ARE LOCATED MIDWAY BETWEEN HIGH POINTS UNLESS NOTED OTHERWISE.
3. DRAPE TENDON IN PARABOLIC PROFILE BETWEEN LOW AND HIGH POINTS.
4. DROP PANELS, WHERE THEY OCCUR, ARE NOT SHOWN FOR CLARITY.

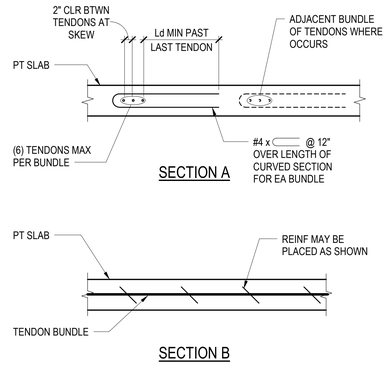
8 TYPICAL POST-TENSIONED TENDON PROFILE



NOTES:

1. SLAB SHEAR REINFORCING NOT SHOWN.

15 TYPICAL COLUMN / SLAB INTERSECTION



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principal architect
project manager
drawn by
checked by
job no. 20052
date 7/15/2022

revisions:

no.	date	by

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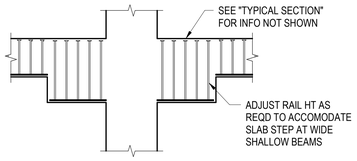
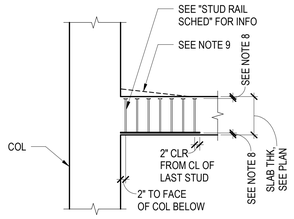
TYPICAL POST-TENSIONED SLAB DETAILS

S4.05

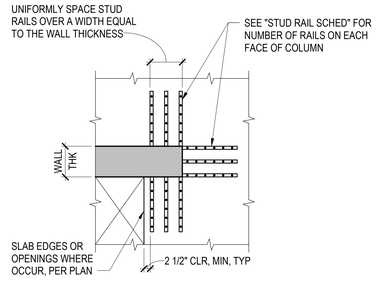


SECTION AT WIDE SHALLOW BEAMS



SECTION

2 TYPICAL STUD RAIL SECTION



7 TYPICAL SLAB SHEAR REINFORCEMENT AT WALLS

NOTES:

1. SEE PLANS FOR LOCATIONS WHERE SLAB SHEAR REINFORCING IS REQUIRED.
2. SLAB SHEAR REINFORCEMENT SHALL CONFORM TO ASTM A1044.
3. ALL STUD RAILS SHALL BE CLEARLY MARKED WITH BRIGHT PAINT TO INDICATE STUD RAIL TYPE.
4. COORDINATE PLACEMENT WITH POST-TENSIONED TENDONS AND ANCHORAGES. PROVIDE POSITIVE MEANS TO KEEP REINFORCEMENT IN PLACE DURING CONCRETE PLACEMENT.
5. NUMBER OF RAILS PER COLUMN SIDE IS PER "STUD RAIL SCHEDULE."
6. PLACE OUTER STUD RAILS FLUSH WITH CORNERS OF COLUMN UNLESS NOTED OTHERWISE. EQUALLY SPACE RAILS ON EACH FACE OF COLUMN.
7. SEE PLANS FOR REINFORCING THROUGH COLUMNS, TYPICAL.
8. STUD RAIL CLEAR COVER MATCHES ADJACENT SLAB REINFORCING, TOP AND BOTTOM, REFER TO "GENERAL NOTES."
9. WHERE TOP OF SLAB SLOPES (REFER TO PLANS), DETERMINE STUD HEIGHT BASED ON MINIMUM SLAB THICKNESS OVER THE LENGTH OF THE RAIL. ALL STUDS ON A SINGLE RAIL SHALL HAVE THE SAME HEIGHT.
10. WHERE SLAB EDGE IS NOT FLUSH WITH COLUMN FACE, SEE "TYPICAL TRIMMED STUD RAIL" DETAIL.
11. AT BALCONIES OR SLAB DEPRESSIONS, SEE "TYPICAL SLAB STEP STUD RAIL LAYOUT" DETAIL.

8 TYPICAL STUD RAIL NOTES

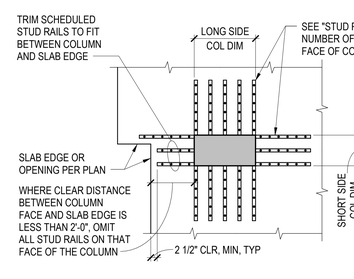
STUD RAIL SCHEDULE

MARK	COLUMN LONG SIDE NUMBER OF RAILS	COLUMN SHORT SIDE NUMBER OF RAILS	STUD DIAMETER (IN)	STUD SPACING (IN)	NUMBER OF STUDS PER RAIL	POST-TENSIONED SLAB INTEGRITY BARS PER SIDE		REMARKS
						LONG DIMENSION	SHORT DIMENSION	
R1	3	3	1/2	3	11	(5) #5x7'-6"	(5) #5x7'-6"	
R2	4	3	1/2	3	11	(4) #5x7'-6"	(3) #5x7'-6"	
R3	6	2	1/2	3	11	(7) #5x10'-0"	(3) #5x10'-0"	
R4	3	3	1/2	3	14	(4) #5x7'-6"	(4) #5x7'-6"	
R5	3	3	1/2	3	17	(4) #5x7'-6"	(4) #5x7'-6"	
R6	4	3	1/2	3	25	(4) #5x7'-6"	(3) #5x7'-6"	
R7	3	3	1/2	3	14	(5) #5x7'-6"	(5) #5x7'-6"	
R8	4	3	1/2	3	15	(4) #5x7'-6"	(3) #5x7'-6"	
R9	4	3	1/2	3	19	(7) #5x10'-0"	(6) #5x10'-0"	
R10	5	3	1/2	3	19	(7) #5x10'-0"	(6) #5x10'-0"	
R11	4	4	1/2	3	25	(7)#5x10'-0"	(6)#5x10'-0"	
R12	6	2	1/2	3	19	(7) #5x10'-0"	(5) #5x10'-0"	
R13	3	3	1/2	3	22	(5) #5x7'-6"	(5) #5x7'-6"	
R14	4	4	1/2	3	17	(7) #5x10'-0"	(7) #5x10'-0"	
R15	3	3	1/2	3	17	(7) #5x10'-0"	(7) #5x10'-0"	
R16	4	4	1/2	4	11			
R17	11	4	1/2	4	11			
R18	5	3	1/2	3	19			
R19	6	4	1/2	3	17	(8) #5x10'-0"	(8) #5x10'-0"	

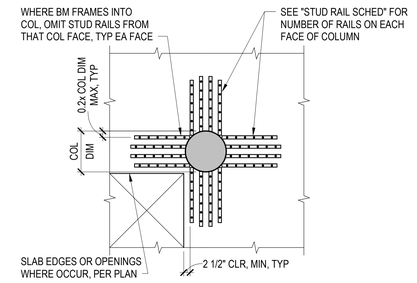
NOTES:

1. SEE TYPICAL SLAB SHEAR REINFORCEMENT DETAILS.

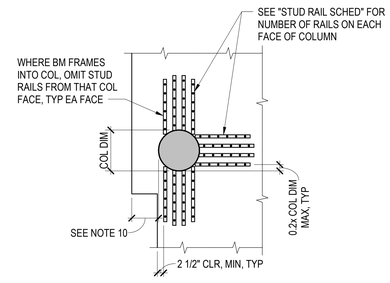
9 STUD RAIL SCHEDULE



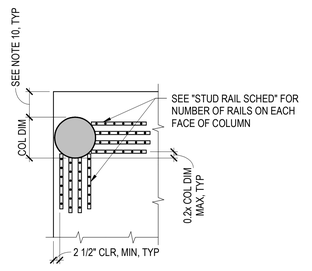
12 TYPICAL TRIMMED STUD RAIL



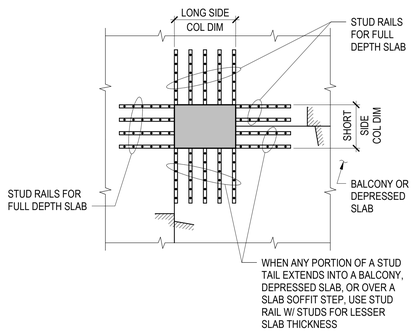
13 TYPICAL SLAB SHEAR REINFORCEMENT AT ROUND INTERIOR COLUMN



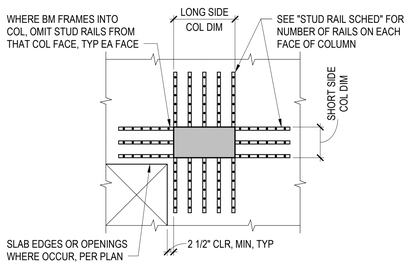
14 TYPICAL SLAB SHEAR REINFORCEMENT AT ROUND EDGE COLUMN



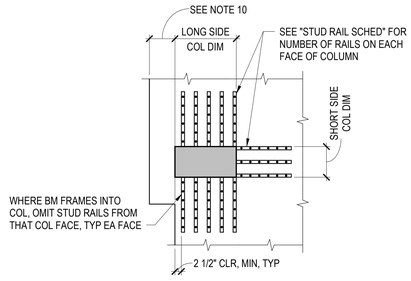
15 TYPICAL SLAB SHEAR REINFORCEMENT AT ROUND CORNER COLUMN



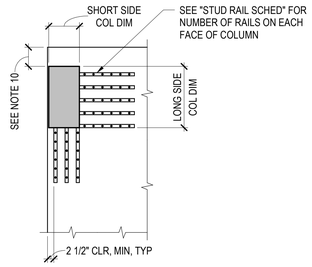
17 TYPICAL SLAB STEP STUD RAIL LAYOUT



18 TYPICAL SLAB SHEAR REINFORCEMENT AT RECTANGULAR INTERIOR COLUMN



19 TYPICAL SLAB SHEAR REINFORCEMENT RECTANGULAR EDGE COLUMN



20 TYPICAL SLAB SHEAR REINFORCEMENT AT RECTANGULAR CORNER COLUMN

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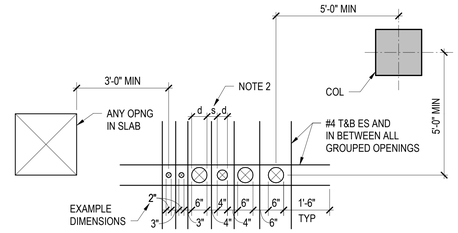
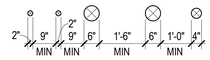
TYPICAL STUD RAIL DETAILS AND SCHEDULE

S4.06



GROUPED OPENINGS IN SLAB

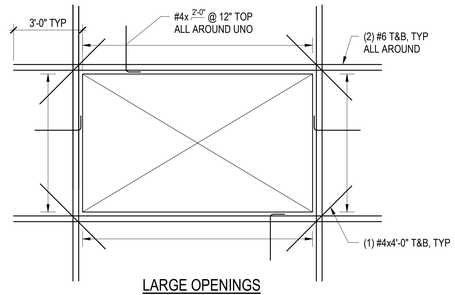


NON-GROUPED OPENINGS IN SLAB

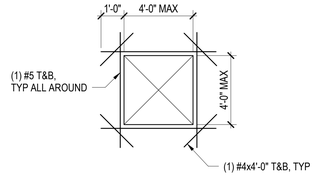
NOTES:

1. ADDED REINFORCING SHOWN APPLIES TO GROUPS OF OPENINGS 12" OR SMALLER. GROUPED OPENINGS ARE DEFINED BY MULTIPLE OPENINGS THAT ARE LOCATED WITHIN THREE TIMES THE DIAMETER OF SMALLER ADJACENT OPENING CLEAR APART, OR A MINIMUM OF 9-INCHES.
2. "s" MUST BE GREATER THAN OR EQUAL TO THE SMALLEST "d" (OPENING) DIMENSION OF ADJACENT SLEEVES. "s" IS EQUAL TO OR GREATER THAN 3-INCHES IN ALL CASES.
3. SEE THE "TYPICAL POST-TENSION CONCRETE SLAB DETAILS" FOR OTHER TENDON PLACEMENT AND REINFORCING REQUIREMENTS.
4. OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR REVIEW AT THE SAME TIME AS SLAB SHOP DRAWINGS. REINFORCING OTHER THAN THAT SHOWN MAY BE REQUIRED.
5. WHERE A TENDON MUST PASS BETWEEN OPENINGS, "s," AT THAT LOCATION, SHALL BE INCREASED TO 6-INCH MINIMUM.
6. WHERE THESE CONDITIONS CANNOT BE MET, SUBMIT TO STRUCTURAL ENGINEER FOR REVIEW.

4 TYPICAL ADDED REINFORCEMENT AT GROUPED OPENINGS IN SLAB



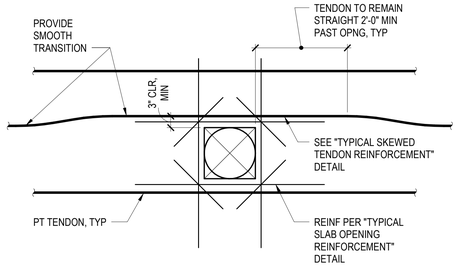
LARGE OPENINGS



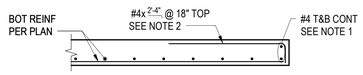
SMALL OPENINGS

NOTES:

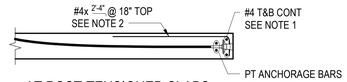
1. LARGE OPENING DETAIL APPLIES AT ALL SLAB OPENINGS WITH ANY SIDE GREATER THAN 4'-0" IN WIDTH.
2. WHERE OPENING IS FLUSH WITH WALL OR BEAM BELOW, REINFORCING MAY BE EXCLUDED ON THAT SIDE OF THE OPENING.
3. HOOK INTERRUPTED REINFORCING.



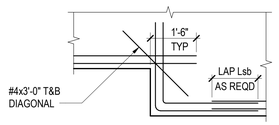
8 TYPICAL TENDON CURVATURE AT OPENING



AT MILD REINFORCED SLABS



AT POST-TENSIONED SLABS

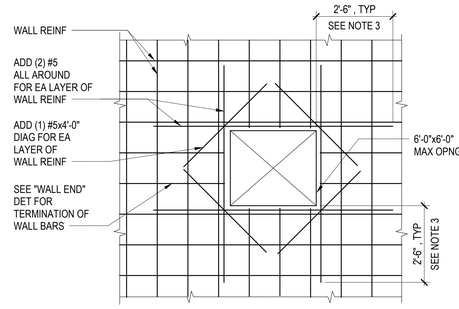


CONTINUOUS BARS AT CORNERS

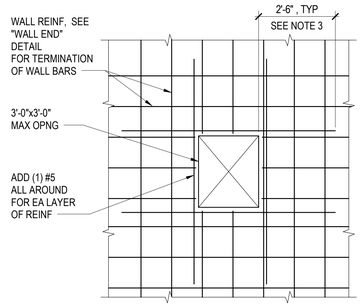
NOTES:

1. PROVIDE #4 TOP AND BOTTOM CONTINUOUS BARS AT ALL SLAB EDGES.
2. FOR LOCATIONS WITH HOOKED TOP BAR PERPENDICULAR TO SLAB EDGE PROVIDED ON PLANS, THIS BAR MAY BE OMITTED.

9 TYPICAL SLAB OPENING REINFORCEMENT



LARGE OPENING

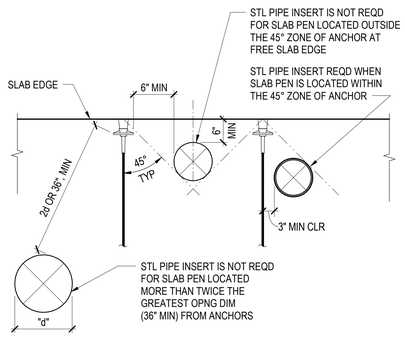


SMALL OPENING

NOTES:

1. OMIT ADDED REINFORCEMENT NOTED ABOVE WHEN SPECIAL REINFORCEMENT, INDICATED ON PLANS OR DETAILS, EXCEEDS THIS REINFORCEMENT.
2. CONTRACTOR SHALL VERIFY ALL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS WITH THE STRUCTURAL ENGINEER BEFORE PLACEMENT.
3. WHEN EDGE OF CONCRETE CLOSE TO OPENING WILL NOT ALLOW THIS LENGTH, CONSULT STRUCTURAL ENGINEER BEFORE CONSTRUCTION.

13 TYPICAL EDGE OF SLAB



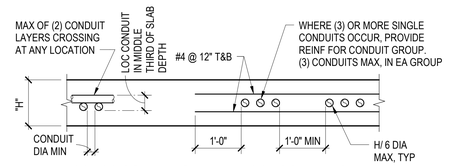
NOTES:

1. SEE "TYPICAL ADDED REINFORCEMENT AT GROUPED OPENINGS IN SLAB" FOR ADDITIONAL SLAB PENETRATION CRITERIA, REINFORCEMENT, AND SUBMITTAL REQUIREMENTS.

14 TYPICAL CONCRETE WALL OPENING REINFORCEMENT

NOTES:

1. DO NOT PLACE CONDUIT WITHIN 4'-0" OF COLUMN FACE OR THROUGH SLAB SHEAR REINFORCING STUD RAILS.
2. KEEP CONDUIT AT LEAST 2'-0" AWAY FROM POST-TENSIONED TENDON ANCHORAGE .
3. MAINTAIN A MINIMUM HORIZONTAL CLEARANCE FROM PARALLEL PT TENDONS OF 1'-0" AND A MINIMUM VERTICAL CLEARANCE OF ONE INCH FROM PT TENDONS BEING CROSSED.
4. CONGESTED AREAS OF CONDUIT THAT DO NOT MEET THIS CRITERIA SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. ADDITIONAL REINFORCEMENT MAY BE NECESSARY.
5. DO NOT TIE CONDUIT DIRECTLY TO REINFORCEMENT PARALLEL TO CONDUIT. PROVIDE MINIMUM db, BUT NOT LESS THAN ONE INCH CLEAR BETWEEN CONDUIT AND REINFORCEMENT.
6. HORIZONTAL OR VERTICAL CONDUIT SHALL NOT BE PLACED IN CONCRETE COLUMNS OR WALLS WITHOUT PRIOR APPROVAL OF STRUCTURAL ENGINEER.



(1) - (2) CONDUITS (3)+ CONDUITS

18 TYP SLAB PENETRATION AT PT ANCHORS

19 TYPICAL CONDUIT PLACEMENT CRITERIA

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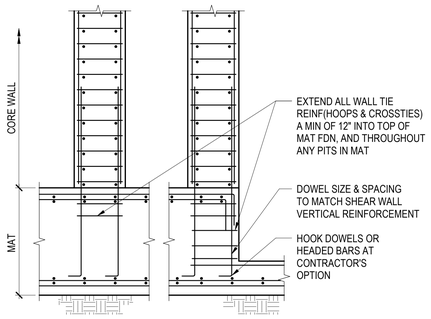
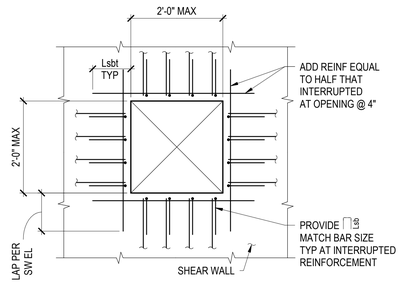
TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE

S4.07



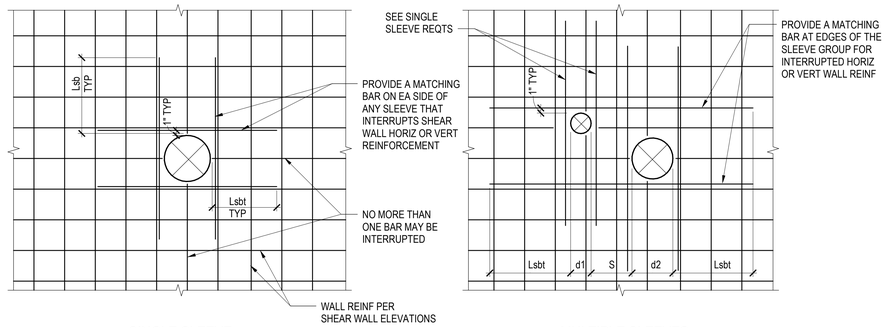
1 TYPICAL SHEAR WALL DOWEL



2 TYPICAL SMALL SHEAR WALL OPENING

NOTES:

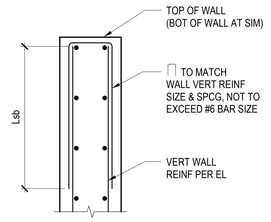
1. MULTIPLE SLEEVES CONFIGURATION IS SHOWN FOR INTERRUPTED FIELD HORIZONTAL REINFORCEMENT. DETAIL IS SIMILAR FOR INTERRUPTED FIELD VERTICAL REINFORCEMENT. NO COUPLING BEAM REINFORCEMENT MAY BE INTERRUPTED BY SLEEVES.
2. DETAIL APPLIES FOR SLEEVES UP TO 12" DIAMETER. ALL SLEEVES TO BE MINIMUM 4'-0" FROM EDGE OF SHEAR WALLS.
3. "S" SHALL BE EQUAL OR GREATER THAN THE GREATER OF d1, d2, OR 3". IF REQUIRED LAYOUT CANNOT CONFORM TO THESE REQUIREMENTS, CONSIDER GROUPING AS ONE COMBINED OPENING AND REFER TO THE "TYPICAL SMALL SHEAR WALL OPENING" DETAIL.
4. ALL SLEEVES NOTES SHOWN ON SHEAR WALL ELEVATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL.



SINGLE SLEEVE

MULTIPLE SLEEVES

3 TYPICAL SLEEVE PLACEMENT REQUIREMENTS IN SHEAR WALLS

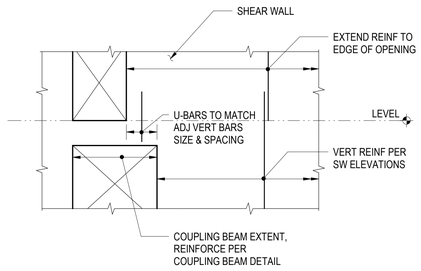


WALL TOP AND BOTTOM

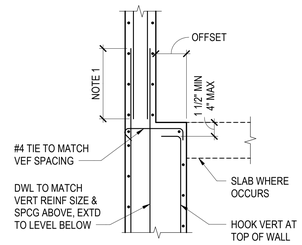
NOTES:

1. SEE "SHEAR WALL SECTIONS" FOR FURTHER REQUIREMENTS.

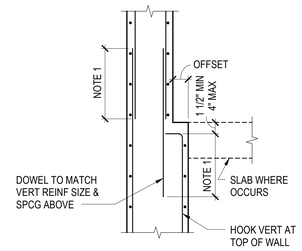
A TYP SHEAR WALL EDGE REINFORCEMENT



8 TYPICAL CONC WALL OPENING TRANSITION



OFFSET > 6"

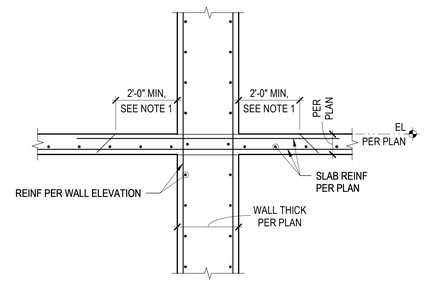


OFFSET ≤ 6"

NOTES:

1. SEE THE SHEAR WALL REINFORCEMENT NOTES FOR LAP LENGTH.
2. SEE PLAN AND WALL ELEVATIONS FOR ADDITIONAL INFORMATION.

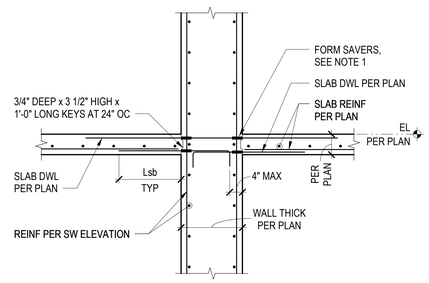
9 TYPICAL REINFORCEMENT AT WALL THICKNESS CHANGE



NOTES:

1. WHERE SHEAR WALL f'c EXCEEDS SLAB f'c, PROVIDE SLAB CONCRETE WITH f'c TO MATCH AT ALL SHEAR WALL TO SLAB JOINTS. THE REMAINING FLOOR SLAB CONCRETE SHALL BE PLACED WHILE THE HIGHER STRENGTH CONCRETE IS STILL PLASTIC AND THE REGION SHALL BE ADEQUATELY VIBRATED TO PROVIDE PROPER INTEGRATION.

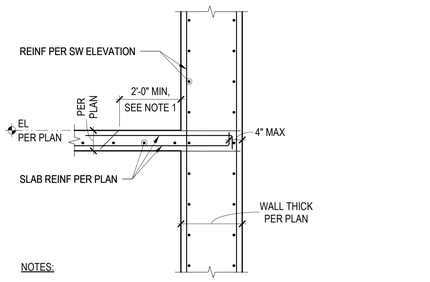
14 TYP SHEAR WALL TO CONTINUOUS SLAB



NOTES:

1. FORM SAVERS TO BE AT EACH END OF STRAIGHT BAR, OR AT CONTRACTOR'S OPTION, ACCEPTABLE TO PROVIDE HOOKS EACH SIDE IN LIEU OF STRAIGHT BAR.

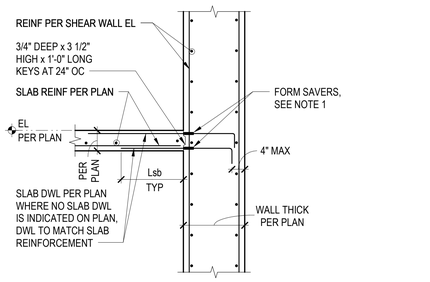
15 TYP JUMP-FORMED SW TO CONT SLAB



NOTES:

1. WHERE SHEAR WALL f'c EXCEEDS SLAB f'c, PROVIDE SLAB CONCRETE WITH f'c TO MATCH AT ALL SHEAR WALL TO SLAB JOINTS. THE REMAINING FLOOR SLAB CONCRETE SHALL BE PLACED WHILE THE HIGHER STRENGTH CONCRETE IS STILL PLASTIC AND THE REGION SHALL BE ADEQUATELY VIBRATED TO PROVIDE PROPER INTEGRATION.

19 TYPICAL SHEAR WALL TO SLAB AT OPENING



NOTES:

1. FORM SAVERS SHALL HAVE Ld STRAIGHT EMBED OR HOOK WITH Ldh EMBED WHERE WALL THICKNESS WILL NOT ALLOW FULL Ld DISTANCE.

20 TYP JUMP-FORMED SW TO SLAB AT OPNG

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checked by
job no. 20052
date 7/15/2022

revisions:

no.	date	by

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TYPICAL SHEAR WALL DETAILS

S4.08

EMBED TABLE 1: UNCONFINED CONDITION / GRADE 60					
BAR SIZE	EMBED LENGTH (IN)				
	NORMAL WEIGHT CONCRETE, f'c (PSI)				
	6,000	7,000	8,000	9,000	≥ 10,000
#5	19	18	17	16	15
#6	23	21	20	19	18
#7	37	34	32	30	28
#8	48	44	41	39	37
#9	61	56	53	50	47
#10	77	71	66	63	59
#11	94	87	82	77	73

1 EMBED TABLE 1

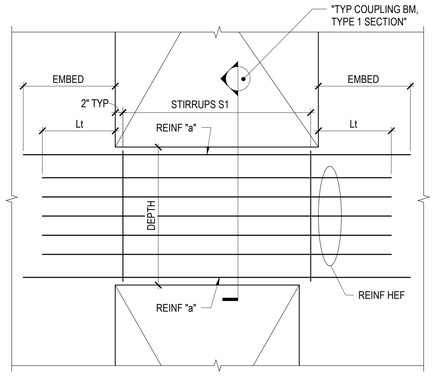
EMBED TABLE 2: CONFINED CONDITION / GRADE 60					
BAR SIZE	EMBED LENGTH (IN)				
	NORMAL WEIGHT CONCRETE, f'c (PSI)				
	6,000	7,000	8,000	9,000	≥ 10,000
#5	19	18	17	16	15
#6	23	21	20	19	18
#7	34	31	29	27	26
#8	42	39	37	35	33
#9	54	50	47	44	42
#10	68	63	59	56	53
#11	84	77	72	68	65

2 EMBED TABLE 2

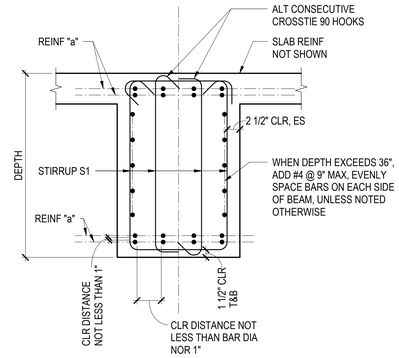
NOTES:

1. CONCRETE STRENGTH OF ALL COUPLING BEAMS SHALL BE THE SAME AS THAT OF ADJACENT WALLS AT THAT LEVEL.
2. IF EMBED LENGTH OF BAR "a" CANNOT BE ACHIEVED DUE TO EDGE OF CORE WALL, PROVIDE 1.25"Ldt LENGTH BEYOND EDGE OF OPENING AND USE A HEADED BAR.
3. EMBED = LENGTH FROM "EMBED TABLE 1" OR "EMBED TABLE 2" AS SPECIFIED IN THE "COUPLING BEAM LONGITUDINAL REINFORCEMENT SCHEDULE."

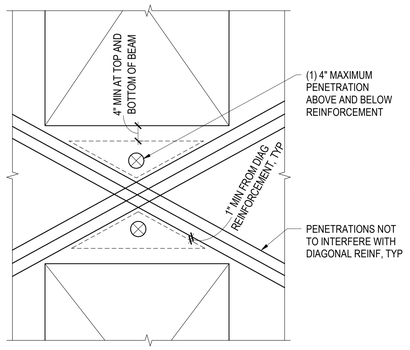
3 GENERAL NOTES FOR COUPLING BEAMS



4 TYPICAL COUPLING BEAM, TYPE 1



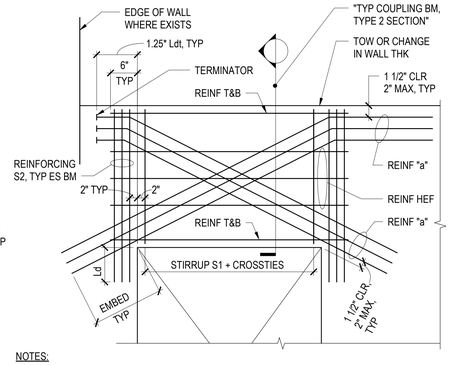
5 TYPICAL COUPLING BEAM,TYPE 1 SECTION



NOTES:

1. REFERENCE TYPICAL COUPLING BEAM TYPE 2 AND 3 FOR REINFORCEMENT LAYOUT

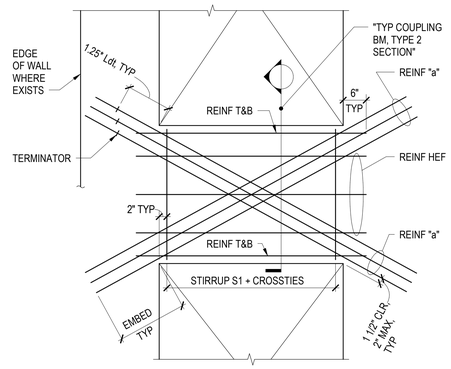
7 TYPICAL COUPLING BEAM PENETRATIONS, TYPE 2 & 3



NOTES:

1. SEE SECTION FOR REINFORCEMENT NOT SHOWN.

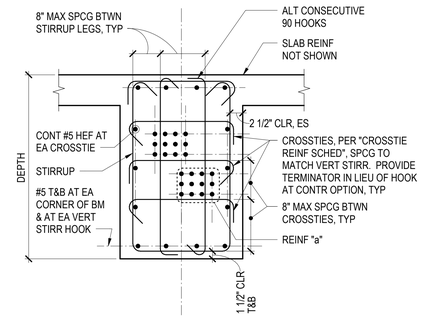
8 TYPICAL COUPLING BEAM, TYPE 3



NOTES:

1. SEE SECTION FOR REINFORCEMENT NOT SHOWN.

9 TYPICAL COUPLING BEAM, TYPE 2



10 TYPICAL COUPLING BEAM, TYPE 2 SECTION

CROSSTIE REINFORCEMENT SCHEDULE		
MAX BEAM DEPTH (IN)	CROSSTIES	
	f'c=6 KSI	f'c=8 KSI
26"	(3) #5	(3) #5
100"	(12) #5	(12) #5
110"	(13) #5	(13) #5
120"	(18) #5	(15) #5
1		

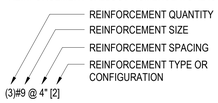
COUPLING BEAM LONGITUDINAL REINFORCEMENT SCHEDULE						
MARK	TYPE	CONFIGURATION DIM (HxW) (IN)	REINF "a"	REINFORCEMENT S2	EMBED TABLE	REMARKS
1	1		(3) #8		TABLE 1	
2	3	4x6	(6) #9 [32]	(6) #5 [4]	TABLE 1	
3	3	4x6	(6) #8 [32]	(5) #5 [3]	TABLE 1	

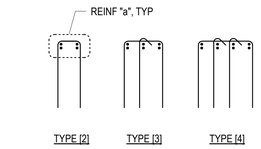
COUPLING BEAM VERTICAL REINFORCEMENT SCHEDULE		
MARK	STIRRUPS S1	REMARKS
A	#5 @ 4" [3]	
B	#5 @ 4" [4]	

REINFORCEMENT "a" CONFIGURATION TYPES

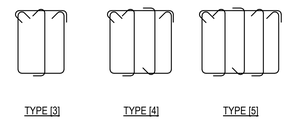
NOTES:

1. DEPTH OF COUPLING BEAMS PER "SHEAR WALL ELEVATIONS." COUPLING BEAM WIDTH MATCHES ADJACENT SHEAR WALLS.
2. ALTERNATE CONSECUTIVE CROSSTIES END FOR END.
3. LONGITUDINAL REINFORCEMENT SHALL BE SPECIAL DUCTILE QUALITY. SEE "GENERAL NOTES" FOR CRITERIA.
4. REINFORCEMENT CALLOUT NOMENCLATURE IS AS FOLLOWS:





REINFORCEMENT S2 CONFIGURATION TYPES



STIRRUP S1 CONFIGURATION TYPES

18 COUPLING BEAM REINFORCEMENT SCHEDULE

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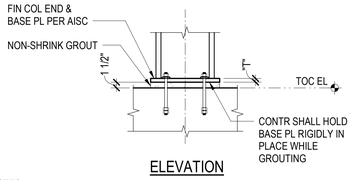
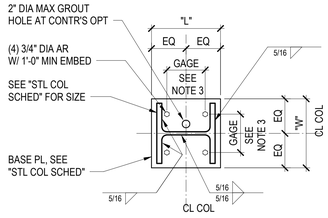
TYPICAL COUPLING BEAM DETAILS AND SCHEDULES

S4.09



ELEVATION



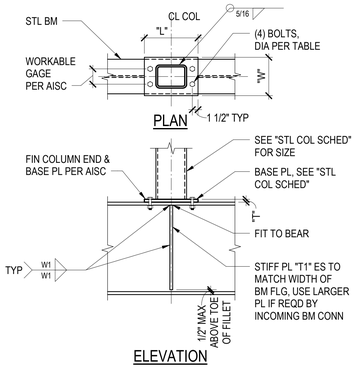
PLAN

NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER "AISC MANUAL - TABLE 14-2", UNLESS NOTED OTHERWISE.
3. ANCHOR ROD GAGE SHALL BE AS FOLLOWS:
   W10: 5 INCHES
   W12: 6 INCHES
   W14: 8 INCHES

   CONTRACTOR TO COORDINATE ANCHOR ROD GAGE WITH CONCRETE REINFORCING.

2 TYPICAL COLUMN BASE PLATE, TYPE 1



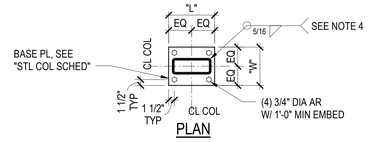
NOTES:

1. DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.

TYPE 6 STIFFENER PLATE TABLE

COLUMN SIZE	BOLT SIZE (IN)	BOLT GRADE	T1 (IN)	W1 (IN)
-	-	-	-	-
-	-	-	-	-
-	-	-	-	-
-	-	-	-	-

4 TYPICAL COLUMN BASE PLATE, TYPE 4

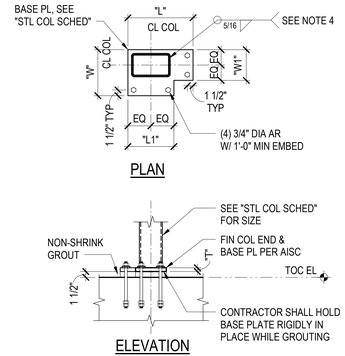


NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER "AISC MANUAL-TABLE 14-2" UNLESS NOTED OTHERWISE.
3. DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
4. WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

TYPE 5 BASE PLATE SCHEDULE

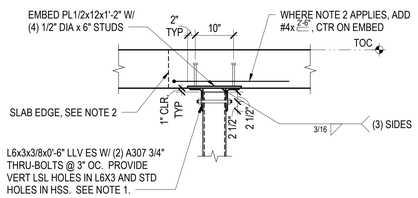
COLUMN SIZE	W1 (IN)	L1 (IN)
-	-	-
-	-	-
-	-	-
-	-	-



NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER "AISC MANUAL-TABLE 14-2" UNLESS NOTED OTHERWISE.
3. DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
4. WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

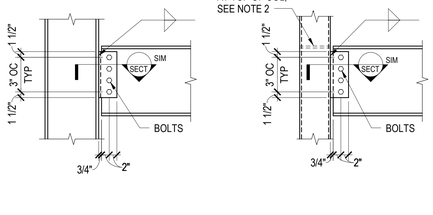
7 TYPICAL BASE PLATE, TYPE 6



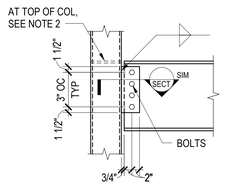
NOTES:

1. WHERE NOTE APPLIES, BOLT TO BE CENTERED IN SLOTTED HOLE IN ANGLE. NUTS TO BE FINGER TIGHT. DAMAGE THREADS OF BOLT TO PREVENT BACK-OFF OF NUT.
2. PROVIDE U-BAR REINFORCEMENT PERPENDICULAR TO SLAB EDGE WHERE EMBED IS LOCATED 6" OR LESS FROM ADJACENT OPENING OR SLAB EDGE.

12 TYPICAL HSS POST TOP SLIP CONNECTION



BEAM TO COLUMN FLANGE

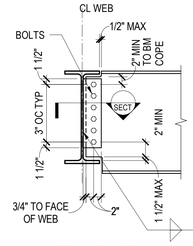


BEAM TO HSS OR PIPE COLUMN

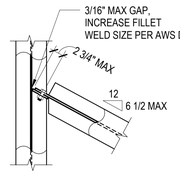
NOTES:

1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
2. AT TOP OF HSS OR PIPE COLUMN, PROVIDE 1/2 INCH CAP PLATE WITH 5/16 INCH FILLET WELD ALL AROUND. IF BEAM IS SHOWN RUNNING OVER TOP OF COLUMN ON PLAN, SEE "TYPICAL BASE PLATE, TYPE 6" DETAIL.

13 TYPICAL BEAM TO BEAM / BEAM TO COLUMN CONNECTION



BEAM TO BEAM



SECTION
(SKEWED BEAMS)

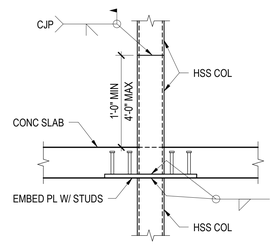
TABLE A

WIDE-FLANGE BEAM DEPTH	NUMBER OF BOLTS REQUIRED	MAXIMUM REACTION (KIPS)
W10	2	27
W12, W14	3	40
W16, W18	4	65
W21	5	91
W24	6	124
W27	7	150
W30 - W44	8	175

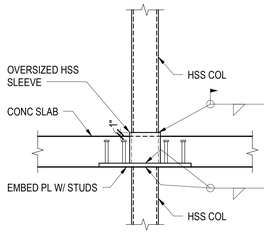
NOTES BELOW APPLY TO ALL TYPICAL CONNECTIONS UNLESS NOTED OTHERWISE:

1. SEE PLANS FOR BEAM SIZE. UNLESS NOTED OTHERWISE, PROVIDE THE NUMBER OF 7/8 INCH DIAMETER GRADE A325 BOLTS SHOWN IN "TABLE A" BASED ON THE BEAM DEPTH.
2. SHEAR TAB PLATES SHALL BE GRADE 50 MATERIAL, AND BE 1/4 INCH THICK WITH 3/16 INCH WELD EACH SIDE FOR (2) BOLTS, 5/16 INCH THICK WITH 1/4 INCH WELD EACH SIDE FOR (3) BOLTS TO (5) BOLTS, AND 3/8 INCH THICK WITH 1/4 INCH WELD EACH SIDE FOR (6) BOLTS OR MORE.
3. BEAMS AND SHEAR TAB PLATES SHALL HAVE STANDARD ROUND HOLES (STD) UNLESS NOTED OTHERWISE. AT CONTRACTOR'S OPTION, HOLES IN SHEAR TAB PLATES MAY BE HORIZONTAL SHORT-SLOTTED HOLES.
4. WHEN CONDITIONS VARY FROM THOSE SHOWN IN THE TYPICAL DETAIL, DESIGN CONNECTIONS ACCORDING TO THE AISC MANUAL OF STEEL CONSTRUCTION.

15 GENERAL NOTES FOR STEEL CONNECTIONS



OPTION 1: STUB COLUMN WITH CJP

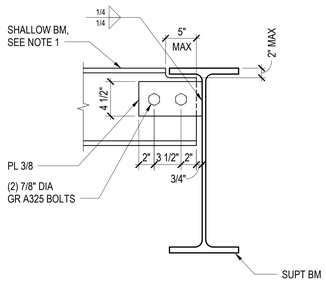


OPTION 2: OVERSIZED TUBE AT EACH FLOOR

NOTES:

1. REBAR, PT, AND STUD RAILS NOT SHOWN FOR CLARITY.

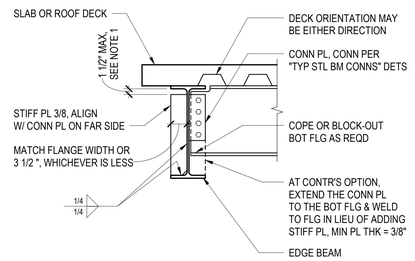
17 TYPICAL HSS COLUMN AT CONCRETE SLAB



NOTES:

1. THIS DETAIL SHALL BE USED ONLY FOR BEAMS UP TO 8 INCHES DEEP AND WEIGHING AT LEAST 8 LB/FT. SEE "TYPICAL BEAM TO BEAM / BEAM TO COLUMN CONNECTION" FOR DEEPER BEAMS.
2. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.

19 TYPICAL SHALLOW BEAM CONNECTION



NOTES:

1. AT LOCATIONS WHERE A CONCRETE SLAB DOES NOT EXIST AT EDGE BEAM, THE STIFFENER PLATE OR CONNECTION PLATE SHALL BE EXTENDED TO FULL DEPTH AND WELDED ON THREE SIDES.
2. THIS DETAIL APPLIES AT ALL EDGE OF SLAB CONDITIONS.

20 TYPICAL STEEL EDGE BEAM STIFFENER

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date 7/15/2022

revisions:

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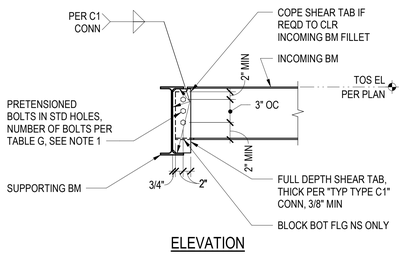
TYPICAL STEEL DETAILS

S4.11



ELEVATION

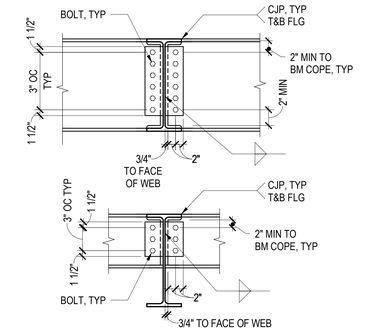
NOTES:

1. SEE "TYPICAL TYPE C1" FOR BOLT SIZE AND TYPE, WELD SIZE, PLATE THICKNESS, AND COPE REQUIREMENTS.
2. THIS CONNECTION SHALL ONLY BE USED WHEN SPECIFICALLY CALLED OUT ON PLAN OR IN A DETAIL.
3. BEAM SIZE IS SHALLOWER OF INCOMING AND SUPPORTING BEAM.

3 TYPICAL STEEL CONNECTION, TYPE C30

3/4" = 1'-0"

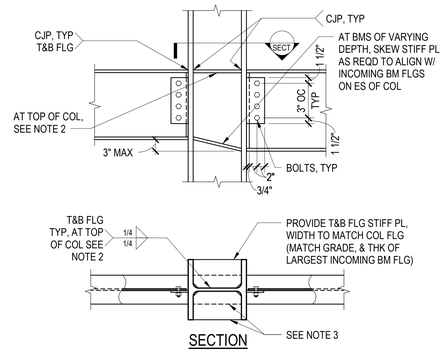
TABLE G	
BM SIZE (NOTE 3)	**MINIMUM NUMBER OF BOLTS**
W10	2
W12, W14	3
W16	4
W18	5
W21	6
W24	7
W27	8
W30	9
W33, W36	10
W40	11
W44	12



NOTES:

1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
2. THIS DETAIL APPLIES ONLY FOR BEAMS OF EQUAL DEPTH EACH SIDE OF SUPPORTING BEAM.

5 TYP BEAM TO BEAM MOMENT CONNECTION

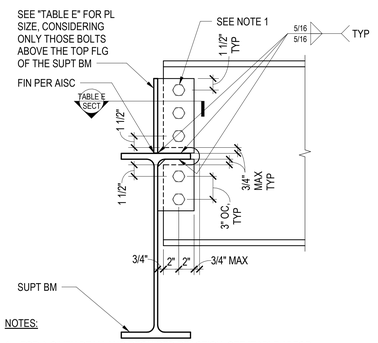


SECTION

NOTES:

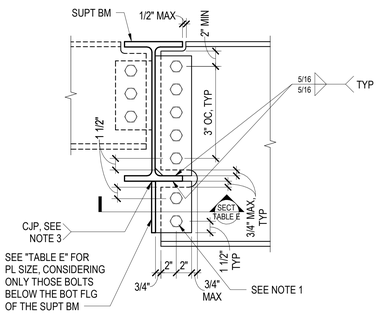
1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
2. AT TOP OF COLUMN, PROVIDE 1/4 INCH PJP WELD IN LIEU OF TOP SIDE FILLET WELD OR, AT CONTRACTOR'S OPTION, A SINGLE CAP PLATE MAY BE USED IN LIEU OF TWO STIFFENER PLATES. USE 5/16 INCH ONE SIDED FILLET WELD FOR CAP PLATE TO EACH COLUMN FLANGE AND EACH SIDE OF COLUMN WEB.
3. AT CONTRACTOR'S OPTION, WIDTH OF STIFFENER PLATES MAY BE REDUCED TO MATCH LARGEST INCOMING BEAM FLANGE IF NO OTHER INCOMING CONNECTIONS OCCUR AT A GIVEN BEAM -TO-COLUMN CONNECTION.

9 TYPICAL BM TO COL FLANGE MOMENT CONN



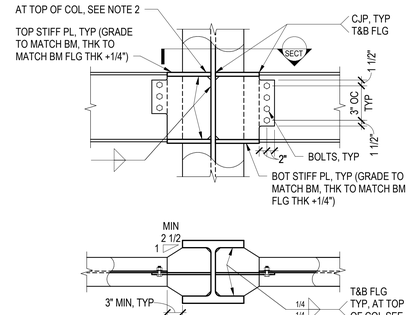
NOTES:

1. FOR A GIVEN BEAM AND REQUIRED REACTION, SEE "TABLE A" FOR BOLT SIZE AND TYPE, TOTAL NUMBER OF BOLTS, AND MAXIMUM COPE LENGTH.
2. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.



3. CONTRACTOR SHALL PERFORM ULTRASONIC TESTING AND INSPECTION OF SUPPORT BEAM BOTTOM FLANGE PER SPECIFICATION FOR WELDED CONNECTIONS.

12 TYPICAL TYPE C21 - BEAM FLANGE INTERSECTING BEAM WEB



SECTION

NOTES:

1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
2. AT TOP OF COLUMN, PROVIDE 1/4 INCH PJP WELD IN LIEU OF TOP SIDE FILLET WELD OR, AT CONTRACTOR'S OPTION, A SINGLE CAP PLATE MAY BE USED IN LIEU OF TWO STIFFENER PLATES. USE 5/16 INCH ONE SIDED FILLET WELD FOR CAP PLATE TO EACH COLUMN FLANGE AND EACH SIDE OF COLUMN WEB.
3. THIS DETAIL APPLIES ONLY FOR BEAMS OF EQUAL DEPTH EACH SIDE OF COLUMN.

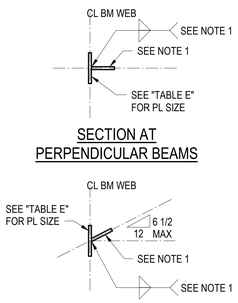
14 TYPICAL BEAM TO COLUMN WEB MOMENT CONNECTION

TABLE E				
7/8" DIA GR A325 BOLTS				
NUMBER OF BOLTS	**PL THICKNESS (IN)**	**PL LENGTH (IN)**	**NOMINAL WEB THICKNESS (IN)**	**STIFF PL THICKNESS (IN)**
2-4	5/8	7	0.31	3/8
5-6	3/4	8	0.44	3/8
7-8	3/4	9	0.50	1/2
9-10	7/8	9	0.61	5/8
11-12	1	10	0.64	5/8
1" DIA GR A490 BOLTS				
NUMBER OF BOLTS	**PL THICKNESS (IN)**	**PL LENGTH (IN)**	**NOMINAL WEB THICKNESS (IN)**	**STIFF PL THICKNESS (IN)**
2-4	5/8	7	0.31	3/8
5-6	3/4	9	0.48	1/2
7-8	7/8	10	0.57	5/8
9-10	1	10	0.65	3/4
11-12	1	10	0.72	1

NOTES:

1. VERTICAL PLATE THICKNESS AND WELD SIZE SHALL BE PER "TABLE A" OR "TABLE B", WHICHEVER IS APPLICABLE.
2. PLATE LENGTH SHALL BE DETERMINED PER "TABLE E" BY THE GREATEST NUMBER OF BOLTS REQUIRED FOR EITHER INCOMING BEAM CONNECTION.

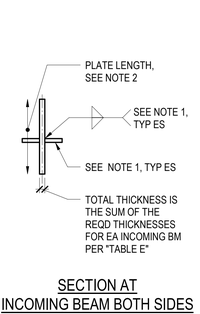
16 TABLE E



SECTION AT
PERPENDICULAR BEAMS

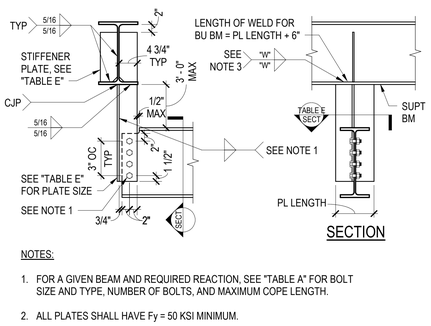
SECTION AT
SKEWED BEAMS

3. WHERE SUPPORT BEAM WEB THICKNESS EXCEEDS NOMINAL WEB THICKNESS IN "TABLE E", STIFFENER PLATES MAY BE OMITTED.



SECTION AT
INCOMING BEAM BOTH SIDES

18 TYP TYPE C23 - BEAM TO DEPRESSED BM CONN

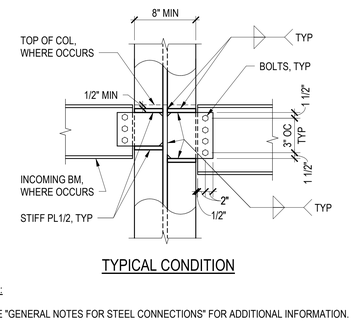


SECTION

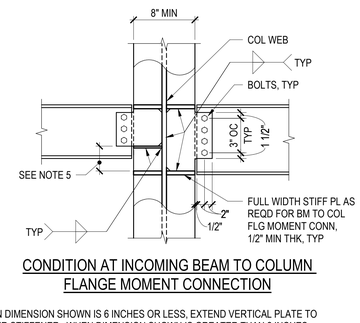
NOTES:

1. FOR A GIVEN BEAM AND REQUIRED REACTION, SEE "TABLE A" FOR BOLT SIZE AND TYPE, NUMBER OF BOLTS, AND MAXIMUM COPE LENGTH.
2. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.
3. PROVIDE FILLET WELD AT BUILT-UP BEAMS. WELD SIZE "W" SHALL BE 0.75tw, WHERE "tw" IS THE SUPPORT BEAM WEB THICKNESS.
4. CONTRACTOR SHALL PERFORM ULTRASONIC TESTING AND INSPECTION OF SUPPORT BEAM BOTTOM FLANGE PER SPECIFICATION FOR WELDED CONNECTIONS.

19 TYPICAL BEAM TO COLUMN WEB SHEAR CONNECTION



TYPICAL CONDITION



CONDITION AT INCOMING BEAM TO COLUMN
FLANGE MOMENT CONNECTION

NOTES:

1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
2. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.
3. BEAMS MAY BE SKEWED UP TO 30 DEGREES.
4. THIS DETAIL SHALL BE USED WITH W10, W12, AND W14 COLUMNS ONLY.
5. WHEN DIMENSION SHOWN IS 6 INCHES OR LESS, EXTEND VERTICAL PLATE TO LOWER STIFFENER. WHEN DIMENSION SHOWN IS GREATER THAN 6 INCHES PROVIDE ADDITIONAL STIFFENER PLATE PL1/2 AT BOTTOM OF VERTICAL PLATE.

Reserved for permit stamp

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date 7/15/2022

revisions:

no.	date	by

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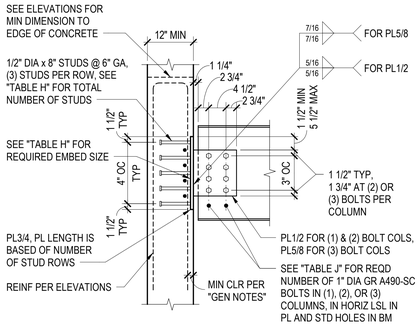
TYPICAL STEEL DETAILS

S4.12



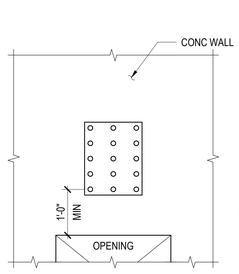
SECTION

TABLE H							
TOTAL NUMBER OF STUDS	MAXIMUM REACTION (KIPS)						
	f'c = 4,000 PSI	f'c = 5,000 PSI	f'c = 6,000 PSI	f'c = 7,000 PSI	f'c = 8,000 PSI	f'c = 9,000 PSI	f'c = 10,000 PSI
9	47	52	58	62	66	71	74
12	55	61	67	72	77	82	87
15	62	70	76	83	88	94	99
18	70	78	86	93	99	105	111
21	78	87	95	103	110	117	123
24	86	96	105	113	121	129	136
27	93	104	114	124	132	140	148
30	101	113	124	134	143	152	160
33	109	122	133	144	154	163	172
36	117	130	143	154	165	175	185
39	124	139	152	165	176	187	197

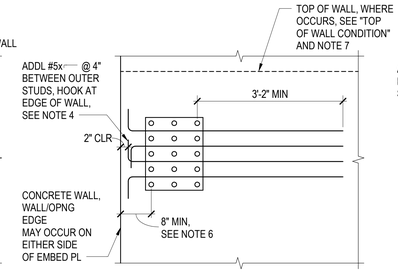
TABLE J			
NUMBER OF BOLTS PER COLUMN	MAXIMUM REACTION (KIPS)		
	(1) BOLT COLUMN	(2) BOLT COLUMNS	(3) BOLT COLUMNS
2	9	19	30
3	18	33	49
4	31	53	71
5	46	75	101
6	62	100	132
7	78	128	167
8	95	158	204
9	112	189	245
10	128	221	288
11	144	253	333
12	160	286	379

NOTES:

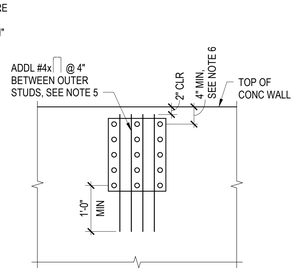
1. EMBED AND SHEAR TAB SHALL BE SIZED FOR THE REACTION SHOWN ON PLAN. CONTRACTOR SHALL VERIFY THAT SELECTED EMBED SIZE IS ADEQUATE TO RECEIVE SHEAR TAB.
2. DO NOT COPE STEEL BEAM FLANGES.
3. MINIMUM NUMBER OF BOLT ROWS SHALL CONFORM TO THE REQUIREMENTS OF "GENERAL NOTES FOR STEEL CONNECTIONS."
4. ADDITIONAL HORIZONTAL REINFORCEMENT MAY BE OMITTED WHERE DISTANCE FROM WALL EDGE TO CLOSEST STUD IS 2'-0" OR GREATER, OR WHERE WALL HORIZONTAL REINFORCEMENT HAS A MAXIMUM SPACING OF 8" OC FOR #4 AND #5 BARS OR 12" OC FOR #6 BARS AND GREATER.
5. ADDITIONAL VERTICAL REINFORCEMENT MAY BE OMITTED WHERE DISTANCE FROM WALL EDGE TO CLOSEST STUD IS 1'-0" OR GREATER.
6. WHEN AN EMBED HAS (21) OR MORE STUDS AND ALL STUDS ARE 12" OR GREATER FROM ANY VERTICAL AND HORIZONTAL WALL EDGES, EMBED CAPACITIES IN "TABLE H" MAY BE INCREASED BY 40%.
7. WHERE ANY STUD ON AN EMBED IS LESS THAN 12" FROM BOTH A VERTICAL AND A HORIZONTAL WALL EDGE, REDUCE EMBED CAPACITIES IN "TABLE H" BY 25%.
8. WHERE A STEEL BEAM FRAMES INTO THE END OF A WALL LESS THAN 32" THICK OR INTO THE FACE OF A COLUMN LESS THAN 32" WIDE, SEE "TYPICAL TYPE C25" DETAIL.
9. WHERE A STEEL BEAM FRAMES INTO A CONCRETE BEAM OR A CONCRETE COUPLING BEAM, SEE "TYPICAL TYPE C26" DETAIL.
10. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.



FIELD OF WALL CONDITION



EDGE OF WALL CONDITION

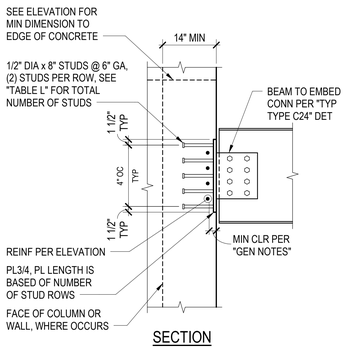


TOP OF WALL CONDITION

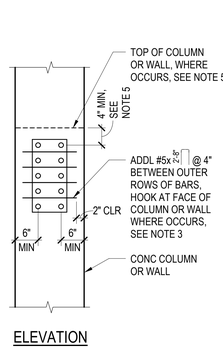
8 TYPICAL TYPE C24 - STEEL CONNECTION TO CONCRETE WALL

NOTES:

1. EMBED AND SHEAR TAB SHALL BE SIZED FOR THE REACTION SHOWN ON PLAN. CONTRACTOR SHALL VERIFY THAT SELECTED EMBED SIZE IS ADEQUATE TO RECEIVE SHEAR TAB.
2. DO NOT COPE STEEL BEAM FLANGES.
3. ADDITIONAL HORIZONTAL U-BAR REINFORCEMENT MAY BE OMITTED WHERE DISTANCE FROM WALL EDGE TO CLOSEST STUD IS 2'-0" OR GREATER, OR WHERE WALL HORIZONTAL REINFORCEMENT HAS A MAXIMUM SPACING OF 8" OC FOR #4 AND #5 BARS OR 12" OC FOR #6 BARS AND GREATER.
4. WHERE THE REACTION SHOWN ON PLAN EXCEEDS THE VALUES SHOWN IN "TABLE L", SEE "TYPICAL TYPE C27" DETAIL.
5. WHERE TOP ROW OF STUDS IS LESS THAN 12" FROM TOP OF WALL OR COLUMN, REDUCE EMBED CAPACITIES IN "TABLE L" BY 30%.
6. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.



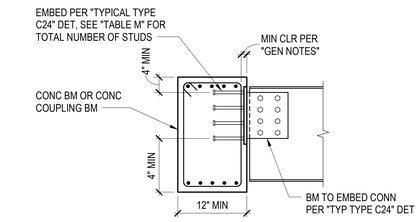
SECTION



ELEVATION

TABLE L							
TOTAL NUMBER OF STUDS	MAXIMUM REACTION (KIPS)						
	f'c = 4,000 PSI	f'c = 5,000 PSI	f'c = 6,000 PSI	f'c = 7,000 PSI	f'c = 8,000 PSI	f'c = 9,000 PSI	f'c = 10,000 PSI
6	42	47	49	49	49	49	49
8	47	53	58	63	66	66	66
10	52	59	64	69	74	79	82
12	58	64	71	76	82	87	91
14	63	70	77	83	89	94	99
16	68	76	83	90	96	102	108
18	73	82	90	97	104	110	116
20	78	88	96	104	111	118	124
22	83	93	102	110	118	125	132
24	89	99	109	117	125	133	140
26	94	105	115	124	133	141	149

13 TYPICAL TYPE C25 - STEEL CONNECTION TO CONCRETE COLUMN OR END OF WALL



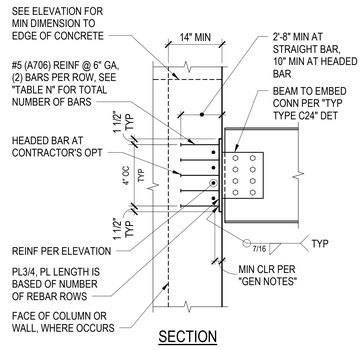
SECTION

TABLE M							
TOTAL NUMBER OF STUDS	MAXIMUM REACTION (KIPS)						
	f'c = 4,000 PSI	f'c = 5,000 PSI	f'c = 6,000 PSI	f'c = 7,000 PSI	f'c = 8,000 PSI	f'c = 9,000 PSI	f'c = 10,000 PSI
9	22	24	27	29	31	33	34
12	27	30	33	36	39	41	43
15	33	37	40	43	46	49	52
18	38	43	47	51	54	57	61
21	44	49	54	58	62	66	69
24	49	55	60	65	70	74	78
27	55	61	67	73	78	82	87
30	60	67	74	80	85	91	96
33	66	74	81	87	93	99	104
36	71	80	87	94	101	107	113
39	77	86	94	102	109	115	122

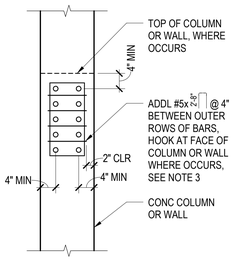
NOTES:

1. EMBED AND SHEAR TAB SHALL BE SIZED FOR THE REACTION SHOWN ON PLAN. CONTRACTOR SHALL VERIFY THAT SELECTED EMBED SIZE IS ADEQUATE TO RECEIVE SHEAR TAB.
2. DO NOT COPE STEEL BEAM FLANGES.
3. ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.

16 TYPICAL TYPE C26 - STEEL CONNECTION TO CONCRETE BEAM



SECTION



ELEVATION

TABLE N	
TOTAL NUMBER OF #5 BARS	MAXIMUM REACTION (KIPS)
6	48
8	67
10	86
12	105
14	124
16	144
18	163
20	182
22	201
24	220
26	240

18 TYPICAL TYPE C27 - HEAVY STEEL CONNECTION TO CONCRETE COLUMN OR WALL

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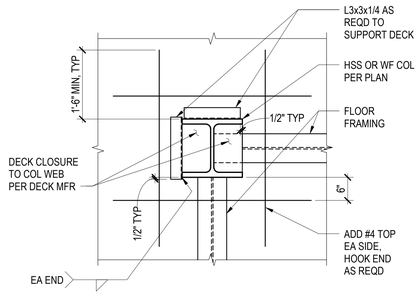
TYPICAL STEEL BEAM CONNECTIONS TO CONCRETE

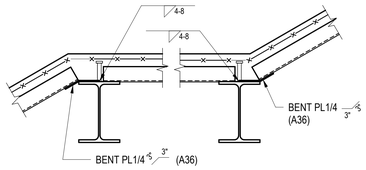
S4.13



NOTES:

1. SEE "TYPICAL DECK EDGE CONDITIONS" FOR DECK EDGE SUPPORT AT COLUMNS.

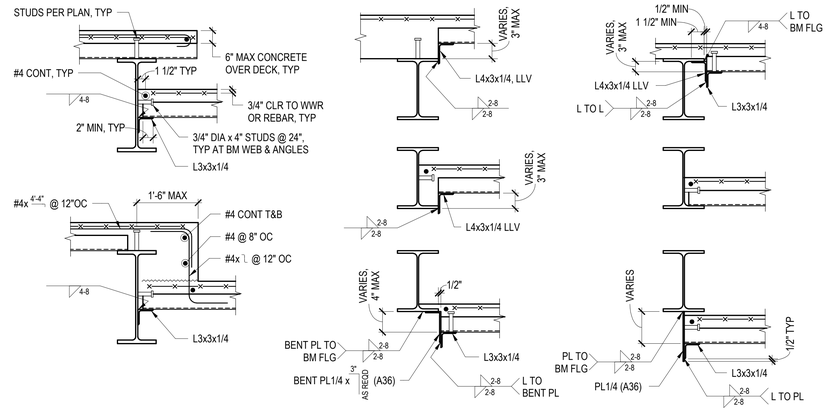
2 TYP DECK SUPPORT AND REINF AT COLS

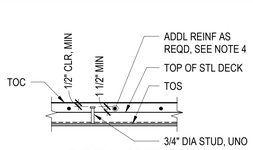


NOTES:

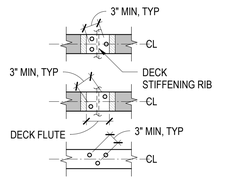
1. STUDS ARE TYPICALLY 3/4 INCH DIAMETER AT 24 INCHES ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE.
2. AT ALL ANGLES AND WEBS LESS THAN OR EQUAL TO 1/4 INCH THICK, USE 1/2 INCH DIAMETER x 4 INCH STUDS AT 12 INCHES ON CENTER.
3. DECK SPANS EITHER DIRECTION, SEE PLANS.
4. ROOF DECK SIMILAR.

3 TYPICAL DECK SUPPORT DETAILS

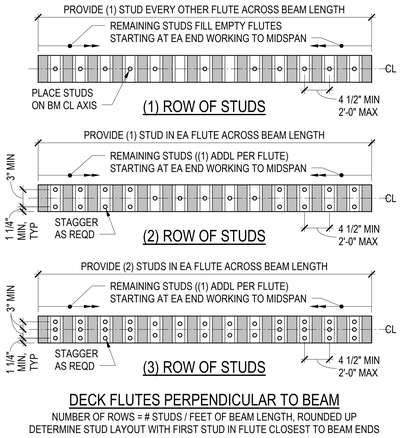




TYPICAL STUD

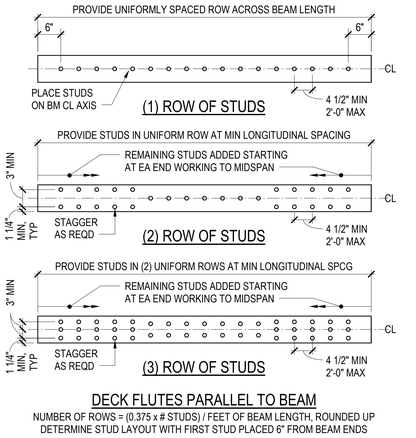


STAGGER DETAILS (USE AS REQUIRED)



DECK FLUTES PERPENDICULAR TO BEAM

NUMBER OF ROWS = # STUDS / FEET OF BEAM LENGTH, ROUNDED UP
DETERMINE STUD LAYOUT WITH FIRST STUD IN FLUTE CLOSEST TO BEAM ENDS



DECK FLUTES PARALLEL TO BEAM

NUMBER OF ROWS = (0.375 x # STUDS) / FEET OF BEAM LENGTH, ROUNDED UP
DETERMINE STUD LAYOUT WITH FIRST STUD PLACED 6" FROM BEAM ENDS

6 STEEL DECK NOTES

NOTES:

1. SHEAR STUD PLACEMENT TO BEAM IS PER TYPICAL DETAIL. COORDINATE DECK AND STEEL FRAMING PLACEMENT. WHERE DECK LOW FLUTE DOES NOT ALIGN WITH BEAM FLANGE BELOW, PROVIDE LOCAL MODIFICATION TO DECK TO FACILITATE SHEAR STUD PLACEMENT. SIMILARLY MODIFY SUPPORT CONDITIONS WHEN DECK IS SKEWED TO THE BEAMS AS REQUIRED TO ALLOW FOR PLACEMENT OF STUDS IN ACCORDANCE WITH THE TYPICAL DETAIL.
2. ALL OPENINGS IN SLAB OR ROOF DECKS SHALL BE COORDINATED, FRAMED, AND REINFORCED PER THE TYPICAL DETAILS. DO NOT CUT DECK UNTIL FRAMING IS PLACEDAND CONCRETE IS CURED AS NOTED.
3. CONDUIT IN SLAB ON DECK IS PERMITTED ONLY WHEN FULLY SATISFYING THE REQUIREMENTS OF THE TYPICAL CONDUIT IN SLAB ON STEEL DECK DETAIL. WHEN CONDUIT VERTICALLY RUNS INTO THE SLAB, IT IS TO BE TREATED AS A DECK OPENING AND REINFORCED AS REQUIRED PER THE TYPICAL DETAILS.
4. JOINTING OF SLABS ON DECK SHALL BE PER THE TYPICAL DETAILS AND SATISFY THE REQUIREMENTS OF THE GENERAL NOTES.
5. SUPPORT ALL DECK AT FRAMING, COLUMNS, AND WALLS PER THE TYPICAL DETAILS. DISCONTINUOUS TOP OF COLUMNS SUPPORTING SLAB OR ROOF DECK SHALL HAVE A 3/8" CAP PLATES SUITABLY SIZED TO SUPPORT THE DECK UNLESS OTHERWISE SHOWN.
6. ROOF DECK EDGE CONDITIONS SHALL BE PER THE TYPICAL DETAILS. AT ROOF DECK EDGE CONDITIONS WHERE PLATE IS NOT REQUIRED TO SUPPORT EXTENT OF ARCHITECTURAL ROOFING, EXTERIOR CLADDING SYSTEMS, OR OTHER ARCHITECTURAL FEATURES IT, MAY BE OMITTED. CONTRACTOR TO COORDINATE.
7. SLAB ON DECK EDGE CONDITIONS SHALL BE PER THE TYPICAL DECK EDGE CONDITIONS DETAIL. AT SLAB EDGE CONDITIONS WHERE RAILING, EXTERIOR CLADDING, OR OTHER ARCHITECTURAL FEATURES REQUIRE A PLATE EDGE, SEE "TYPICAL DECK EDGE AT EXTERIOR CLADDING OR RAILING" DETAIL. CONTRACTOR TO COORDINATE.
8. CONTRACTOR TO COORDINATE SLAB ON DECK EDGE CONDITIONS WITH ELEVATOR AND DOCK LEVELER SILLS DETAIL WHERE OCCURS. REFER TO "TYPICAL ELEVATOR AND DOCK LEVELER SILL" DETAIL FOR REQUIREMENTS.

7 TYPICAL SHEAR STUD PLACEMENT AND ADDED REINFORCING

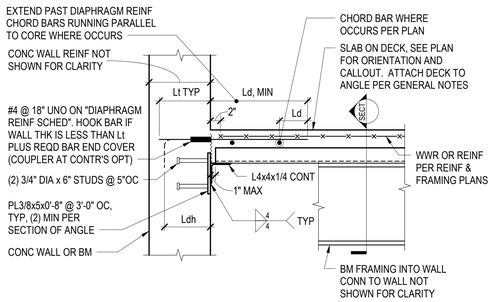
NOTES:

1. SEE PLAN FOR REQUIRED NUMBER OF STUDS. SEE "GENERAL NOTES" FOR MINIMUM NUMBER OF STUDS AND MINIMUM STEEL COMPOSITE DECK TO STEEL BEAMS FASTENING REQUIREMENTS.
2. UNLESS NOTED OTHERWISE, STUDS ARE TO BE SPACED AS SHOWN IN THIS DETAIL AND PLACED SYMMETRICALLY ABOUT THE BEAM CENTERLINE AXIS. IF REQUIRED SPACING IS NOT POSSIBLE DUE TO DECK CONFIGURATION, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED.
3. FOR CANTILEVER SPANS, STUDS SHALL BE PLACED IN ONE ROW ALONG THE BEAM CENTERLINE AXIS AT A MAXIMUM SPACING OF 2'-0". STUDS PLACED ON THE CANTILEVER SPAN ARE NOT INCLUDED IN THE NUMBER OF STUDS SHOWN ON PLAN.
4. WHERE WELDED WIRE REINFORCING IS USED AS SLAB REINFORCEMENT, ADDITIONAL REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE BEAM, ACROSS THE BEAM AND CANTILEVERED SPANS AS FOLLOWS:

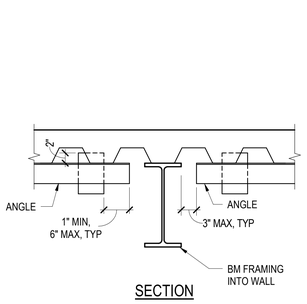
   < 3 STUDS / FT - ADD NONE
   3 OR MORE STUDS / FT - ADD #4x5'-0" @ 12"

5. FOR BEAMS WEIGHING LESS THAN OR EQUAL TO 16 LBS/FT, STUDS SHALL BE PLACED ONLY ON THE CENTERLINE OF THE BEAM.

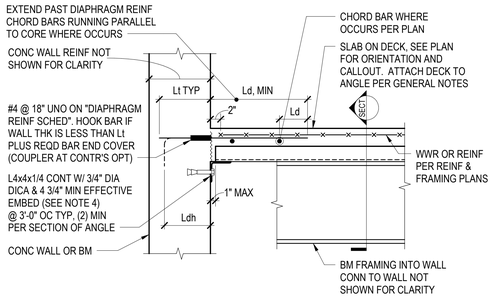
16 TYPICAL DECK SUPPORT AT CONCRETE



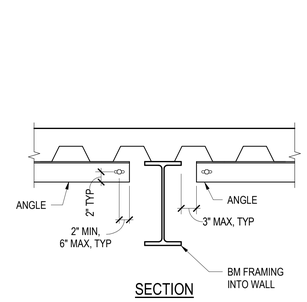
CAST-IN-PLACE OPTION



SECTION



POST-INSTALLED OPTION

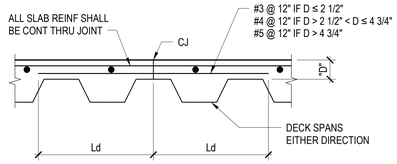


SECTION

NOTES:

1. ROOF DECK IS SIMILAR.
2. CONTRACTOR TO COORDINATE REINF AND ANCHOR / EMBED PLACEMENT.
3. AT CONTRACTOR'S OPTION, FOR DECK PARALLEL TO WALL CONDITION, IT IS ACCEPTABLE TO SHORE DECK AT EDGES UNTIL CONCRETE REACHES DESIGN COMPRESSIVE STRENGTH IN LIEU OF PROVIDING L4x4 AND DICAS / EMBEDS. PROVIDE JOINT KEY PER GENERAL NOTES.
4. ANCHORS SHALL BE HILTI KWIK BOLT TZ, SIMPSON STRONG TIE STB2, OR APPROVED EQUAL. PROVIDE HORIZONTAL LSL HOLES IN L4x4.

DIAPHRAGM REINF SCHEDULE		
LOCATION	BAR TYPE	SPACING
-	-	-



NOTES:

1. SUBMIT LOCATIONS OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE BEFORE FORMING.
2. REFER TO GENERAL NOTES FOR CONSTRUCTION JOINT LOCATION REQUIREMENTS.

13 TYP SLAB ON DECK CONSTRUCTION JOINT

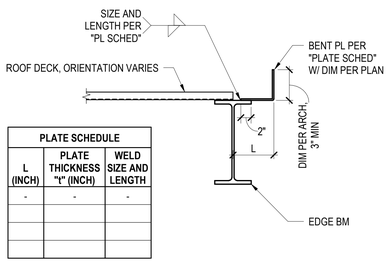
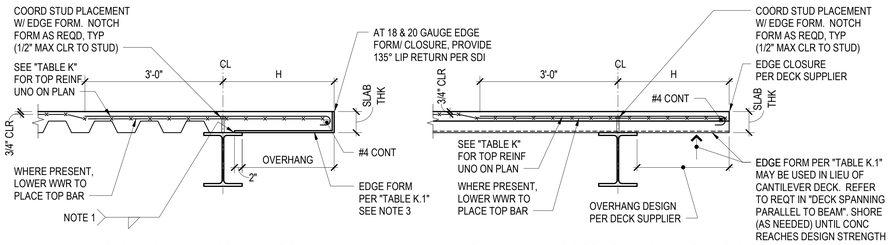


PLATE SCHEDULE		
L (INCH)	PLATE THICKNESS "t" (INCH)	WELD SIZE AND LENGTH
-	-	-

NOTES:

1. FIELD INSTALL BENT PLATE WHERE REQUIRED TO SATISFY ARCHITECTURAL TOLERANCES.
2. PROVIDE PJP 1/4 (E) MIN WELD AT VERTICAL LEG TO SPLICE BENT PLATES, TYP. FOR PL THICKNESS LESS THAN 3/8", PROVIDE PJP 1/8 (E) MIN. GRIND SMOOTH AT ARCHITECTURALLY EXPOSED CONDITIONS.

18 TYPICAL ROOF DECK EDGE



DECK SPANNING PARALLEL TO BEAM

DECK SPANNING PERPENDICULAR TO BEAM

TABLE K	
H	REINFORCING
0" TO 9" OVER 9" TO 1'-4"	- #4 @ 18"
OVER 1'-4" TO 2'-0"	#4 @ 12"
OVER 2'-0" TO 2'-6"	#4 @ 8"

TABLE K.1

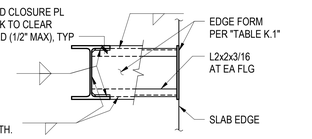
MAX SLAB THICKNESS (IN)	SLAB EDGE CLOSURE PLATE GAUGE / THICKNESS AT DECK PARALLEL TO BEAM															
	OVERHANG (IN)															
	0	1	2	3	4	5	6	7	8	9	10	11	12	14	16	18
4	20	20	20	18	16	16	16	14	14	12	12	10	10	3/16	3/16	1/4
4.5	20	20	20	18	16	16	16	14	12	12	12	10	3/16	3/16	1/4	1/4
5	20	20	18	18	16	16	14	14	12	12	10	10	3/16	3/16	1/4	1/4
5.5	20	18	18	16	16	14	14	12	12	12	10	10	3/16	3/16	1/4	1/4
6	20	18	16	16	14	14	12	12	12	12	10	10	3/16	3/16	1/4	1/4
6.5	18	16	16	14	14	12	12	12	12	10	10	3/16	3/16	3/16	1/4	5/16
7	18	16	14	14	12	12	12	12	10	10	10	3/16	3/16	1/4	1/4	5/16
7.5	16	14	14	12	12	12	12	10	10	10	3/16	3/16	3/16	1/4	1/4	5/16
8	16	14	12	12	12	12	10	10	10	3/16	3/16	3/16	3/16	1/4	1/4	5/16

NOTES:

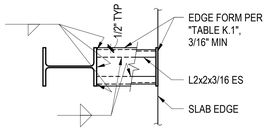
1. WELDING SHALL BE AS FOLLOWS:

MATERIAL	WELD SIZE x LENGTH AT SPACING
GAUGE METAL	1/8 "x 1 1/2" @ 12" OC (1" MIN EA PIECE END)
PLATES	3/16 "x 2" @ 12" OC (2" MIN EA PIECE END)

2. MATERIALS ARE TO BE AS FOLLOWS:
   18 GAUGE AND LIGHTER - ASTM A653 - Fy = 33 KSI MINIMUM.
   16 GAUGE AND HEAVIER - ASTM A653 - Fy = 50 KSI MINIMUM.
   AT CONTRACTOR'S OPTION, ASTM A36 (Fy=36 KSI MIN) MAY BE USED FOR PLATES.
3. FOR CONDITIONS WHERE SLAB EDGE EXCEEDS 18 INCH OVERHANG OR SLAB THICKNESS EXCEEDS MAX PER TABLE K.1, PROVIDE 1/4 INCH PLATE EDGE FORM AND SHORE THE EDGE UNTIL CONCRETE REACHES 28 DAY COMPRESSIVE STRENGTH.
4. AT CONTRACTOR'S OPTION, ALTERNATE METHODS OF PROVIDING EDGE CONSTRUCTION MAY BE USED AS DESIGNED BY THE CONTRACTOR. PROVIDE SHORING AND FRAMING MATERIALS AS REQUIRED.
5. THE SLAB EDGE CLOSURE PLATE IS ONLY DESIGNED TO SUPPORT WET CONCRETE AND CONSTRUCTION LOAD. DO NOT USE THE PLATE TO DIRECTLY SUPPORT CLADDING LOADS WITHOUT PRIOR APPROVAL FROM THE ENGINEER OF RECORD.
6. AT CORNER COLUMNS, USE BOTH "TYPICAL EDGE CONDITION AT COLUMN" DETAILS. EDGE FORM SHALL BE MITERED AND WELDED.
7. AT BLOCKOUTS FOR CLADDING CONNECTIONS, EDGE REINFORCING IS TO BE CONTINUOUS.
8. AT RECESS CONDITIONS, EDGE PLATE HEIGHT TO MATCH THE SLAB HEIGHT AT RECESS.
9. FOR ELEVATOR AND DOCK LEVELER SILLS SEE "TYPICAL ELEVATOR AND DOCK LEVELER SILL" DETAIL.
10. CONSTRUCTION LOAD AT EDGE FORM SHALL NOT EXCEED 20 PSF.
11. SEE "TYPICAL EXTENDED SLAB ON DECK EDGE" DETAIL.



COLUMN WITH WEB PARALLEL



COLUMN WITH WEB PERPENDICULAR

TYPICAL EDGE CONDITION AT COLUMN
SEE NOTE 6

19 TYPICAL DECK EDGE CONDITIONS

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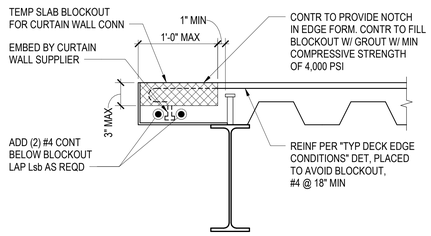
TYPICAL STEEL DECK DETAILS

S4.14

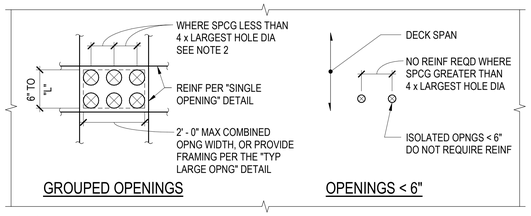
1 TYPICAL CURTAIN WALL CONNECTION



NOTES:

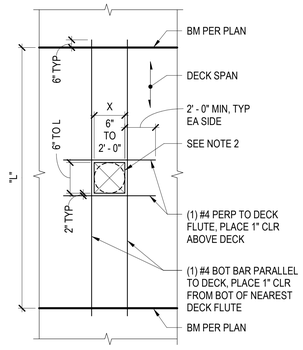
1. SEE "TYPICAL DECK EDGE CONDITIONS" DETAIL FOR INFORMATION NOT SHOWN.

2 TYPICAL COMPOSITE DECK - SMALL OPENINGS (X ≤ 2'-0")



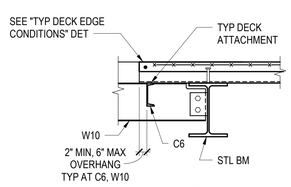
NOTES:

1. BLOCK OUT OPENING BEFORE PLACING CONCRETE.
2. REMOVE BLOCKOUT AND CUT DECK AFTER CONCRETE HAS REACHED ITS DESIGN STRENGTH.
3. SMALL OPENINGS WHERE NOTED REQUIRE A MINIMUM CENTER-TO-CENTER SPACING FROM ADJACENT SMALL OPENINGS OF FOUR TIMES THE MAXIMUM OPENING DIMENSION. IF THIS SPACING REQUIREMENT CANNOT BE ACHIEVED, THE GROUP SHALL BE REINFORCED AS IF ONE COMBINED OPENING OR PENETRATION.
4. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS WITH THE ARCHITECTURAL DRAWINGS AND MECH, ELEC, PLUMBING, AND OTHER TRADES.
5. WHERE OPENING WIDTH "X" IS GREATER THAN 2'-0", REFER TO THE "TYPICAL COMPOSITE FLOOR DECK - FRAMED OPENINGS" DETAIL.



SINGLE OPENING

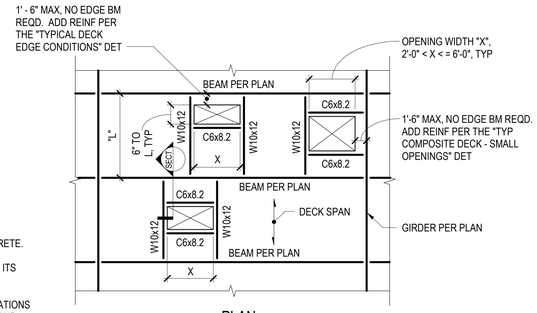
4 TYPICAL COMPOSITE DECK - FRAMED OPENINGS (2'-0" < X ≤ 6'-0")



SECTION

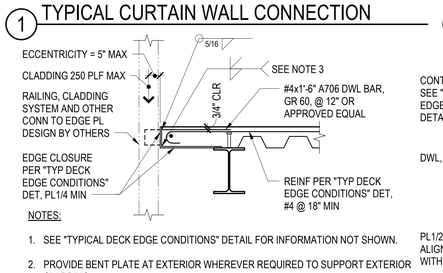
NOTES:

1. BLOCK OUT OPENING AND INSTALL FRAMING BEFORE PLACING CONCRETE.
2. REMOVE BLOCKOUT AND CUT DECK AFTER CONCRETE HAS REACHED ITS DESIGN STRENGTH.
3. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS WITH THE ARCHITECTURAL DRAWINGS AND MECH, ELEC, PLUMBING, AND OTHER TRADES.
4. WHERE OPENING WIDTH "X" IS 2'-0" OR LESS, REFER TO THE "TYPICAL COMPOSITE FLOOR DECK - SMALL OPENING" DETAIL.



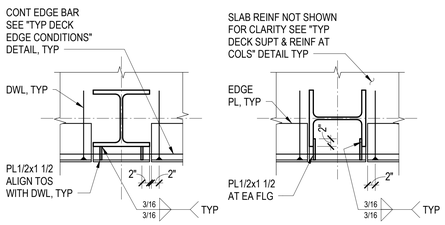
PLAN

6 TYPICAL DECK EDGE AT EXTERIOR CLADDING OR RAILING



NOTES:

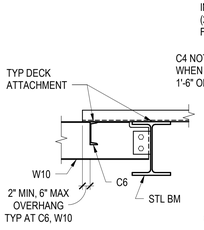
1. SEE "TYPICAL DECK EDGE CONDITIONS" DETAIL FOR INFORMATION NOT SHOWN.
2. PROVIDE BENT PLATE AT EXTERIOR WHEREVER REQUIRED TO SUPPORT EXTERIOR CLADDING.
3. AT CONTRACTOR'S OPTION, FABRICATE EDGE PLATE FROM TWO PIECES OF PLATE AND WELD. GRIND SMOOTH TO MATCH BENT PLATE AT EXPOSED PLATE CONDITIONS.
4. EXTERIOR SLAB EDGE SHALL NOT VERTICALLY SUPPORT PRECAST.
5. CLADDING LOAD MAGNITUDES AND LOCATIONS SHALL BE SUBMITTED TO ENGINEER FOR REVIEW PRIOR TO FABRICATION. BASED ON REVIEW, ADJUSTMENT TO THIS DETAIL MAY BE REQUIRED.
6. AT RAILING ATTACHMENT, PROVIDE 1/2" BENT PLATE. REDUCE DOWEL AND SLAB EDGE REINFORCING SPACING TO 6".



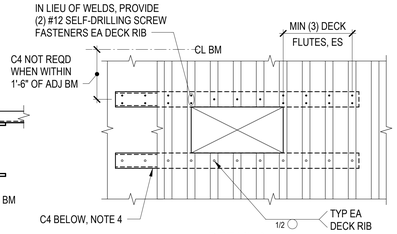
COLUMN W/ WEB PERPENDICULAR

COLUMN W/ WEB PARALLEL

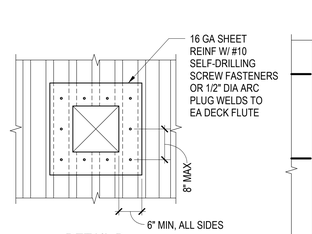
TYPICAL EDGE CONDITION AT COLUMN



SECTION



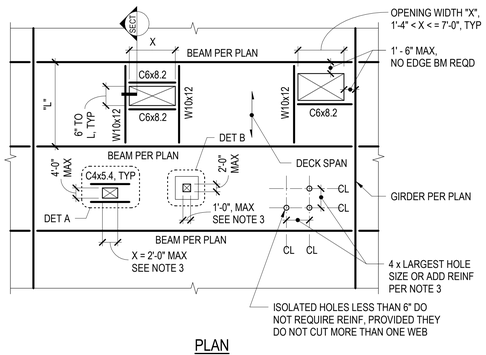
DETAIL A



DETAIL B

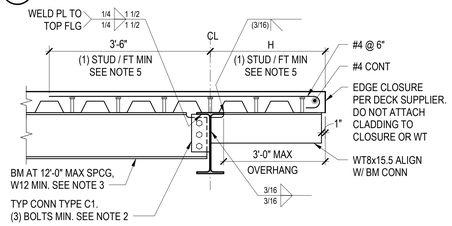
NOTES:

1. INSTALL INFILL FRAMING OR DECK REINFORCEMENT BEFORE CUTTING HOLES OR OPENINGS.
2. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS WITH THE ARCHITECTURAL DRAWINGS AND MECH, ELEC, PLUMBING, AND OTHER TRADES.
3. SMALL OPENINGS WHERE NOTED REQUIRE A MINIMUM ON-CENTER SPACING FROM ADJACENT OPENINGS OF FOUR TIMES THE MAXIMUM OPENING DIMENSION. WHERE THE REQUIRED OPENING LAYOUT CANNOT CONFORM TO THESE REQUIREMENTS, PROVIDE DECK REINFORCEMENT OR INFILL FRAMING AS IF THE GROUP WAS ONE COMBINED LARGE OPENING.
4. WITH ARCHITECT APPROVAL, THE CONTRACTOR MAY OPT TO INSTALL CHANNELS ON TOP OF DECK WITH EQUIVALENT ARC PLUG WELDS OR SCREW FASTENERS.

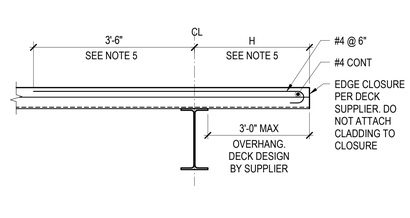


PLAN

8 TYPICAL ROOF DECK OPENINGS (X ≤ 7'-0")



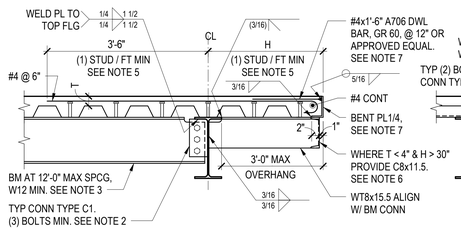
DECK SPANNING PARALLEL TO BEAM



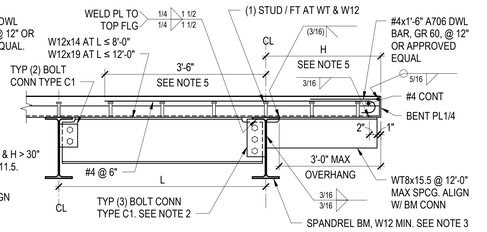
DECK SPANNING PERPENDICULAR TO BEAM

NOTES:

1. SEE "TYPICAL DECK EDGE CONDITIONS" DETAIL FOR INFORMATION NOT SHOWN.
2. SEE "TYPICAL TYPE C1 - SINGLE PLATE SHEAR CONNECTION" DETAIL FOR BOLTS, PLATE AND WELDS. HOLES SHALL BE STANDARD ROUND HOLES (STD).
3. IF BACKSPAN OR SPANDREL BEAM ARE LESS THAN W12, NOTIFY ENGINEER OF RECORD FOR REVIEW.
4. AT COLUMN WELD WT OUTRIGGER TO COLUMN WITH ALL-AROUND 3/16" FILLET WELD.
5. WHERE CLADDING IS SUPPORTED AT EDGE, NO SLAB OPENINGS LARGER THAN 1'-0" WIDE AT OVERHANG AND FIRST 3'-6" OF BACK-SPAN. IF NOT MET, NOTIFY ENGINEER OF RECORD FOR REVIEW AND APPROVAL.

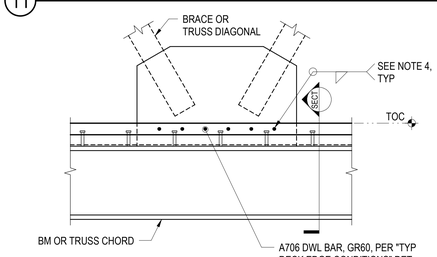


DECK SPANNING PARALLEL TO BEAM



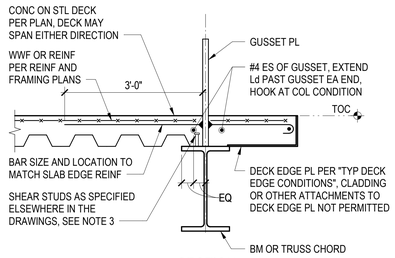
DECK SPANNING PERPENDICULAR TO BEAM

11 TYPICAL EXTENDED DECK EDGE (1'-6" < OVERHANG ≤ 3'-0")



NOTES:

1. SIMILAR AT GUSSET TO COLUMN CONNECTIONS.
2. SEE "TYPICAL DECK EDGE CONDITIONS" DETAIL FOR ADDITIONAL INFORMATION.
3. AT GUSSET, LOCATE STUDS ON OPPOSITE SIDE OF GUSSET PLATE FROM DECK EDGE. WHERE WELDING ACCESS IS PRECLUDED BY CONNECTION GEOMETRY, STUDS SHALL BE UNIFORMLY DISTRIBUTED ON EACH SIDE OF GUSSET PLATE.
4. WELD SIZE SHALL BE 5/16" FOR #4 BARS AND 7/16" FOR #5 BARS.

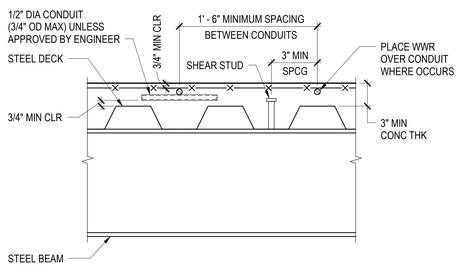


SECTION

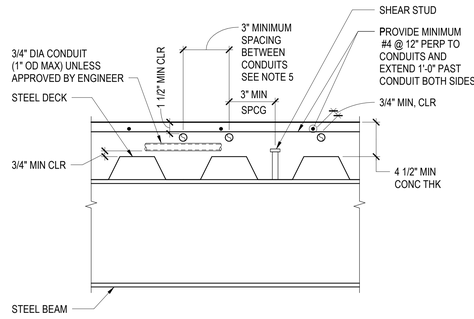
13 TYPICAL EXTENDED DECK EDGE AT EXTERIOR CLADDING OR RAILING (1'-6" < OVERHANG ≤ 3'-0")

NOTES:

1. REINFORCEMENT TAKES PRECEDENCE OVER CONDUIT. DO NOT RAISE, LOWER OR MOVE REINFORCEMENT TO ACCOMMODATE CONDUIT.
2. LOCATE CONDUITS IN CONCRETE THICKNESS ABOVE THE DECK FLUTES ONLY. CHAIR CONDUIT OFF DECK TO PROVIDE MINIMUM SPECIFIED CLEAR COVER.
3. ROUTE CONDUIT UNDER THE DECK OR IN OTHER LOCATIONS IF SLAB THICKNESS OR SPACING REQUIREMENTS CANNOT BE MET.
4. CONDUIT LAYOUT SHALL BE COORDINATED BY THE CONTRACTOR WITH MEP, ARCH AND OTHER TRADES. CONTRACTOR TO SUBMIT CONDUIT LAYOUT TO ENGINEER FOR REVIEW AND APPROVAL.
5. THE MAXIMUM NUMBER OF CONDUITS IN A GROUP IS (10). PROVIDE 3'-0" CLEAR MINIMUM BETWEEN GROUPS OF CONDUITS.
6. DENSE CLUSTERS OF CONDUITS, WHERE MINIMUM SPACING CANNOT BE MET, TO BE TREATED AND REINFORCED AS OPENINGS.
7. PLACE TRANSVERSE CONDUITS RUNNING OVER COMPOSITE BEAMS IN OUTER THIRD OF BEAM SPANS.
8. MINIMUM ANGLE BETWEEN CONDUITS AT CROSSOVERS SHALL BE 45 DEGREES.



LIGHT CONDUIT IN CONCRETE ON STEEL DECK



HEAVY CONDUIT IN CONCRETE ON STEEL DECK

16 TYPICAL SLAB EDGE AT GUSSET PLATE

18 TYPICAL CONDUIT IN SLAB ON STEEL DECK

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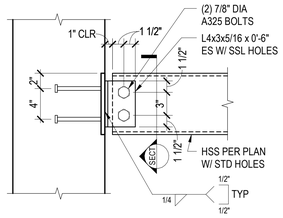
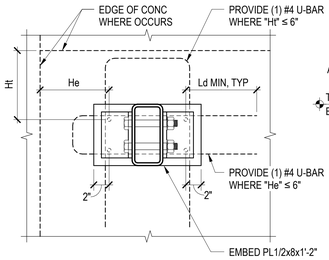
TYPICAL STEEL DECK DETAILS

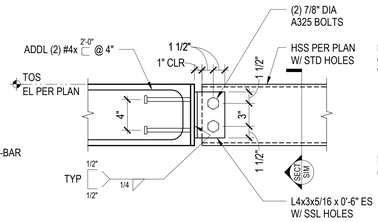
S4.15



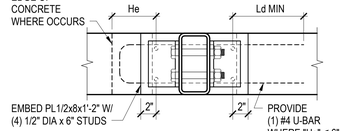
AT CONCRETE WALL



SECTION AT
CONCRETE WALL

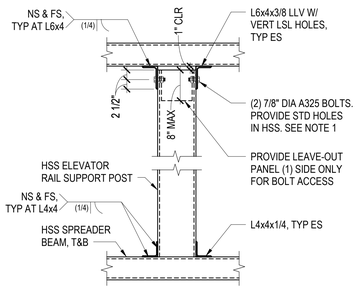


AT CONCRETE SLAB

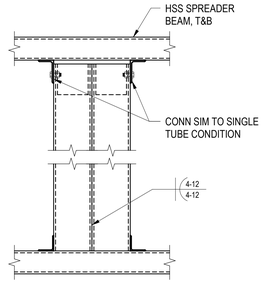


SECTION AT
CONCRETE SLAB

3 TYPICAL ELEVATOR SPREADER BEAM CONNECTION



SINGLE TUBE

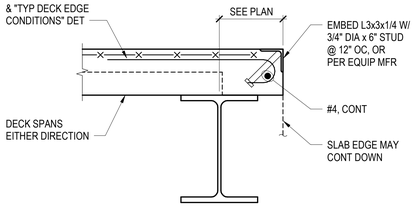


DOUBLE TUBE

NOTES:

1. WHERE NOTE APPLIES, BOLT TO BE CENTERED IN SLOTTED HOLE IN ANGLE. NUTS TO BE FINGER TIGHT. DAMAGE THREADS OF BOLT TO PREVENT BACK-OFF OF NUT.

9 TYP ELEVATOR AND DOCK LEVELER SILL



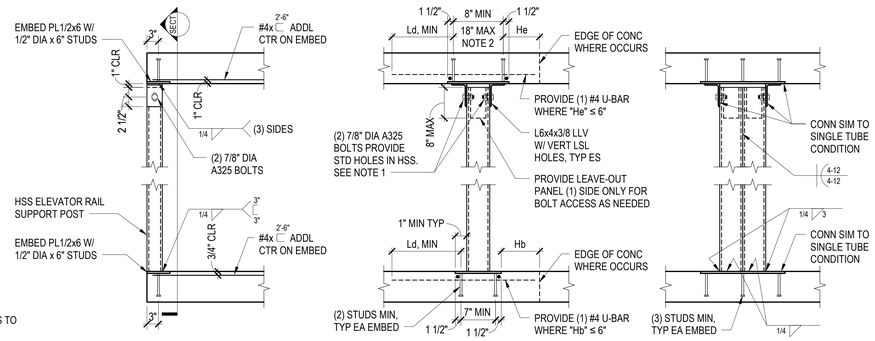
10 ELEVATOR RAIL SUPPORT POST SCHEDULE

ELEVATOR RAIL SUPPORT POST SCHEDULE	
MARK	POST SIZE
R1	HSS8x4x1/4
R2	HSS12x4x5/16
R3	HSS14x4x1/2
R4	HSS16x4x5/16
R5	HSS16x4x1/2

NOTES:

1. SEE TYPICAL ELEVATOR RAIL SUPPORT POST DETAILS FOR MORE INFORMATION.

7 TYPICAL ELEVATOR RAIL SUPPORT POST TO SPREADER BEAM



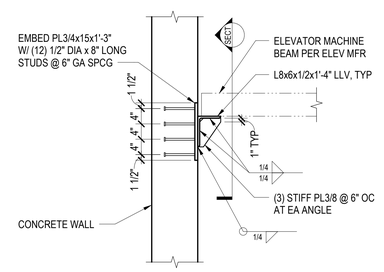
SINGLE TUBE

DOUBLE TUBE

SECTION

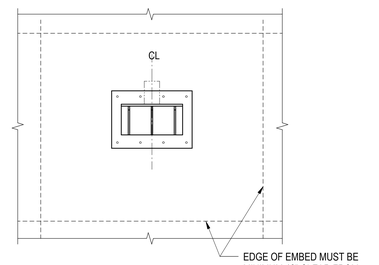
NOTES:

1. WHERE NOTE APPLIES, BOLT TO BE CENTERED IN SLOTTED HOLE IN ANGLE. NUTS TO BE FINGER TIGHT. DAMAGE THREADS OF BOLT TO PREVENT BACK-OFF OF NUT.
2. CONTRACTOR TO COORDINATE STUD SPACING WHERE ELEVATOR RAIL SUPPORT POSTS STACK ON ADJACENT LEVELS TO AVOID CONFLICTING STUD LOCATIONS. PROVIDE MINIMUM 2" OC SPACING BETWEEN STUDS.



NOTE:

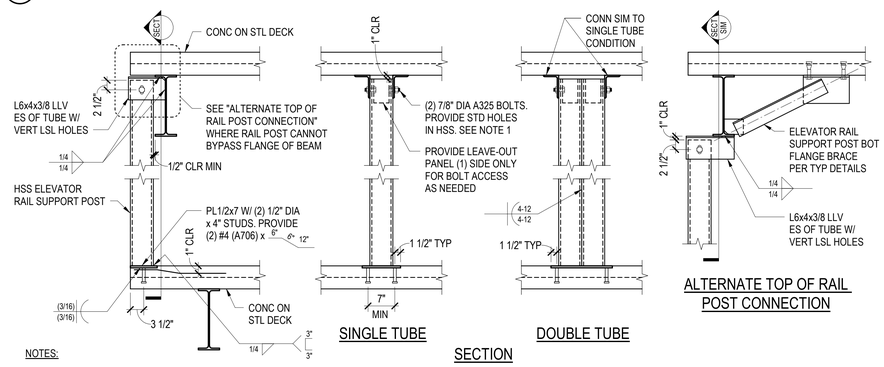
1. COORDINATE LOCATION OF EMBEDS AND ANGLES WITH ELEVATOR MANUFACTURER.
2. CONNECTION ABOVE HAS BEEN DESIGNED FOR MAXIMUM ULTIMATE REACTION OF 40 KIPS.



SECTION

11 TYPICAL ELEVATOR RAIL SUPPORT POST TO CONCRETE SLAB

14 TYPICAL ELEVATOR MACHINE BEAM LEDGER



SINGLE TUBE

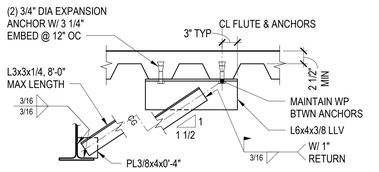
DOUBLE TUBE

ALTERNATE TOP OF RAIL
POST CONNECTION

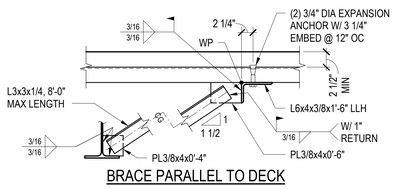
SECTION

NOTES:

1. WHERE NOTE APPLIES, BOLT TO BE CENTERED IN SLOTTED HOLE IN ANGLE.
NUTS TO BE FINGER TIGHT. DAMAGE THREADS OF BOLT TO PREVENT BACK-OFF OF NUT.



BRACE PERPENDICULAR TO DECK



BRACE PARALLEL TO DECK

18 ELEVATOR RAIL SUPPORT POST BOTTOM FLANGE BRACING

19 TYPICAL ELEVATOR RAIL SUPPORT POST TO STEEL FRAMING

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date 7/15/2022

revisions:

no.	date	by

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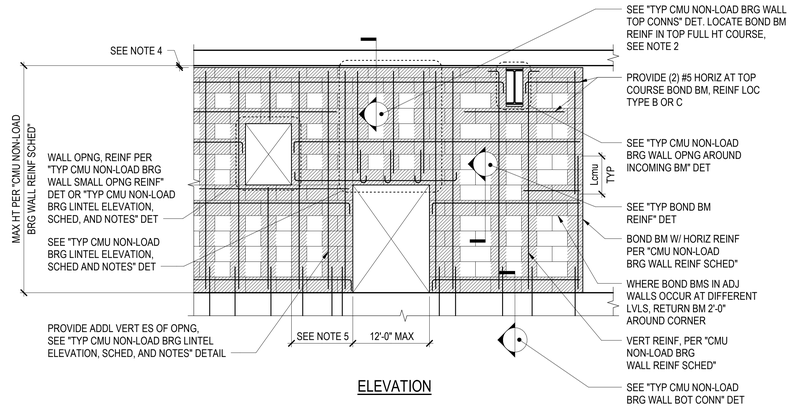
TYPICAL STEEL
DETAILS

S4.16

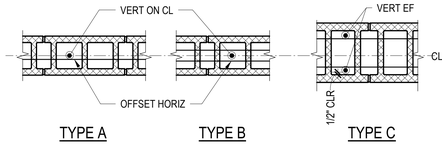


ELEVATION

CMU NON-LOAD BEARING WALL REINFORCEMENT SCHEDULE							
	HORIZONTAL REINF		VERTICAL REINF				
NOMINAL WALL THICKNESS (IN)	BOND BEAM REINF	MAX SPCG (IN)	REINF	MAX SPCG (IN)	BAR LOCATION TYPE	MAXIMUM WALL HEIGHT (FT)	REMARKS
8"	(1) #5	48	(1) #5	48	A	13'-0"	TYPE B BAR LOCATION AT TOP AND BOTTOM OF WALL
8"	(1) #5	48	(1) #5	32	A	15'-0"	TYPE B BAR LOCATION AT TOP AND BOTTOM OF WALL
8"	(1) #5	48	(1) #5	24	A	18'-0"	TYPE B BAR LOCATION AT TOP AND BOTTOM OF WALL
12"	(2) #5	48	(2) #5	48	C	17'-0"	
12"	(2) #5	48	(2) #5	24	C	22'-0"	

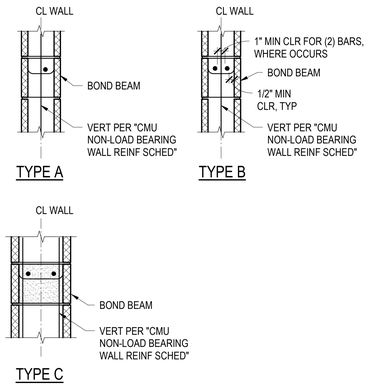
NOTES:

1. GROUT SOLID ONLY THOSE CELLS THAT CONTAIN REINFORCEMENT OR EMBEDDED ITEMS. PRIOR TO CONSTRUCTION, CONTRACTOR TO COORDINATE LOCATIONS WHERE POST INSTALLED CONNECTIONS TO WALLS WILL BE REQUIRED AND FULLY GROUT CELLS AS NEEDED FOR CONNECTIONS.
2. GROUT TOP COURSE SOLID. WHERE TOP COURSE IS NOT FULL BLOCK HEIGHT, GROUT COURSE BELOW SOLID AS WELL.
3. AT CONTRACTORS OPTION, IN PLACE OF WIRE JOINT REINFORCEMENT PROVIDE HORIZONTAL STEEL REINFORCEMENT IN A FULLY GROUTED BOND BEAM AT A MAXIMUM VERTICAL SPACING OF 96 INCHES. AT 8-INCH NOMINAL BLOCKS PROVIDE (1) #4 HORIZONTAL BAR TYPE A BAR LOCATION. AT 12-INCH NOMINAL BLOCKS PROVIDE (2) #4 HORIZONTAL BARS TYPE C BAR LOCATION.
4. PROVIDE 1 INCH CLEAR MINIMUM, 1 1/2-INCHES CLEAR MAXIMUM BETWEEN TOP OF WALL AND BOTTOM OF PLATE, DECK, OR FIREPROOFING ON BEAMS.
5. MINIMUM SPACING BETWEEN TWO DOOR OPENINGS AND SMALL OPENINGS REQUIRED BY ARCHITECTURAL AND MEP IS 5'-4". MINIMUM SPACING BETWEEN OPENINGS AND ENDS OF WALLS IS 2'-8". CONTRACTOR SHALL SUBMIT CONDITIONS NOT MEETING THIS REQUIREMENT FOR ENGINEER TO REVIEW.
6. CONTRACTOR TO COORDINATE AND SUBMIT CMU WALL ELEVATIONS SHOWING ALL OPENINGS TO BE BUILT IN A GIVEN WALL (ARCH AND MEP) FOR ENGINEER OF RECORD TO REVIEW PRIOR TO WALL CONSTRUCTION.

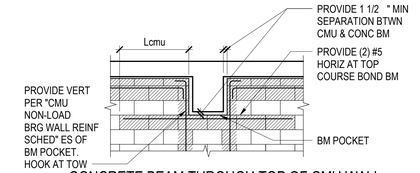


BAR LOCATION TYPES

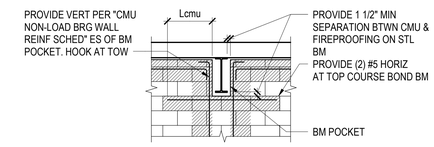
6 TYPICAL CMU NON-LOAD BEARING WALL ELEVATION, SCHEDULE AND NOTES



11 TYPICAL CMU BOND BEAM REINFORCING

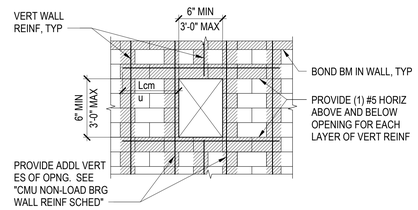


CONCRETE BEAM THROUGH TOP OF CMU WALL



STEEL BEAM THROUGH TOP OF CMU WALL

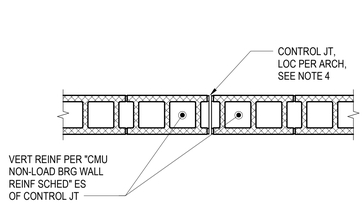
3 TYPICAL CMU NON-LOAD BEARING WALL OPENING AROUND INCOMING BEAM



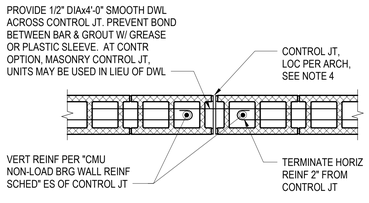
NOTES:

1. SEE "TYPICAL CMU NON-LOAD BEARING WALL ELEVATION, SCHEDULE, AND NOTES" FOR ADDITIONAL INFORMATION.
2. CONDUITS AND PIPES SHALL BE PLACED THROUGH SLEEVES OR BLOCK OUTS PROVIDED DURING WALL CONSTRUCTION.
3. WALL OPENINGS SHALL BE A MINIMUM OF 1'-6" ABOVE ANY DOOR OR WINDOW OPENINGS AND SPACED AT LEAST THREE TIMES THE LARGEST OPENING.
4. WALL PENETRATIONS LESS THAT 6-INCH DIAMETER OR 6-INCH SQUARE THAT DO NOT DISRUPT REINFORCEMENT AND ARE SPACED AT LEAST 2'-0" APART DO NOT REQUIRE ADDITIONAL TRIM REINFORCEMENT.
5. CONTRACTOR TO COORDINATE AND SUBMIT CMU WALL ELEVATIONS SHOWING ALL OPENINGS TO BE BUILT IN A GIVEN WALL (ARCH AND MEP) FOR ENGINEER OF RECORD TO REVIEW PRIOR TO WALL CONSTRUCTION.

8 TYPICAL CMU NON-LOAD BEARING WALL SMALL OPENING REINFORCING



COURSES WITHOUT HORIZONTAL REINF



COURSES WITH HORIZONTAL REINF

NOTES:

1. CONTROL JOINT SHALL BE CONSTRUCTED AS CONTINUOUS VERTICAL HEAD JOINTS USING FULL AND HALF MASONRY UNITS.
2. MORTAR SHALL BE RAKED BACK 3/4-INCH AND FILLED WITH SEALANT PER ARCHITECTURAL DRAWINGS.
3. LOCATION OF CONTROL JOINTS ARE SHOWN ON ARCHITECTURAL DRAWINGS. CONTROL JOINTS SHALL NOT OCCUR WITHIN 2'-8" OF WALL OPENINGS. REFER TO THE NATIONAL CONCRETE MASONRY ASSOCIATION TEK 10-2C "CONTROL JOINTS FOR CONCRETE MASONRY WALLS - EMPIRICAL METHOD" FOR RECOMMENDED CONTROL JOINT SPACING AND LOCATIONS.
4. ADDITIONAL PREFORMED GASKETS OR JOINT FILLINGS REQUIRED FOR FIRE RATED WALLS. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ADDITIONAL REQUIREMENTS.

12 TYPICAL CMU CONTROL JOINT

STRAIGHT BAR DEVELOPMENT & SPLICE

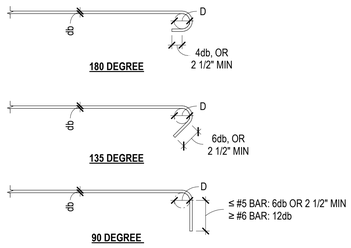
SPLICE AT HOOKED REINFORCEMENT

Lcmu, f'm = 1,900	
BAR SIZE	LENGTH (IN)
#3	17
#4	30
#5	45
#6	54
#7	63
#8	72
#9	82

STRAIGHT REINFORCEMENT DEVELOPMENT & LAP LENGTH

Le, f'm = 1,900	
BAR SIZE	LENGTH (IN)
#3	5
#4	7
#5	9
#6	10
#7	12
#8	13
#9	15

HOOKED REINFORCEMENT EQUIVALENT EMBEDMENT LENGTH

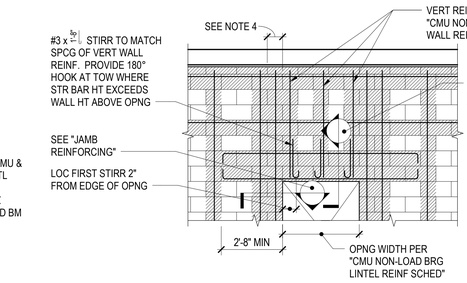


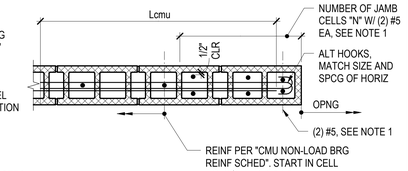
HOOK DEVELOPMENT

NOTES:

1. FOR EPOXY COATED BARS, INCREASE THE ABOVE EMBEDMENT LENGTHS BY 20 PERCENT.
2. FULL DEVELOPMENT LENGTH FOR HOOKED REINFORCING BAR IS Lcmu - Le.

17 CMU REINFORCING DEVELOPMENT AND SPLICE LENGTH TABLES

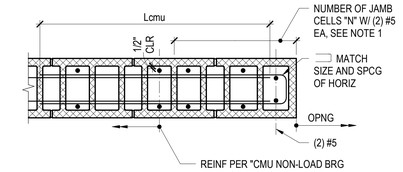




NOTES:

1. AT WALL HEIGHTS NOT EXCEEDING 13'-0", PROVIDE ONE #5 VERTICAL JAMB BAR, LOCATION TYPE A PER "TYPICAL CMU NON-LOAD BERING WALL ELEVATION, SCHEDULE AND NOTES", IN PLACE OF TWO #5 BARS INDICATED.

8" MASONRY WALL



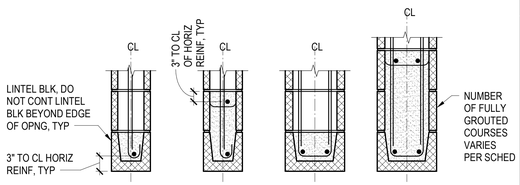
12" MASONRY WALL

JAMB REINFORCING

CMU NON-LOAD BEARING LINTEL REINFORCEMENT SCHEDULE						
NOMINAL WALL THICKNESS (IN)	MAX OPENING WIDTH (FEET)	BOTTOM HORIZONTAL REINFORCING	TOP HORIZONTAL REINFORCING	NUMBER OF COURSES	LINTEL BAR LOCATION TYPE	NUMBER OF JAMB REINFORCING CELLS "N"
8"	4'-0"	(1) #5	-	1	A-1	2
8"	8'-0"	(1) #5	(1) #5	2	A-2	3
8"	12'-0"	(1) #5	(1) #5	3	A-2	4
12"	4'-0"	(2) #5	-	1	C-1	2
12"	8'-0"	(2) #5	(2) #5	2	C-2	3
12"	12'-0"	(2) #5	(2) #5	3	C-2	4

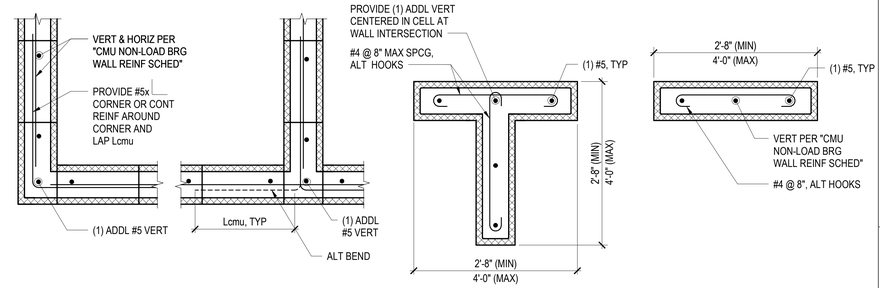
NOTES:

1. PROVIDE (1) COURSE BOND BEAM AT SILL CONDITIONS BELOW OPENINGS WITH (2) #5 HORIZONTALS. AT OPENINGS 10 FT WIDE OR GREATER, IF THE SILL IS MORE THAN 5 FT ABOVE THE SUPPORTING LEVEL, PROVIDE (2) COURSE BOND BEAM WITH (2) #5 HORIZONTALS IN EACH COURSE.
2. SEE "TYPICAL CMU NON-LOAD BEARING WALL ELEVATION, SCHEDULE, AND NOTES FOR MORE INFORMATION.
3. CONTACT ENGINEER IF HEIGHT OF WALL ABOVE OPENING IS LESS THAN NUMBER OF COURSES CALLED FOR IN LINTEL REINFORCEMENT SCHEDULE.
4. PROVIDE TOP OF WALL CONNECTION PER "TYP CMU NON-LOAD BEARING WALL TOP CONNECTIONS" WITHIN 8 INCHES OF OPENING. FOR WALLS GREATER THAN 18'-0" TALL WITH OPENINGS GREATER THAN 4'-0", PROVIDE TWO TOP OF WALL CONNECTIONS WITHIN JAMB REINFORCEMENT WIDTH.
5. CONTRACTOR TO COORDINATE AND SUBMIT CMU WALL ELEVATIONS SHOWING ALL OPENINGS TO BE BUILT IN A GIVEN WALL (ARCH AND MEP) FOR ENGINEER OF RECORD TO REVIEW PRIOR TO WALL CONSTRUCTION.



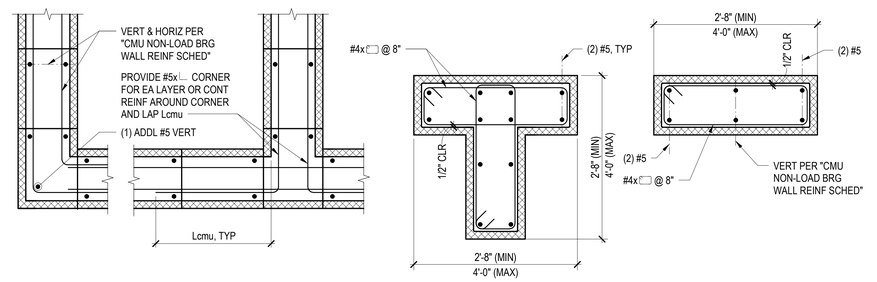
LINTEL BAR LOCATION TYPES

9 TYPICAL CMU NON-LOAD BEARING WALL LARGE OPENING LINTEL AND JAMB REINFORCING



CORNER INTERSECTION SHORT INTERSECTING WALLS SHORT WALL

8" MASONRY WALL



CORNER INTERSECTION SHORT INTERSECTING WALLS SHORT WALL

12" MASONRY WALL

NOTES:

1. BLOCK WEBS NOT SHOWN FOR CLARITY.
2. VERTICAL JAMB REINFORCEMENT WHERE REQUIRED BY "TYPICAL CMU NON-LOAD BEARING LINTEL ELEVATION, SCHEDULE AND NOTES" SHALL REPLACE TYPICAL "SHORT WALL" AND "SHORT INTERSECTING WALLS" VERTICAL REINFORCEMENT.

19 TYPICAL CMU NON-LOAD BEARING WALL REINFORCING AT CORNERS, INTERSECTIONS, AND SHORT WALLS

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project manager
drawn by
checked by
job no. 20052
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revisions:

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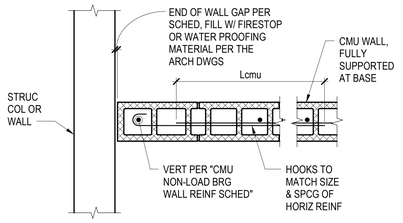
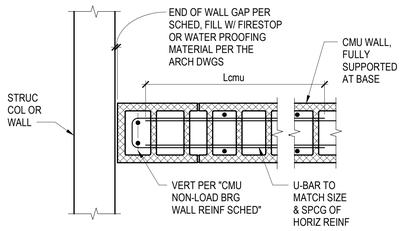
TYPICAL NON-LOAD BEARING CMU WALL DETAILS

S4.21



8" MASONRY WALL



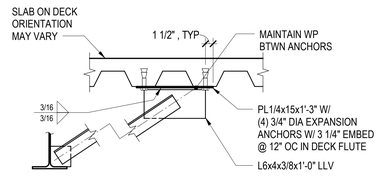
12" MASONRY WALL

END OF WALL ABUTTING TO STRUCTURE, GAP SCHEDULE	
WALL LOCATION	GAP
BELOW GRADE	1"
LEVEL 1	1 1/2"
LEVEL 1 AT AB CONNECTOR	2 1/2"

NOTES:

1. DETAIL IS NOT APPLICABLE WHERE OPENINGS ARE WITHIN 2'-8" OF THE END OF WALL. CONTRACTOR SHALL SUBMIT CONDITIONS WITH OPENINGS FOR ENGINEER TO REVIEW.
2. WHERE A GAP IS NOT SCHEDULED, PROVIDE A GAP AND FLEXIBLE FILLER MATERIAL CAPABLE OF ACCOMMODATING BUILDING STORY DRIFTS AS NOTED IN THE "GENERAL NOTES".

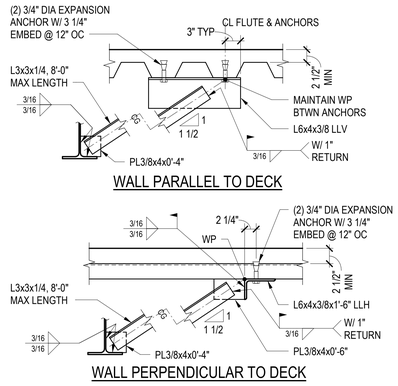
6 TYPICAL NON-LOAD BEARING CMU WALL ABUTTING STRUCTURAL ELEMENTS



NOTES:

1. SEE WALL PARALLEL TO DECK CONDITION FOR ADDITIONAL INFORMATION.

BRACE AT EXTERIOR WALL



2 TYPICAL CMU BRACE TO UNDERSIDE OF CONCRETE ON STEEL DECK

TOP OF WALL CONNECTION SPACING SCHEDULE			
MAXIMUM WALL THICKNESS	MAXIMUM WALL HEIGHT	TOP CONNECTION SPACING "S1"	TOP CONNECTION SPACING "S2"
8"	18'-0"	4'-0"	4'-0"
12"	13'-0"	4'-0"	4'-0"
12"	17'-0"	3'-0"	4'-0"
12"	22'-0"	2'-0"	3'-0"

NOTES:

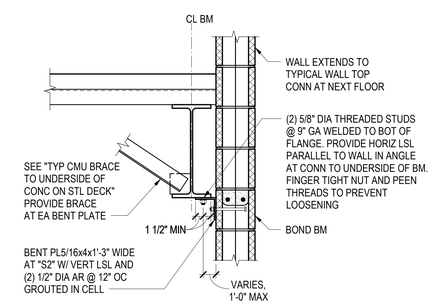
1. SPACING "S1" AND "S2" SHALL NOT EXCEED 4'-0".
2. SEE NOTE 4 OF "TYPICAL CMU NON-LOAD BEARING WALL LARGE OPENING LINTEL AND JAMB REINFORCING" DETAIL FOR TOP OF WALL CONNECTION REQUIREMENTS AT EDGES OF LARGE OPENINGS.

4 TOP OF WALL CONNECTION SPACING SCHEDULE

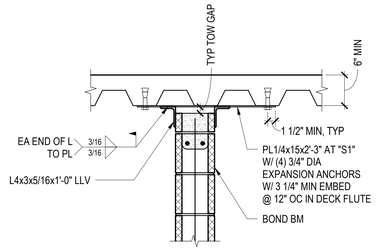
NOTES:

1. SEE "TYPICAL CMU NON-LOAD BEARING WALL ELEVATION, SCHEDULE AND NOTES" FOR TYPICAL TOP OF WALL GAP.
2. WHERE FIREPROOF, ACOUSTIC, OR OTHER FILLER IS REQUIRED PER ARCHITECTURE, PROVIDE FILLER WITH MINIMUM 50 PERCENT COMPRESSIBILITY.
3. FIREPROOFING REMOVED OR DAMAGED BY INSTALLATION OF TOP OF WALL CONNECTIONS SHALL BE REPLACED.
4. SEE "TOP OF WALL CONNECTION SPACING SCHEDULE" FOR SPACING REQUIREMENTS "S1" AND "S2".
5. TYPICAL EXPANSION ANCHOR SHALL BE HILTI KWIK BOLT TZ OR APPROVED EQUIVALENT.
6. AT CONTRACTORS OPTION, ALTERNATE TOP OF WALL CONNECTIONS PROVIDING EQUIVALENT OUT OF PLANE SUPPORT AND ALLOWING VERTICAL AND IN PLANE MOVEMENT OF THE STRUCTURE RELATIVE TO THE WALL MAY BE SUBSTITUTED. THE CONTRACTOR SHALL SUBMIT ALL PROPOSED ALTERNATE TYPICAL DETAILS WITH RELATED CALCULATIONS TO THE ENGINEER FOR APPROVAL PRIOR TO SHOP DRAWING PRODUCTION AND FIELD USE.

5 TOP OF WALL CONN GENERAL NOTES



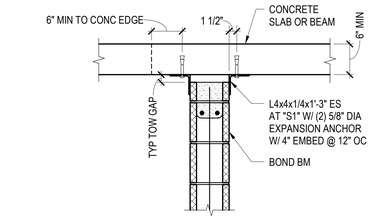
CONTINUOUS WALL PARALLEL TO WF BEAM



NOTES:

1. EXPANSION ANCHORS TO BE LOCATED MINIMUM 6" FROM EDGE OF CONCRETE IN ANY DIRECTION.

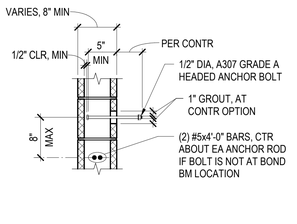
STEEL DECK PARALLEL TO WALL



NOTES:

1. THIS DETAIL IS NOT TO BE USED AT POST-TENSIONED SLABS OR AT POST-TENSIONED BEAMS NEAR TENDON LOW POINTS.

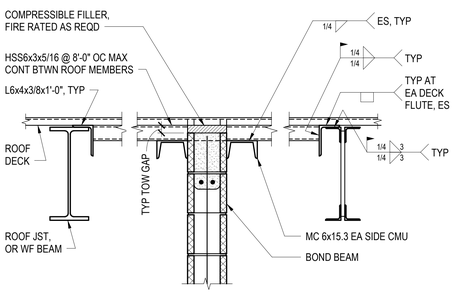
WALL AT CONCRETE SLAB OR BEAM



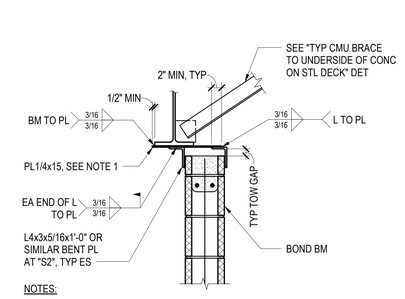
NOTES:

1. MAKE BAR CONTINUOUS WHERE BOLTS ARE SPACEDAT 24" OR LESS. EXTEND 1'-0" PAST LAST BOLT.

11 TYPICAL ANCHOR BOLT AT CMU WALL



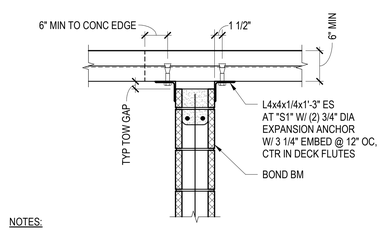
WALL PERPENDICULAR TO STEEL ROOF DECK



NOTES:

1. AT CONTRACTORS OPTION, PROVIDE CONTINUOUS BENT PLATE OVER FAR EDGE OF WALL SIMILAR TO "WALL AT CONCRETE SLAB EDGE"

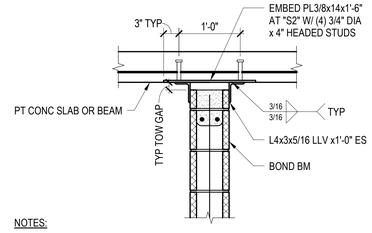
WALL OFFSET FROM STEEL BEAM



NOTES:

1. AT EXTERIOR WALLS, PROVIDE CONNECTION SIMILAR TO "STEEL DECK AT PARALLEL WALL" WITH PL1/4 AND (4) 3/4" DIA EXPANSION ANCHORS.

STEEL DECK PERPENDICULAR TO WALL

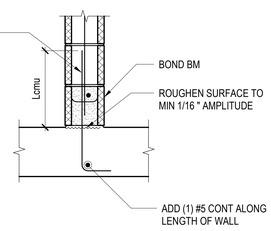


NOTES:

1. AT CONTRACTORS OPTION, THIS DETAIL CAN BE USED AT MILD CONCRETE SLABS OR BEAMS.

WALL AT POST-TENSIONED CONCRETE SLAB OR BEAM

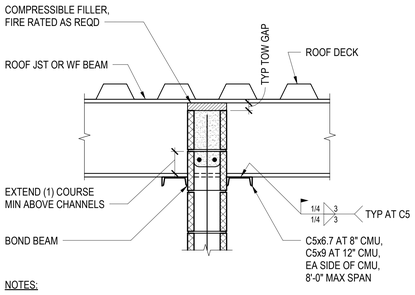
DOWEL TO MATCH WALL REINF, SPACING PER "NON-LOAD BRG WALL REINF SCHED". PROVIDE STD HOOK, ALTERNATE BENDS



NOTES:

1. DOWEL SPACING TO MATCH VERTICAL REINFORCING.

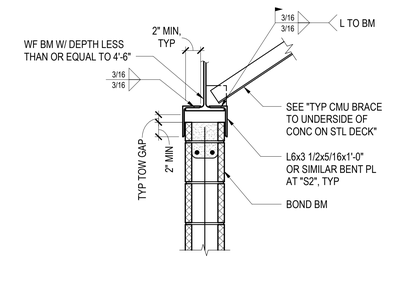
16 TYPICAL CMU NON-LOAD BEARING WALL BOTTOM CONNECTION



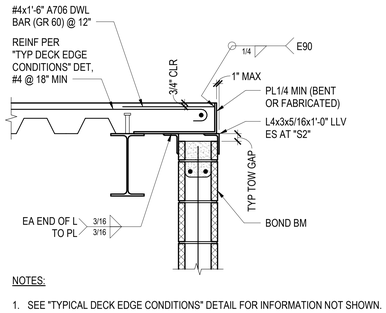
NOTES:

1. SEE "TYPICAL CMU NON-LOAD BEARING WALL OPENING AROUND INCOMING BEAM".

WALL PARALLEL TO STEEL ROOF DECK



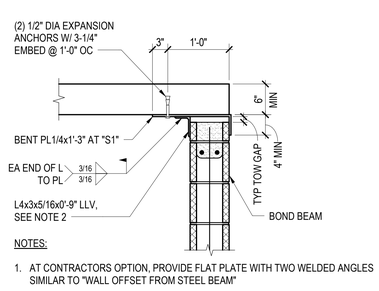
WALL CENTERED ON STEEL BEAM



NOTES:

1. SEE "TYPICAL DECK EDGE CONDITIONS" DETAIL FOR INFORMATION NOT SHOWN.
2. THIS DETAIL IS NOT TO BE USED WITH GAUGE METAL EDGE PLATES.

WALL AT UNDERSIDE OF CONCRETE ON STEEL DECK EDGE



NOTES:

1. AT CONTRACTORS OPTION, PROVIDE FLAT PLATE WITH TWO WELDED ANGLES SIMILAR TO "WALL OFFSET FROM STEEL BEAM"
2. AT NOMINAL WALL THICKNESS GREATER THAN 8", CENTER ANGLE BETWEEN EXPANSION ANCHORS.

WALL AT CONCRETE SLAB EDGE

17 TYPICAL CMU NON-LOAD BEARING WALL TOP CONNECTIONS

Reserved for permit stamp

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date 7/15/2022

revisions:

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TYPICAL NON-LOAD BEARING CMU WALL DETAILS

S4.22

TOWER A - ROOF STEEL COLUMN SCHEDULE

	SCA12	SCA13	SCA14	SCA15	SCA16	SCA17	SCA18	SCA19	SCA20	SCA21	SCA22	SCA23	SCA24	SCA25	SCA26	SCA27	SCA28
TOWER A - ROOF																	
8419' - 6" TOWER B - LEVEL 5					HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16			HSS6x6x5/16
8412' - 0" TOWER A - LEVEL 6																	
8407' - 6" TOWER A - LEVEL 5	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x3/8	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	HSS6x6x5/16	HSS6x6x3/8	BASE PL 1x8x1'-0" 4/S4.11
8395' - 0"	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11											BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	
Column Locations	SCA12	SCA13	SCA14	SCA15	SCA16	SCA17	SCA18	SCA19	SCA20	SCA21	SCA22	SCA23	SCA24	SCA25	SCA26	SCA27	SCA28

	SCA29	SCA30	SCA31	SCA32	SCA33	SCA34	SCA35	SCA36	SCA37	SCA38	SCA39	SCA40	SCA41	SCA42	SCA43	SCA44	SCA45	
TOWER A - ROOF																		TOWER A - ROOF
8419' - 6" TOWER B - LEVEL 5	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x3/8	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	8419' - 6" TOWER B - LEVEL 5
8412' - 0" TOWER A - LEVEL 6																		8412' - 0" TOWER A - LEVEL 6
8407' - 6" TOWER A - LEVEL 5	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	8407' - 6" TOWER A - LEVEL 5
8395' - 0"																		8395' - 0"
Column Locations	SCA29	SCA30	SCA31	SCA32	SCA33	SCA34	SCA35	SCA36	SCA37	SCA38	SCA39	SCA40	SCA41	SCA42	SCA43	SCA44	SCA45	

TOWER A - ROOF STEEL COLUMN SCHEDULE

TOWER A - STEEL COLUMN SCHEDULE

	SCA1	SCA2	SCA3	SCA4	SCA5	SCA6	SCA7	SCA8	SCA9	SCA10	SCA11	
TOWER A - LEVEL 6												TOWER A - LEVEL 6
8407' - 6" TOWER A - LEVEL 5	HSS8x6x1/2	HSS8x6x1/2		HSS6x6x1/2			W10x49		W10x49	W10x49	W10x49	8407' - 6" TOWER A - LEVEL 5
8395' - 0" TOWER A - LEVEL 4	HSS8x6x1/2	HSS8x6x1/2	W10x77		W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	8395' - 0" TOWER A - LEVEL 4
8383' - 0" TOWER A - LEVEL 3	HSS8x6x5/8	HSS8x6x5/8	W10x88		W10x60	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	8383' - 0" TOWER A - LEVEL 3
8371' - 0" TOWER A - LEVEL 2	HSS12x6x5/8	HSS12x6x5/8	W12x120		W10x60	W10x49	W10x60	W10x49		W10x60	W10x49	8371' - 0" TOWER A - LEVEL 2
8359' - 0" TOWER A - LEVEL 1	HSS12x6x5/8	HSS12x6x5/8	W12x152		W10x77	W10x60	W10x77	W10x77	W10x100	W10x77	W10x77	8359' - 0" TOWER A - LEVEL 1
8345' - 0" PARKING LEVEL 2					W10x88						W10x77	8345' - 0" PARKING LEVEL 2
8333' - 0"												8333' - 0"
Column Locations	SCA1	SCA2	SCA3	SCA4	SCA5	SCA6	SCA7	SCA8	SCA9	SCA10	SCA11	

TOWER A - STEEL COLUMN SCHEDULE

TOWER AB CONNECTOR - STEEL COLUMN SCHEDULE

	SC31	SC1	SC32	SC2	SC3	SC4	SC5	SC6	SC7	SC8	SC9	SC10	SC11	SC12	SC13	SC14	SC15
TOWER A - LEVEL 2																	
8359' - 0" TOWER A - LEVEL 1	HSS6x6x1/2	HSS8x8x5/8	W8x40	HSS6x6x1/2	W8x40	HSS6x6x1/2	W8x40	HSS6x6x1/2	HSS6x6x1/2	HSS6x6x1/2	HSS6x6x1/2	W12x152	HSS6x6x1/2	W12x152	W12x152	W12x152	HSS8x8x5/8
8345' - 0"	BASE PL 2 1/2x18x1'-6" 18/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x18x1'-6" 19/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x18x1'-6" 19/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x18x1'-6" 19/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 2 1/2x18x1'-6" 18/S6.00
Column Locations	SC31	SC1	SC32	SC2	SC3	SC4	SC5	SC6	SC7	SC8	SC9	SC10	SC11	SC12	SC13	SC14	SC15

	SC16	SC17	SC18	SC19	SC20	SC21	SC22	SC23	SC24	SC25	SC26	SC27	SC28	SC28	SC29	SC30	
TOWER A - LEVEL 2																	TOWER A - LEVEL 2
8359' - 0" TOWER A - LEVEL 1	HSS6x6x1/2	HSS6x6x5/8	HSS6x6x5/8	HSS6x6x5/8	HSS6x6x5/8	HSS6x6x1/2	W12x152	HSS6x6x1/2	W12x152	HSS8x8x5/8	W12x152	HSS8x8x5/8	HSS6x6x1/2	HSS8x8x5/8	HSS8x8x5/8	W12x152	8359' - 0" TOWER A - LEVEL 1
8345' - 0"	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 1 1/2x16x1'-4" 17/S6.00	BASE PL 2 1/2x18x1'-6" 18/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 3x22x2'-0" 20/S6.00	BASE PL 2 1/2x18x1'-6" 18/S6.00	BASE PL 2 1/2x18x1'-6" 18/S6.00	BASE PL 2 1/2x18x1'-6" 18/S6.00	BASE PL 2 1/2x18x1'-6" 18/S6.00	BASE PL 3x22x2'-0" 20/S6.00	8345' - 0"
Column Locations	SC16	SC17	SC18	SC19	SC20	SC21	SC22	SC23	SC24	SC25	SC26	SC27	SC28	SC28	SC29	SC30	

TOWER AB CONNECTOR - STEEL COLUMN SCHEDULE

Reserved for permit stamp

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TYPICAL STEEL
COLUMN
SCHEDULE

S4.A.10

TOWER B - ROOF STEEL COLUMN SCHEDULE

TOWER B - ROOF STEEL COLUMN SCHEDULE												
TOWER B - ROOF												
8448' - 6" TOWER B - LEVEL 7	HSS7x5x1/2	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x3/8	HSS6x6x3/8	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x3/8	HSS6x6x5/16
8436' - 6"		BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11
Column Locations	SCB1	SCB11	SCB12	SCB13	SCB14	SCB15	SCB16	SCB17	SCB18	SCB19	SCB20	SCB21

TOWER B - ROOF												
8448' - 6" TOWER B - LEVEL 7	HSS6x6x1/2	HSS6x6x5/16	HSS6x6x1/2	HSS6x6x1/2	HSS6x6x3/8	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x1/2	HSS6x6x5/16
8436' - 6"	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11
Column Locations	SCB22	SCB23	SCB24	SCB25	SCB26	SCB27	SCB28	SCB29	SCB30	SCB31	SCB32	SCB33

TOWER B - ROOF													TOWER B - ROOF
8448' - 6" TOWER B - LEVEL 7	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	8448' - 6" TOWER B - LEVEL 7
8436' - 6"	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	8436' - 6"
Column Locations	SCB34	SCB35	SCB36	SCB37	SCB38	SCB39	SCB40	SCB41	SCB42	SCB43	SCB44	SCB45	

TOWER B - STEEL COLUMN SCHEDULE

TOWER B - STEEL COLUMN SCHEDULE											
TOWER B - LEVEL 7											TOWER B - LEVEL 7
8436' - 6" TOWER B - LEVEL 6	W10x49	HSS8x6x1/2	HSS8x6x1/2	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x77	8436' - 6" TOWER B - LEVEL 6
8424' - 0" TOWER B - LEVEL 5	W10x49	HSS8x6x1/2	HSS8x6x1/2	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x112	8424' - 0" TOWER B - LEVEL 5
8412' - 0" TOWER B - LEVEL 4	W10x49	HSS12x6x1/2	HSS12x6x1/2	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W12x120	8412' - 0" TOWER B - LEVEL 4
8400' - 0" TOWER B - LEVEL 3	W10x60	HSS12x6x1/2	HSS12x6x1/2	W10x60	W10x49	W10x60	W10x49	W10x49	W10x60	W12x152	8400' - 0" TOWER B - LEVEL 3
8388' - 0" TOWER B - LEVEL 2	W10x77	HSS12x6x5/8	HSS12x6x5/8	W10x68	W10x60	W10x68	W10x49	W10x60	W10x77	W12x152	8388' - 0" TOWER B - LEVEL 2
8376' - 0" TOWER B - LEVEL 1	W12x136	HSS14x6x5/8	HSS14x6x5/8	W10x112	W10x77	W10x112	W10x60	W10x88	W10x112	W12x170	8376' - 0" TOWER B - LEVEL 1
8357' - 0" PARKING LEVEL 1		HSS14x6x5/8							W12x136		8357' - 0" PARKING LEVEL 1
8345' - 0" PARKING LEVEL 2									W12x152		8345' - 0" PARKING LEVEL 2
8333' - 0"											8333' - 0"
Column Locations	SCB1	SCB2	SCB3	SCB4	SCB5	SCB6	SCB7	SCB8	SCB9	SCB10	

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TYPICAL STEEL
COLUMN
SCHEDULE

S4.B.10

TOWER C - ROOF STEEL COLUMN SCHEDULE

Column Locations	SCC15	SCC1	SCC16	SCC17	SCC18	SCC19	SCC20	SCC21	SCC34	SCC22	SCC23	SCC24	SCC25	SCC26	SCC27	SCC28	SCC9	SCC29	SCC30	SCC31	SCC32	SCC33
ROOF 8475' - 0" to LEVEL 8	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x3/8	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x1/2	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x1/2	HSS6x6x1/2	HSS6x6x5/16	HSS6x6x3/8	HSS6x6x3/8	HSS6x6x3/8	HSS6x6x5/8	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x5/16	HSS6x6x1/2
LEVEL 8 8463' - 0"	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11

1

TOWER C - ROOF STEEL COLUMN SCHEDULE

TOWER C - STEEL COLUMN SCHEDULE

Column Locations	SCC1	SCC2	SCC3	SCC5	SCC6	SCC8	SCC9	SCC10	SCC11	SCC12	SCC13	SCC14
LEVEL 8 8463' - 0" to LEVEL 7	W10x77	W10x49	W10x49	HSS18x6x5/8	HSS8x6x1/2	HSS8x6x5/8	HSS12x6x5/8	W10x60	W10x49	W10x49	W10x60	W10x88
LEVEL 7 8450' - 6" to LEVEL 6	W10x112	W10x49	W10x49	HSS18x6x5/8	HSS8x6x1/2	HSS12x6x1/2	HSS18x6x5/8	W10x60	W10x49	W10x49	W10x60	W12x106
LEVEL 6 8438' - 6" to LEVEL 5	W10x112	W10x49	W10x49	HSS20x8x5/8 (1)	HSS12x6x1/2	HSS18x6x5/8	HSS14x6x5/8	W10x100	W10x49	W10x49	W10x100	W12x106
LEVEL 5 8426' - 6" to LEVEL 4	W12x136	W10x49	W10x49	HSS20x8x5/8	HSS12x6x1/2	HSS18x6x5/8	HSS18x6x5/8	W12x120	W10x49	W10x49	W12x106	W12x136
LEVEL 4 8414' - 6" to LEVEL 3	W12x136	W10x49	W10x49	W12x152	HSS18x6x5/8 (1)	HSS20x8x5/8 (1)	HSS18x6x5/8 (1)	W12x120	W10x49	W10x49	W12x136	W12x152
LEVEL 3 8402' - 6" to LEVEL 2	W12x170	W10x49	W10x49	W12x190	HSS20x8x5/8	W12x136	HSS20x8x5/8	W12x152		W10x60	W12x152	W12x152
LEVEL 2 8390' - 6" to LEVEL 1	W12x279	W10x100	W10x100	(1)				(1)	W10x100		W12x152	(1)
LEVEL 1 8376' - 6" to PARKING						(1)					W12x170	
PARKING 8364' - 6"	BASE PL 1 1/2x15x1'-7"; SEE 2/S4.11	BASE PL 3/4x12x1'-2"; SEE 2/S4.11	BASE PL 3/4x12x1'-2"; SEE 2/S4.11	BASE PL 1 1/2x15x1'-7"; SEE 2/S4.11	BASE PL 1 1/8x15x1'-10"; SEE 4/S4.11	BASE PL 1 1/2x14x1'-4"; SEE 2/S4.11	BASE PL 1 1/8x15x1'-10"; SEE 4/S4.11	BASE PL 1 1/2x14x1'-4"; SEE 2/S4.11	BASE PL 3/4x12x1'-2"; SEE 2/S4.11	BASE PL 3/4x12x1'-2"; SEE 2/S4.11	BASE PL 1 1/2x14x1'-4"; SEE 2/S4.11	BASE PL 1 1/2x14x1'-4"; SEE 2/S4.11

TOWER C - STEEL COLUMN SCHEDULE

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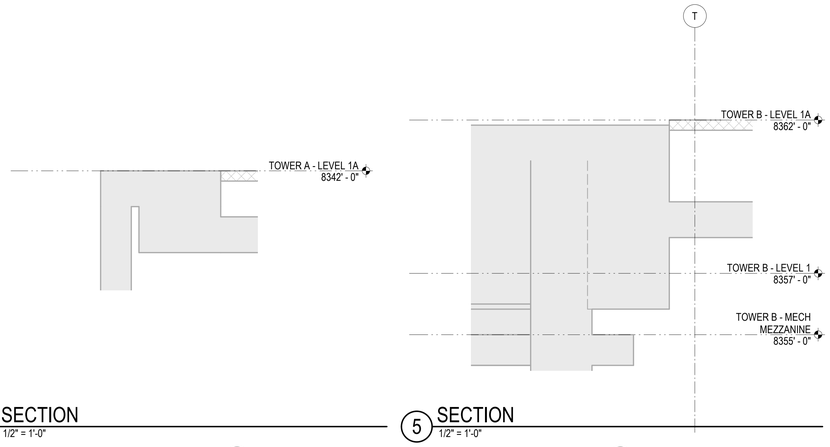
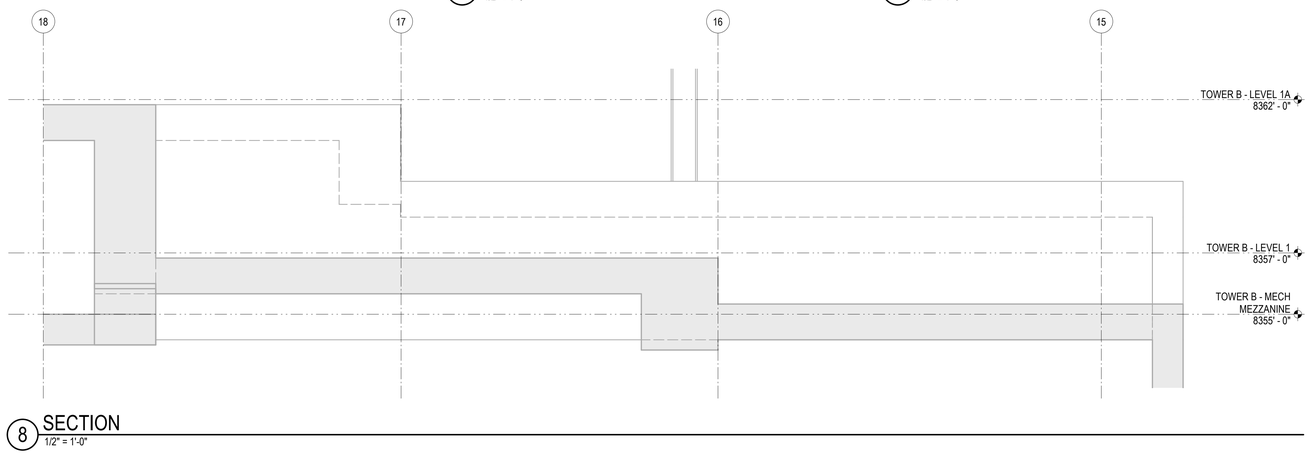
TOWER C STEEL COLUMN SCHEDULE

S4.C.10

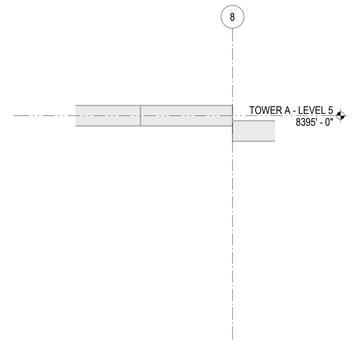


4 SECTION
1/2" = 1'-0"

5 SECTION
1/2" = 1'-0"



8 SECTION
1/2" = 1'-0"



20 SECTION
1/2" = 1'-0"

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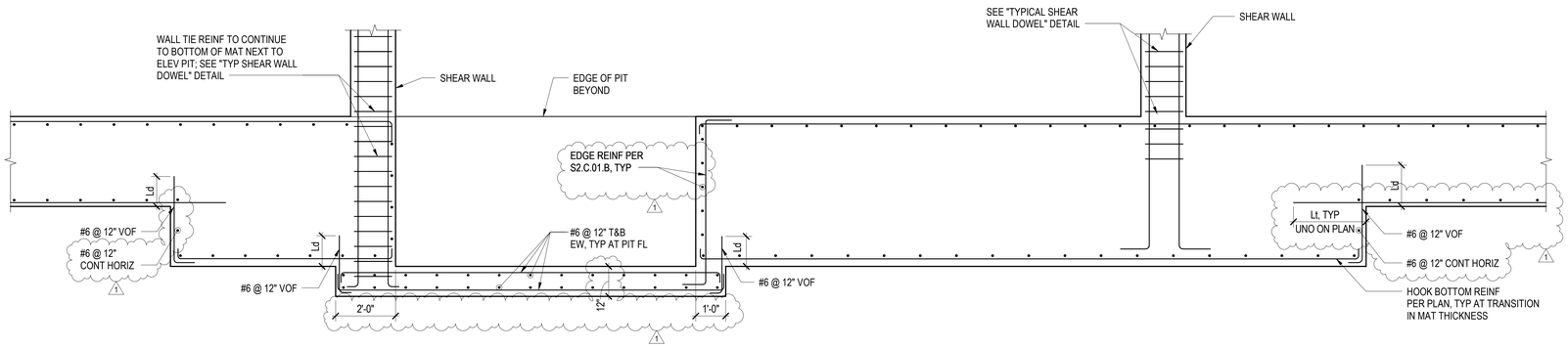
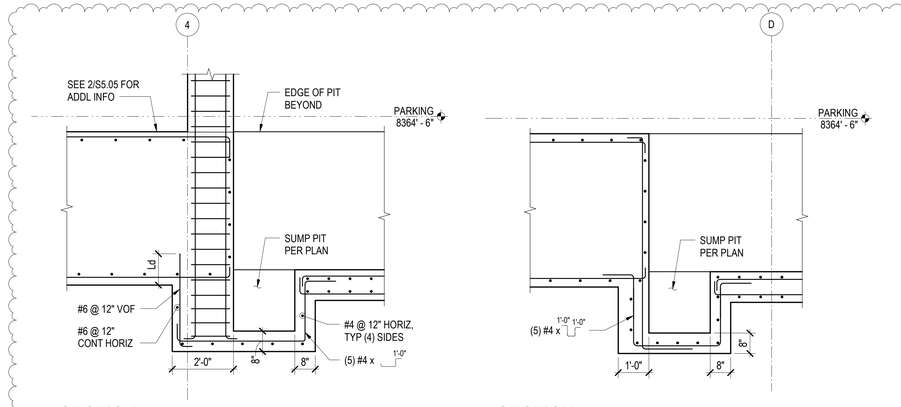
TOWER A & B
CONCRETE
SECTIONS AND
DETAILS

S5.00

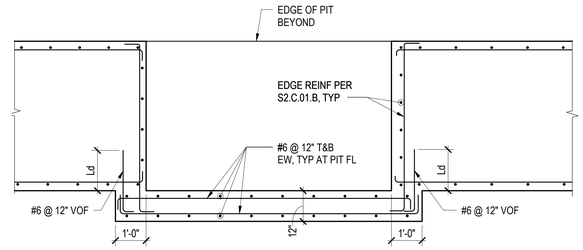


2 SECTION
1/2" = 1'-0"

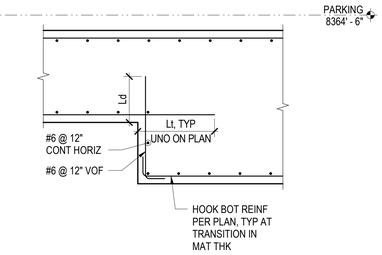


7 SECTION
1/2" = 1'-0"

8 SECTION
1/2" = 1'-0"



9 SECTION
1/2" = 1'-0"



14 SECTION
1/2" = 1'-0"

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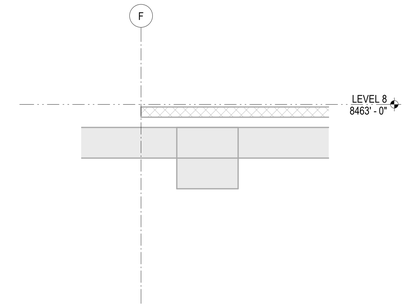
TOWER C
CONCRETE
SECTIONS AND
DETAILS

S5.05



5 SECTION
1/2" = 1'-0"

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TOWER C
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SECTIONS AND
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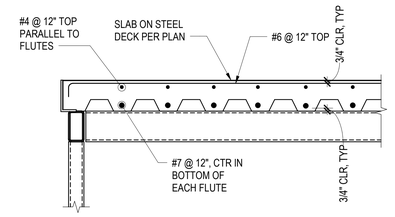
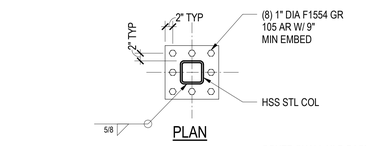


5 SECTION
3/4" = 1'-0"



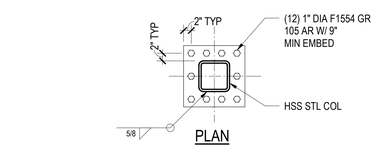
PLAN

ELEVATION

NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER " AISC MANUAL. TABLE 14-2, 1 UNLESS NOTED OTHERWISE.

17 BASE PLATE AT SMALL HSS IN CONNECTOR
3/4" = 1'-0"



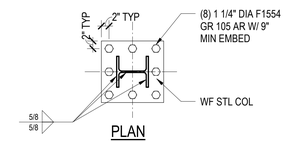
PLAN

ELEVATION

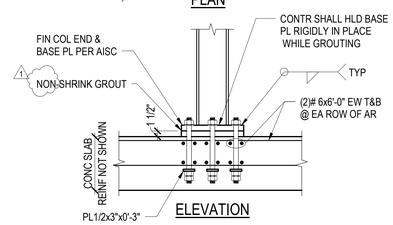
NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER " AISC MANUAL. TABLE 14-2, 1 UNLESS NOTED OTHERWISE.

18 BASE PLATE AT LARGE HSS IN CONNECTOR
3/4" = 1'-0"



PLAN

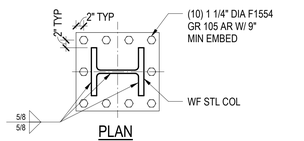


ELEVATION

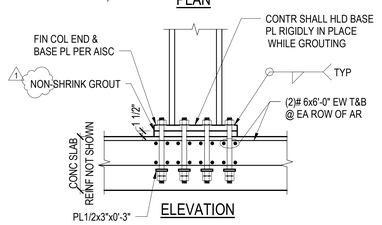
NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER " AISC MANUAL. TABLE 14-2, 1 UNLESS NOTED OTHERWISE.

19 BASE PLATE AT SMALL WF IN CONNECTOR
3/4" = 1'-0"



PLAN



ELEVATION

NOTES:

1. TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
2. BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER " AISC MANUAL. TABLE 14-2, 1 UNLESS NOTED OTHERWISE.
3. WHERE BAR LENGTH CANNOT BE ACHIEVED DUE TO SLAB EDGE, HOOK REINFORCING.

20 BASE PLATE AT LARGE WF IN CONNECTOR
3/4" = 1'-0"

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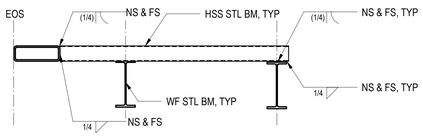
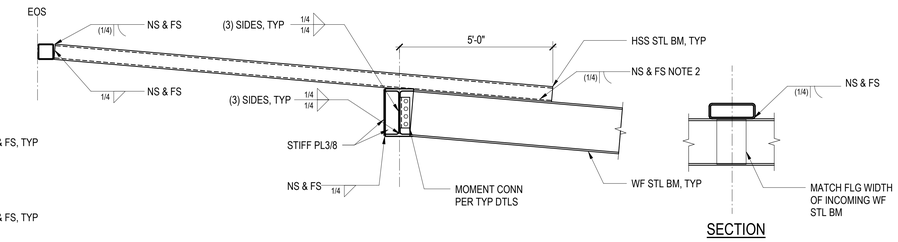
TOWER A & B
STEEL SECTIONS
AND DETAILS

S6.00



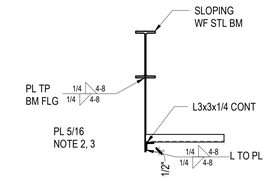
2 TYPICAL HSS PERPENDICULAR TO SLOPE
1/2" = 1'-0"



NOTES:

1. WHEN INCOMING WF STEEL BEAM CONNECTS TO WEB OF WF BEAM, ADD (2) STIFFENER PLATES AT EACH SIDE OF VERTICAL STIFFENER PLATE TO MATCH THE THICKNESS AND WIDTH OF THE INCOMING WF STEEL BEAM.
2. ORIENTATION OF WELD MAY VARY DEPENDING ON RELATIVE WIDTH OF THE HSS BEAM AND THE WF BEAM.

3 TYPICAL CANOPY HSS PARALLEL TO SLOPE
1/2" = 1'-0"



NOTES:

1. REFER TO CONNECTION TYPE C23 AT INCOMING BEAM.
2. PLATE HEIGHT WILL VARY ALONG THE LENGTH OF THE SLOPING WF STEEL BEAM.
3. CONTINUOUSLY BUTT WELD PLATE TO PLATES PER "TABLE E" WHEREVER THERE IS AN INCOMING BEAM.

5 TYPICAL CANOPY SLOPED BEAM
1/2" = 1'-0"

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TOWER C STEEL SECTIONS AND DETAILS

S6.05