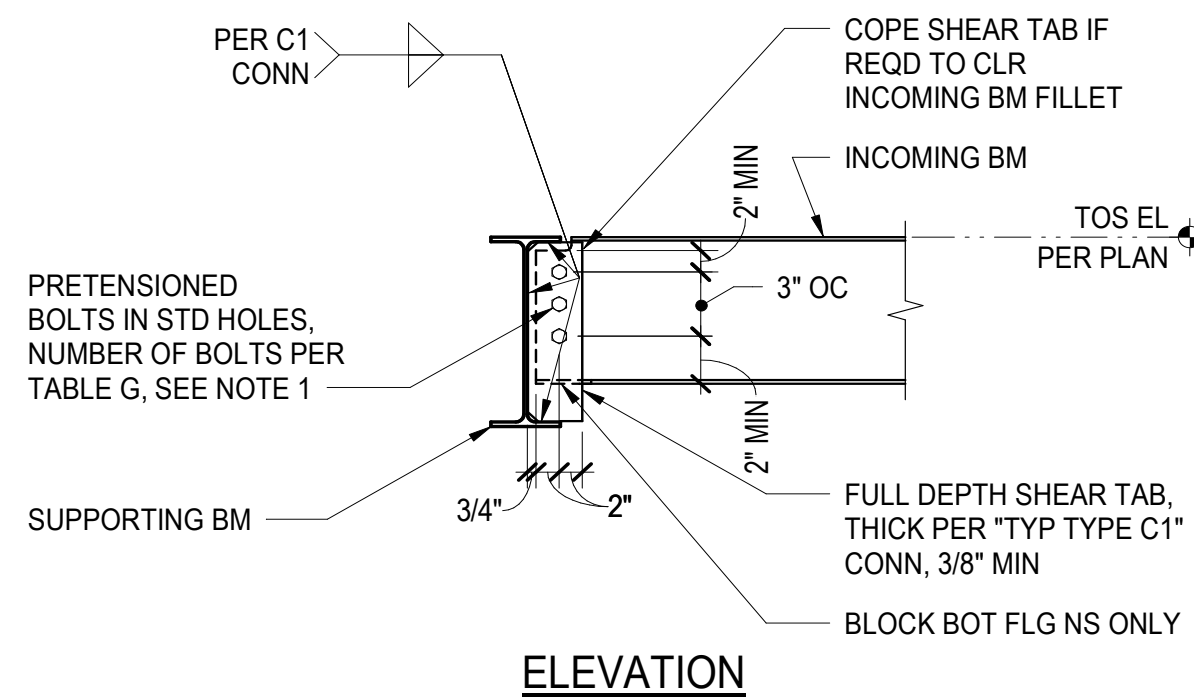
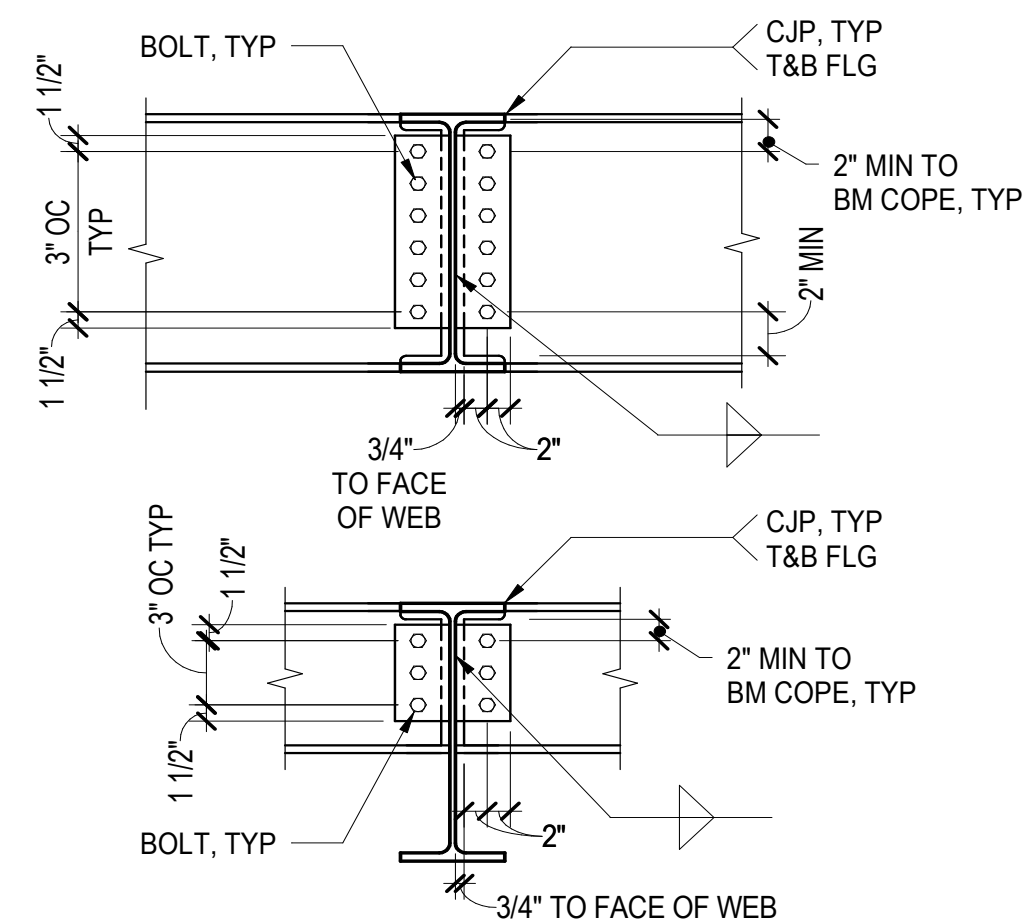




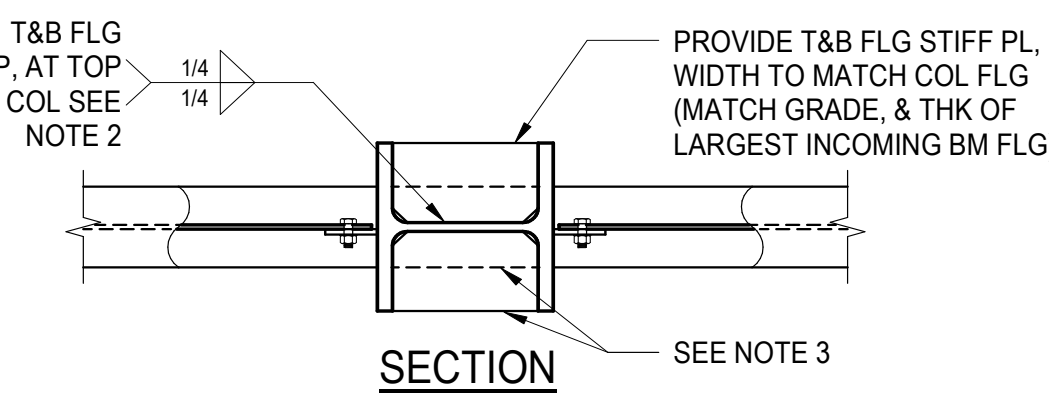
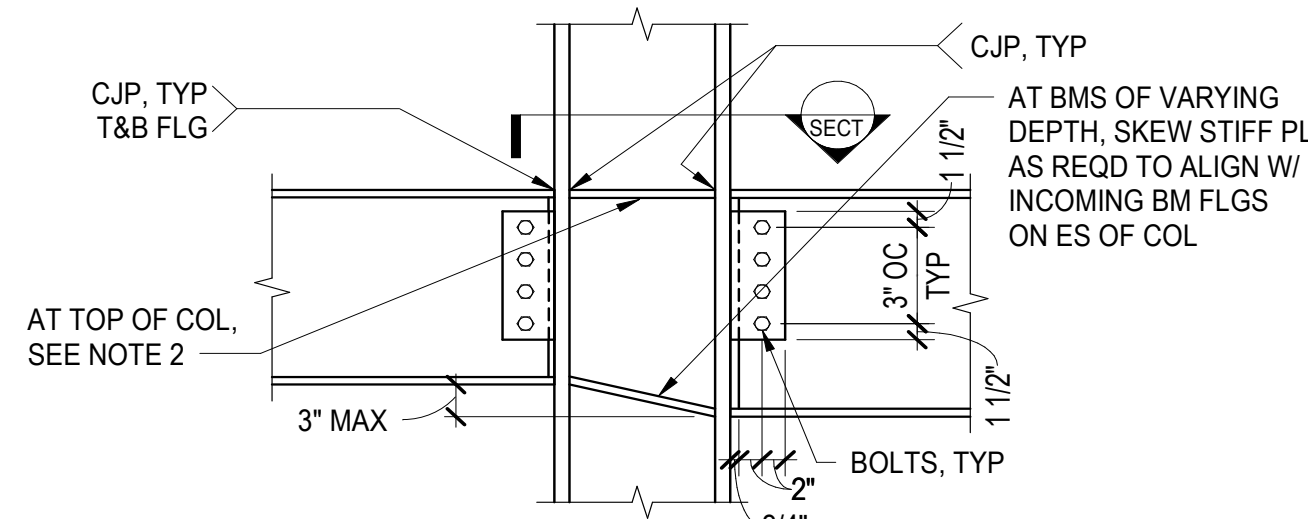
- NOTES:**
1. SEE "TYPICAL TYPE C1" FOR BOLT SIZE AND TYPE, WELD SIZE, PLATE THICKNESS, AND COPE REQUIREMENTS.
 2. THIS CONNECTION SHALL ONLY BE USED WHEN SPECIFICALLY CALLED OUT ON PLAN OR IN A DETAIL.
 3. BEAM SIZE IS SHALLOWER OF INCOMING AND SUPPORTING BEAM.

TABLE G	
BM SIZE (NOTE 3)	MINIMUM NUMBER OF BOLTS
W10	2
W12, W14	3
W16	4
W18	5
W21	6
W24	7
W27	8
W30	9
W33, W36	10
W40	11
W44	12



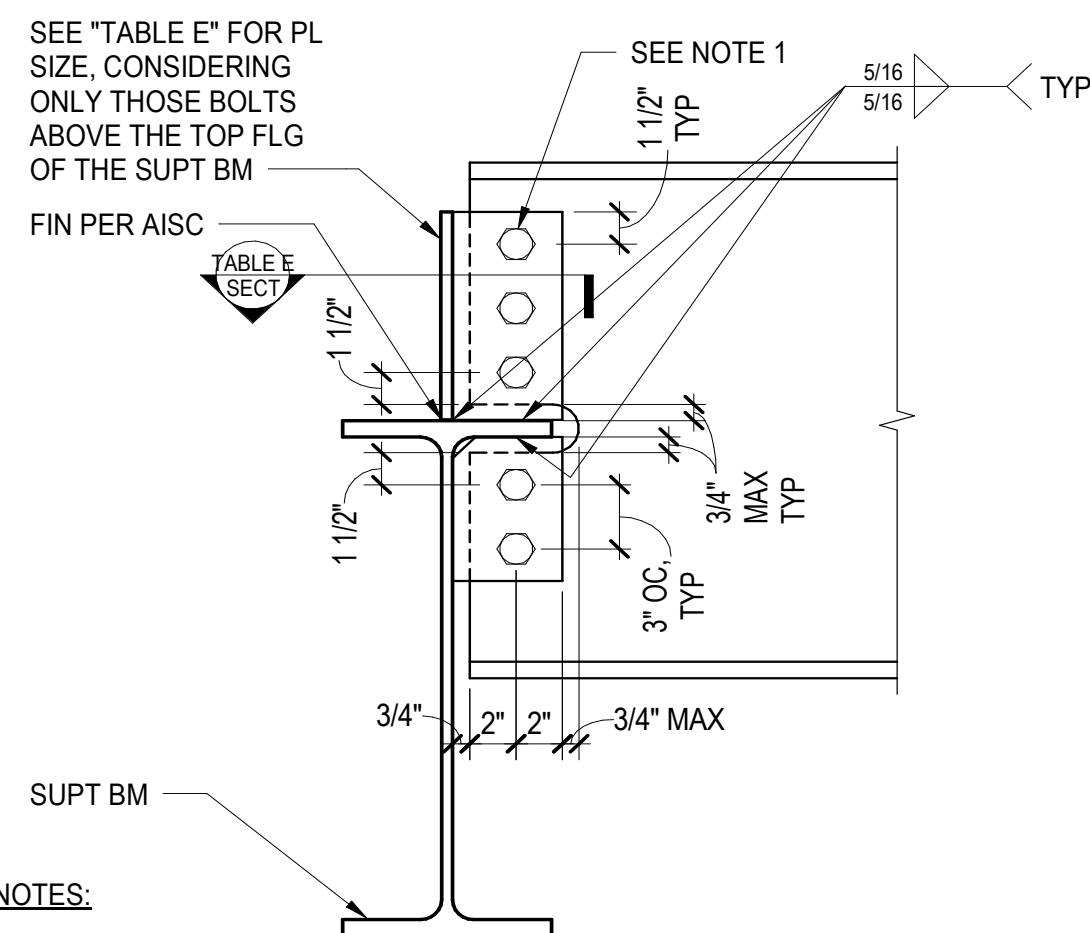
- NOTES:**
1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
 2. THIS DETAIL APPLIES ONLY FOR BEAMS OF EQUAL DEPTH EACH SIDE OF SUPPORTING BEAM.

5 TYP BEAM TO BEAM MOMENT CONNECTION



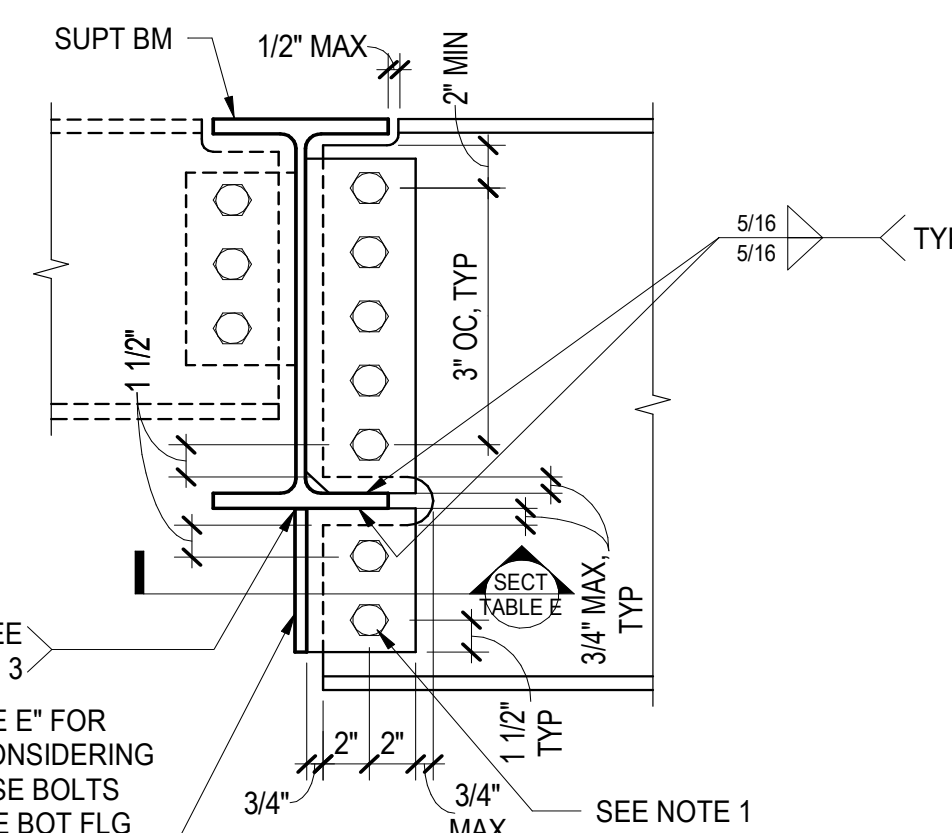
- NOTES:**
1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
 2. AT TOP OF COLUMN, PROVIDE 1/4" INCH R/P WELD IN LIEU OF TOP SIDE FILLET WELD OR, AT CONTRACTOR'S OPTION, A SINGLE CAP PLATE MAY BE USED IN LIEU OF TWO STIFFENER PLATES. USE 5/16" INCH ONE SIDED FILLET WELD FOR CAP PLATE TO EACH COLUMN FLANGE AND EACH SIDE OF COLUMN WEB.
 3. AT CONTRACTOR'S OPTION, WIDTH OF STIFFENER PLATES MAY BE REDUCED TO MATCH LARGEST INCOMING BEAM FLANGE IF NO OTHER INCOMING CONNECTIONS OCCUR AT A GIVEN BEAM-TO-COLUMN CONNECTION.

9 TYPICAL BM TO COL FLANGE MOMENT CONN



- NOTES:**
1. FOR A GIVEN BEAM AND REQUIRED REACTION, SEE "TABLE A" FOR BOLT SIZE AND TYPE, TOTAL NUMBER OF BOLTS, AND MAXIMUM COPE LENGTH.
 2. ALL PLATES SHALL HAVE $F_y = 50$ KSI MINIMUM.

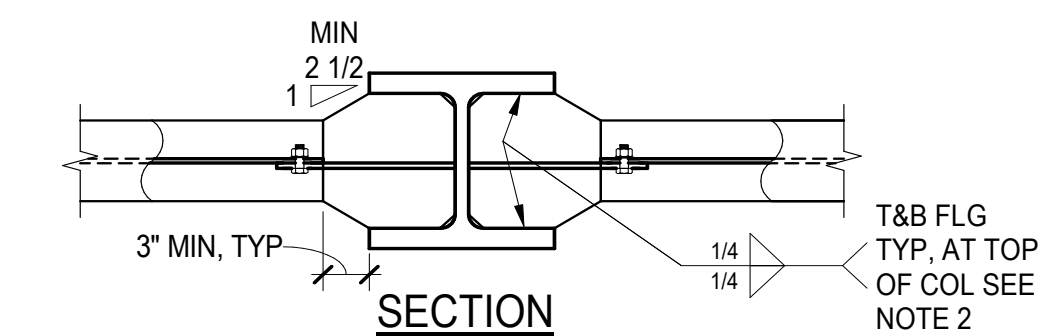
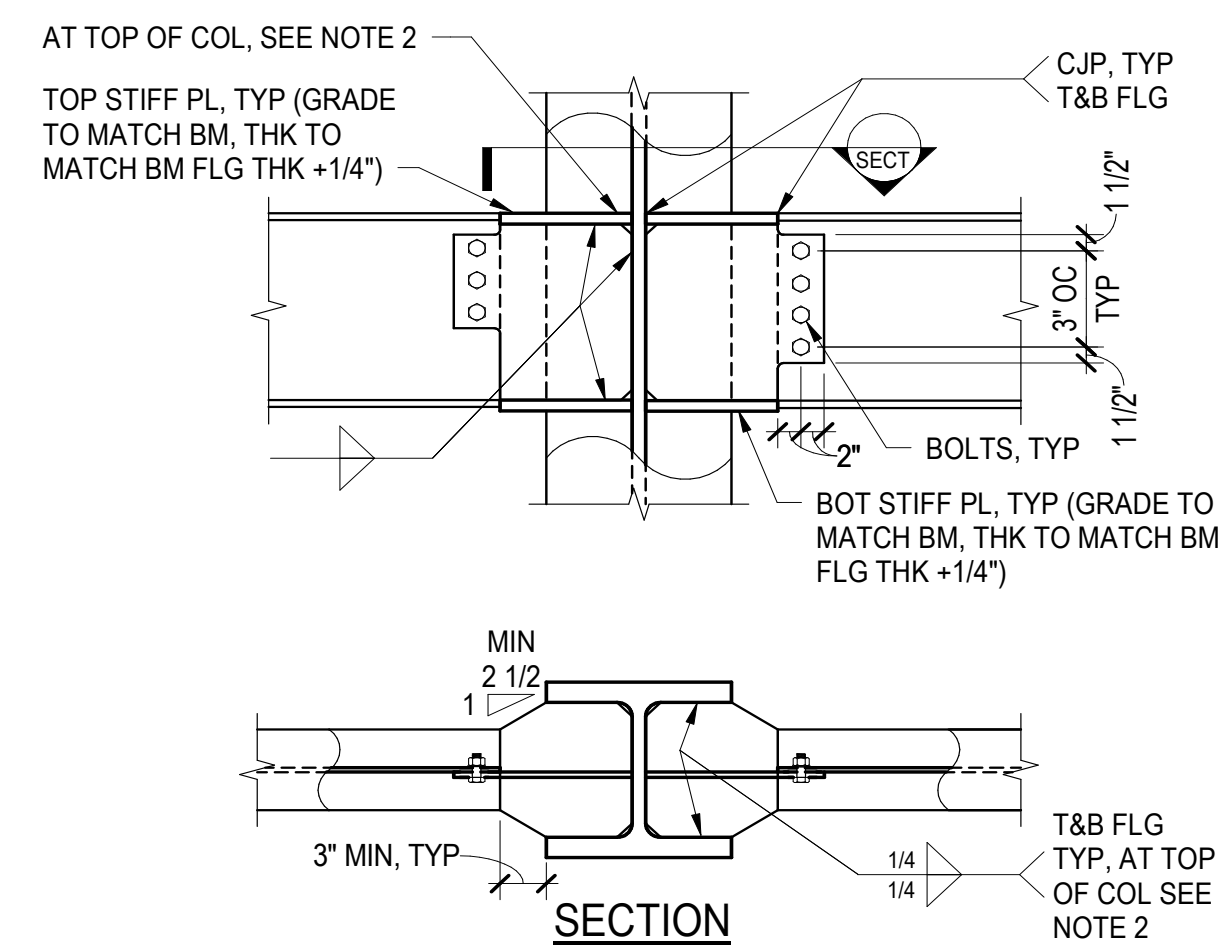
12 TYPICAL TYPE C21 - BEAM FLANGE INTERSECTING BEAM WEB



3. CONTRACTOR SHALL PERFORM ULTRASONIC TESTING AND INSPECTION OF SUPPORT BEAM BOTTOM FLANGE PER SPECIFICATION FOR WELDED CONNECTIONS.

- NOTES:**
1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
 2. AT TOP OF COLUMN, PROVIDE 1/4" INCH R/P WELD IN LIEU OF TOP SIDE FILLET WELD OR, AT CONTRACTOR'S OPTION, A SINGLE CAP PLATE MAY BE USED IN LIEU OF TWO STIFFENER PLATES. USE 5/16" INCH ONE SIDED FILLET WELD FOR CAP PLATE TO EACH COLUMN FLANGE AND EACH SIDE OF COLUMN WEB.
 3. THIS DETAIL APPLIES ONLY FOR BEAMS OF EQUAL DEPTH EACH SIDE OF COLUMN.

14 TYPICAL BEAM TO COLUMN WEB MOMENT CONNECTION



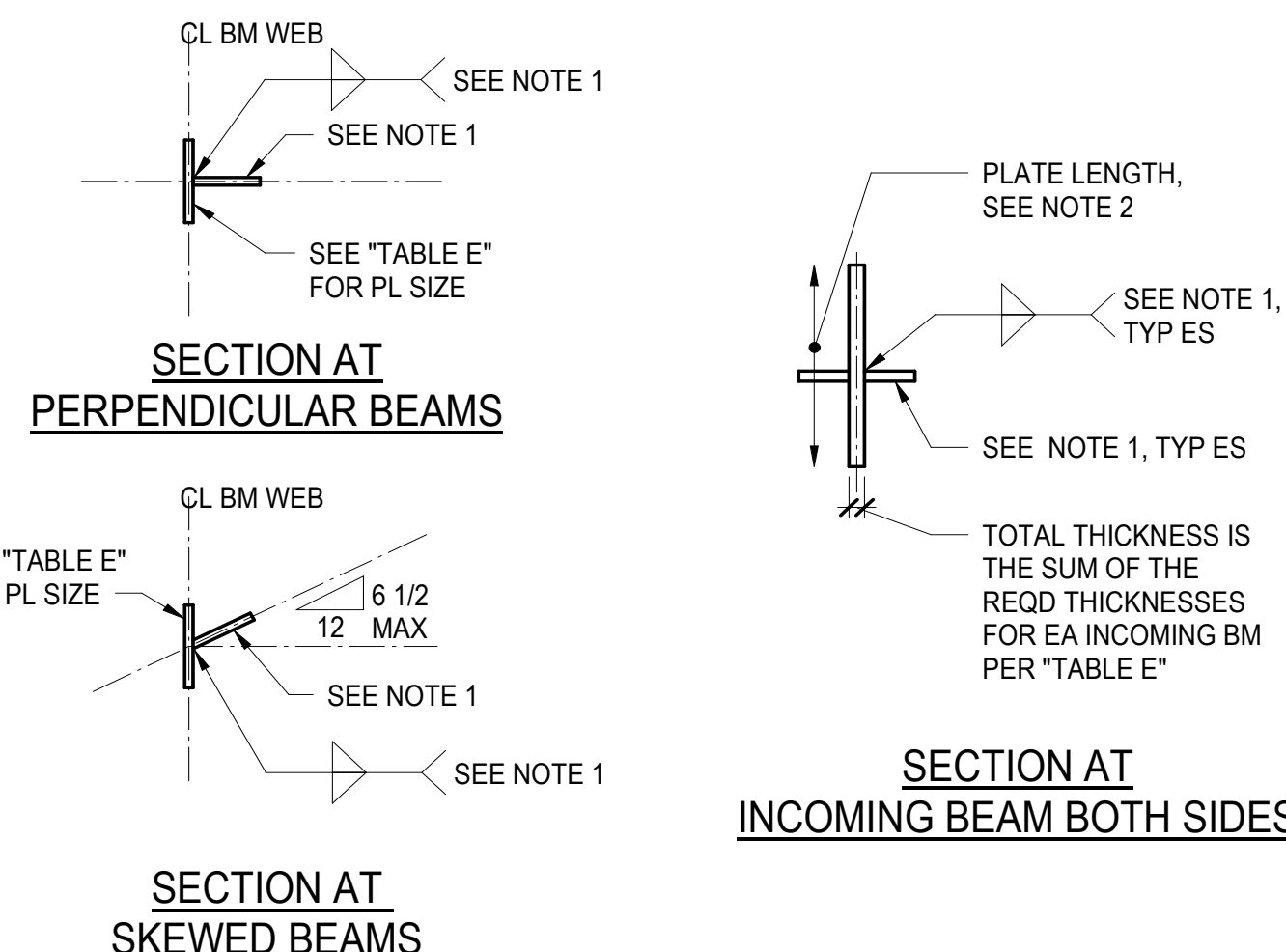
- NOTES:**
1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
 2. ALL PLATES SHALL HAVE $F_y = 50$ KSI MINIMUM.
 3. BEAMS MAY BE SKEWED UP TO 30 DEGREES.
 4. THIS DETAIL SHALL BE USED WITH W10, W12, AND W14 COLUMNS ONLY.
 5. WHEN DIMENSION SHOWN IS 6 INCHES OR LESS, EXTEND VERTICAL PLATE TO LOWER STIFFENER. WHEN DIMENSION SHOWN IS GREATER THAN 6 INCHES PROVIDE ADDITIONAL STIFFENER PLATE PL 1/2 AT BOTTOM OF VERTICAL PLATE.

19 TYPICAL BEAM TO COLUMN WEB SHEAR CONNECTION

TABLE E				
7/8" DIA GR A325 BOLTS				
NUMBER OF BOLTS	PL THICKNESS (IN)	PL LENGTH (IN)	NOMINAL WEB THICKNESS (IN)	STIFF PL THICKNESS (IN)
2-4	5/8	7	0.31	3/8
5-6	3/4	8	0.44	3/8
7-8	3/4	9	0.50	1/2
9-10	7/8	9	0.61	5/8
11-12	1	10	0.64	5/8
1" DIA GR A490 BOLTS				
NUMBER OF BOLTS	PL THICKNESS (IN)	PL LENGTH (IN)	NOMINAL WEB THICKNESS (IN)	STIFF PL THICKNESS (IN)
2-4	5/8	7	0.31	3/8
5-6	3/4	9	0.48	1/2
7-8	7/8	10	0.57	5/8
9-10	1	10	0.65	3/4
11-12	1	10	0.72	1

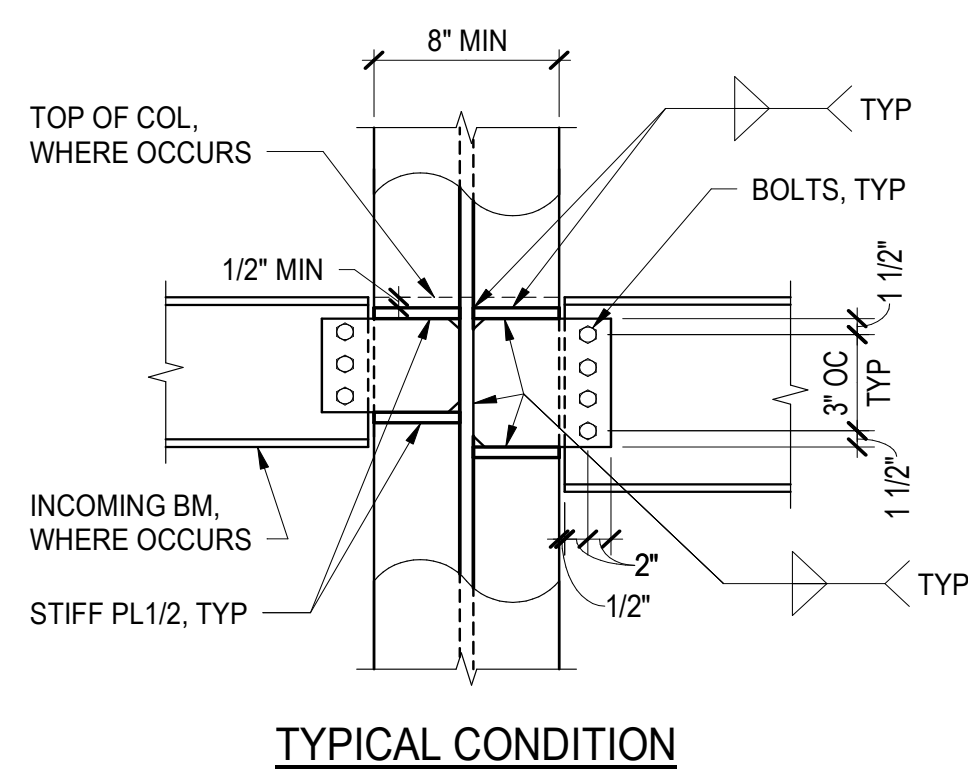
- NOTES:**
1. VERTICAL PLATE THICKNESS AND WELD SIZE SHALL BE PER "TABLE A" OR "TABLE B", WHICHEVER IS APPLICABLE.
 2. PLATE LENGTH SHALL BE DETERMINED PER "TABLE E" BY THE GREATEST NUMBER OF BOLTS REQUIRED FOR EITHER INCOMING BEAM CONNECTION.

16 TABLE E

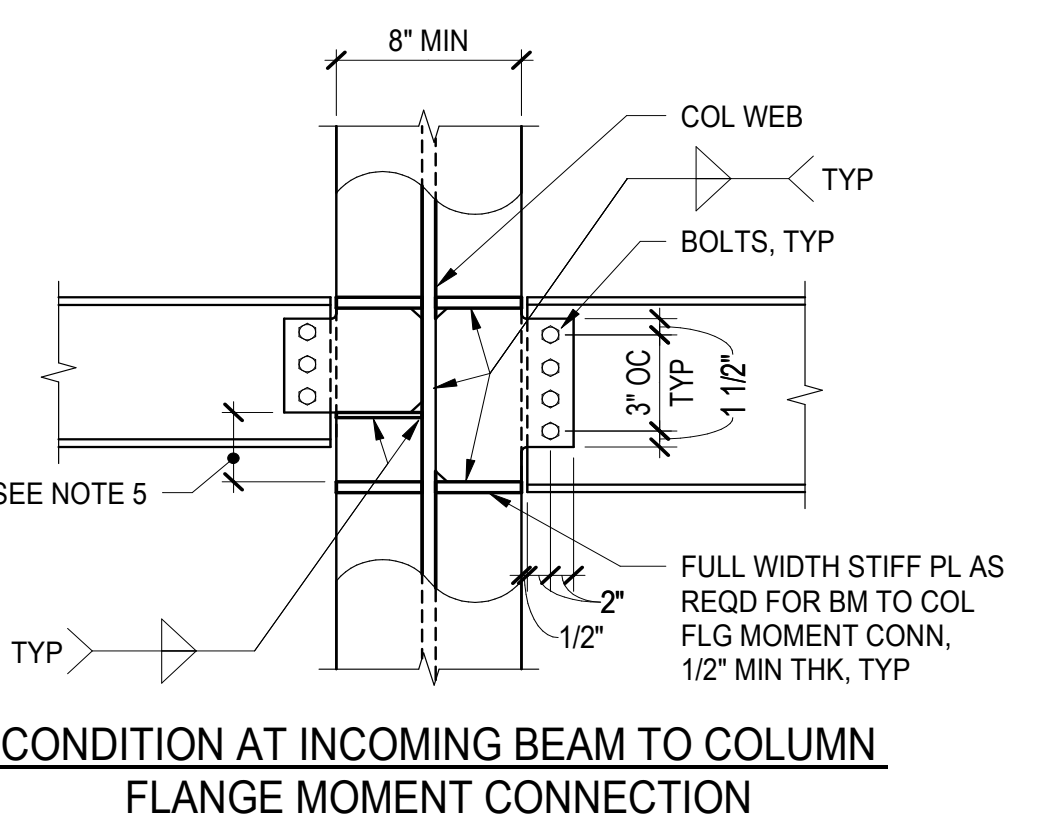


- NOTES:**
1. FOR A GIVEN BEAM AND REQUIRED REACTION, SEE "TABLE A" FOR BOLT SIZE AND TYPE, NUMBER OF BOLTS, AND MAXIMUM COPE LENGTH.
 2. ALL PLATES SHALL HAVE $F_y = 50$ KSI MINIMUM.
 3. PROVIDE FILLET WELD AT BUILT-UP BEAMS. WELD SIZE "W" SHALL BE $0.75t_w$, WHERE "tw" IS THE SUPPORT BEAM WEB THICKNESS.
 4. CONTRACTOR SHALL PERFORM ULTRASONIC TESTING AND INSPECTION OF SUPPORT BEAM BOTTOM FLANGE PER SPECIFICATION FOR WELDED CONNECTIONS.

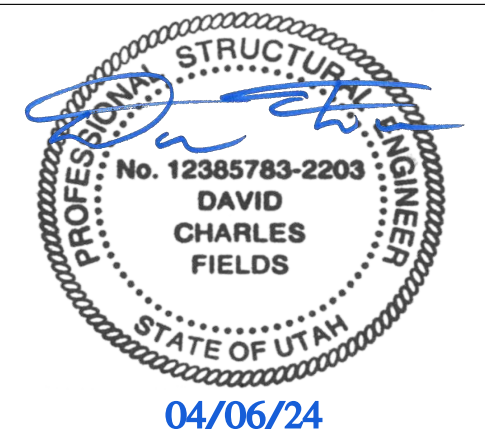
18 TYP TYPE C23 - BEAM TO DEPRESSED BM CONN



- NOTES:**
1. SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
 2. ALL PLATES SHALL HAVE $F_y = 50$ KSI MINIMUM.
 3. BEAMS MAY BE SKEWED UP TO 30 DEGREES.
 4. THIS DETAIL SHALL BE USED WITH W10, W12, AND W14 COLUMNS ONLY.



CONDITION AT INCOMING BEAM TO COLUMN FLANGE MOMENT CONNECTION



Reserved for permit stamp

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DEER VALLEY, UTAH

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principal architect _____
project manager _____
drawn by _____
checked by _____
job no. 20052
date 04/08/2024

revisions:

2	04/08/2024	IFC SET 1 OF 3
1	11/18/2022	95% CD
no.	date	by

ISSUED FOR CONSTRUCTION
SET 1 OF 3

04/08/2024

TYPICAL STEEL DETAILS

S4.12