

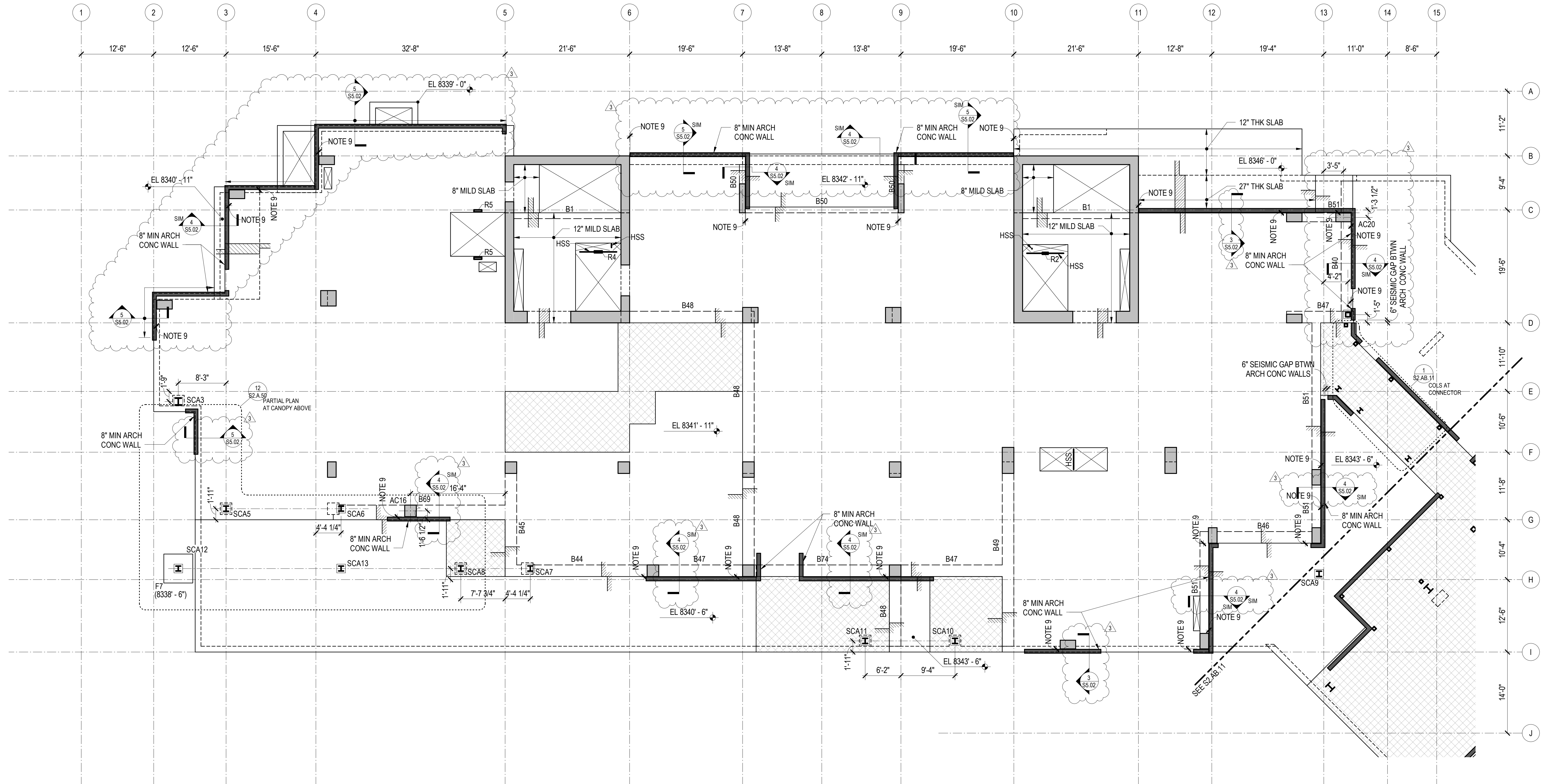
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**Olson Kundig**

project  
**SOMMET BLANC - ABC**  
DEER VALLEY, UTAH

**MAGNUSSON KLEMENCIC ASSOCIATES**  
Structural + Civil Engineers  
Seattle, Chicago  
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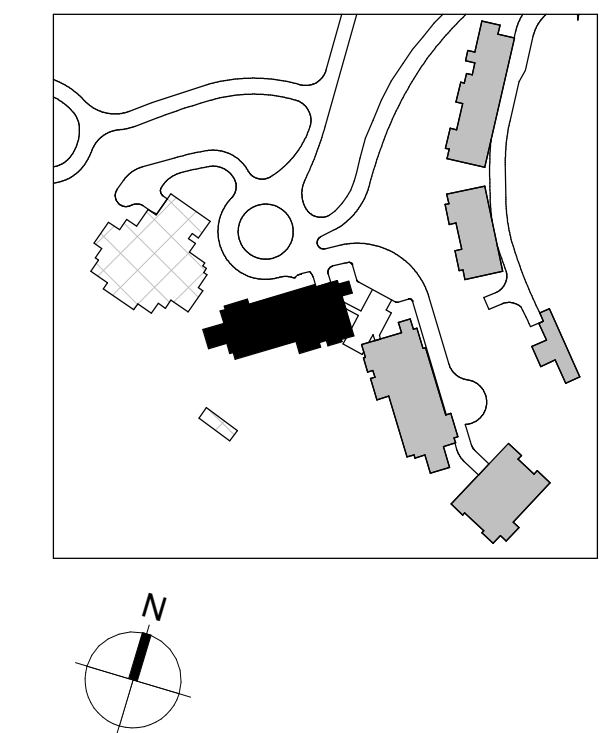
1 TOWER A - LEVEL 1 FRAMING PLAN  
1/8" = 1'-0"

**REFERENCE DRAWINGS**

- S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
- S1.XX LOAD DIAGRAMS
- S2.XX PLANS
- S3.XX ELEVATIONS
- S4.XX TYPICAL DETAILS AND SCHEDULES
- S5.XX CONCRETE SECTIONS AND DETAILS
- S6.XX STEEL SECTIONS AND DETAILS

**NOTES**

1. REFERENCE FLOOR ELEVATION IS 8345' - 0". TOP OF STRUCTURAL CONCRETE SLAB IS 8344' - 11" UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. THE STRUCTURAL SLAB IS A 14-INCH THICK MILD TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE THE TYPICAL MILD SLAB DETAILS.
3. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
4. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
5. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.
6. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
7. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
8. [Symbol] INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.
9. WHERE NOTED, ARCHITECTURAL CONCRETE WALLS ARE TO MAINTAIN 1" MINIMUM GAP TO PRIMARY STRUCTURAL COLUMNS/WALLS/SLABS.



principal architect \_\_\_\_\_  
 project manager \_\_\_\_\_  
 drawn by \_\_\_\_\_  
 checked by \_\_\_\_\_  
 job no. 20052  
 date 05/17/2024  
 revisions: \_\_\_\_\_

3 8/19/2024 ASI.004  
 04/08/2024 IFC SET 1 OF 3  
 11/18/2022 95% CD  
 no. date by

IFC SET 2 OF 3  
05/17/2024

TOWER A LEVEL 1  
FRAMING PLAN

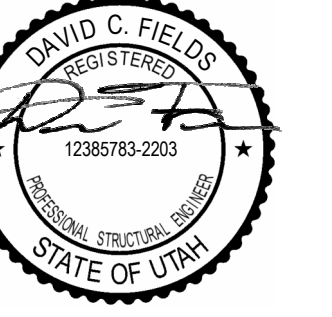
**S2.A.11**











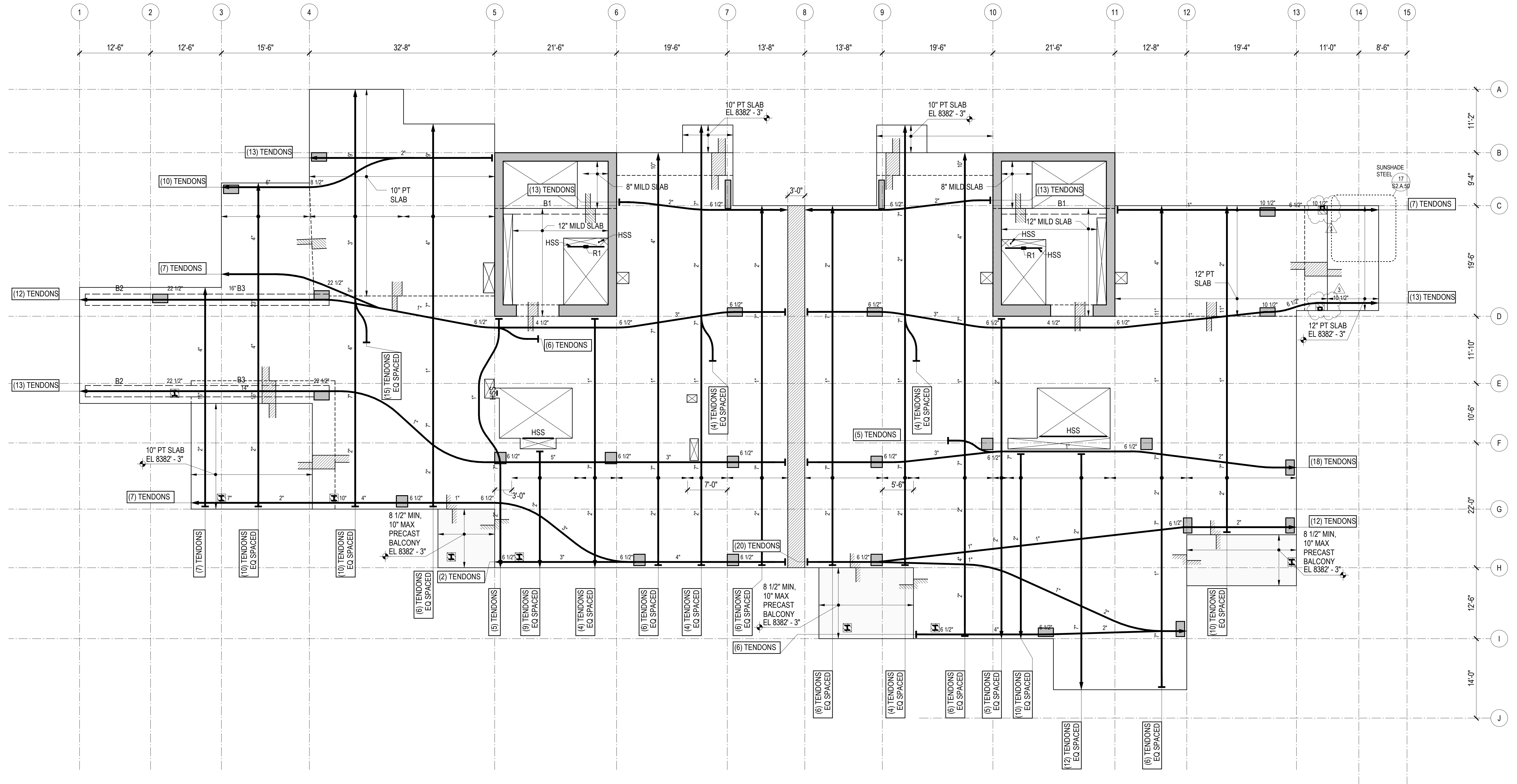
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1 TOWER A - LEVEL 4 FRAMING PLAN  
1/8" = 1'-0"

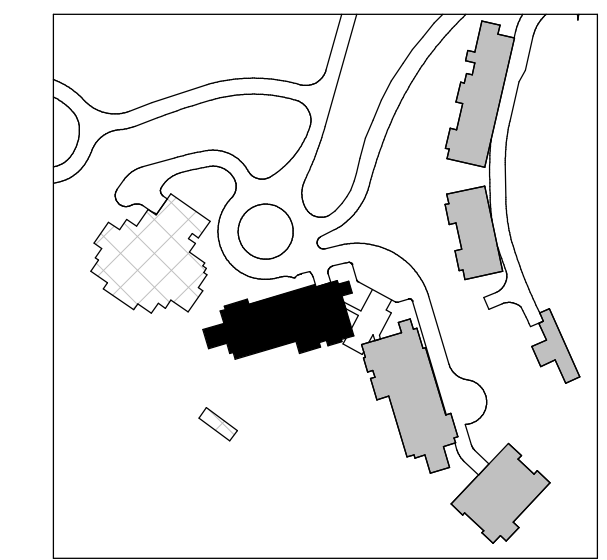
REFERENCE DRAWINGS

- S0.XX DRAWING INDEX, ABBREVIATIONS, LEGENDS, GENERAL NOTES
- S1.XX LOAD DIAGRAMS
- S2.XX PLANS
- S3.XX ELEVATIONS
- S4.XX TYPICAL DETAILS AND SCHEDULES
- S5.XX CONCRETE SECTIONS AND DETAILS
- S6.XX STEEL SECTIONS AND DETAILS

NOTES

1. REFERENCE FLOOR ELEVATION IS 8383'-0". TOP OF STRUCTURAL CONCRETE SLAB IS 8382'-11", UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE SLOPES NOT SHOWN.
2. STRUCTURAL SLAB IS AN 8-INCH THICK UNBONDED POST-TENSIONED TWO-WAY SLAB UNLESS NOTED OTHERWISE. SEE TYPICAL POST-TENSIONED SLAB DETAILS FOR ADDITIONAL INFORMATION.
3. THE MINIMUM NUMBER OF REQUIRED POST-TENSIONING TENDONS IS SHOWN ON THE DRAWINGS. FINAL COUNT, LAYOUT, AND LIVE END LOCATION IS PER DEFERRED DESIGN-BUILD SUBMITTAL PROVIDED BY THE CONTRACTOR.
4. CONCRETE PLACED IN THE SLAB/SHEAR WALL INTERSECTION, INCLUDING COUPLING BEAMS, SHALL HAVE MINIMUM CONCRETE STRENGTH EQUAL TO THAT SPECIFIED FOR THE SHEAR WALLS.
5. CONCRETE PLACED IN THE SLAB/COLUMN INTERSECTION SHALL HAVE MINIMUM CONCRETE STRENGTH AS SHOWN IN THE GENERAL NOTES, BUT NO LESS THAN THAT SPECIFIED FOR THE COLUMNS DIVIDED BY 1.4.
6. COORDINATE LOCATION OF ALL EMBEDS WITH MECHANICAL, ELECTRICAL, PLUMBING, AND EXTERIOR WALL SYSTEMS PRIOR TO CASTING THE SLAB.

7. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER DISCIPLINES DRAWINGS FOR OPENING SIZES AND LOCATIONS NOT SHOWN ON PLAN. SEE "TYPICAL OPENINGS AND EMBEDMENTS IN CONCRETE" DETAILS FOR OPENING PLACEMENT CRITERIA. NOTIFY THE STRUCTURAL ENGINEER OF ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS FOR WHICH THE TYPICAL DETAILS DO NOT APPLY.
8. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT, AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS, BOLLARDS, AND EDGE ANGLES. REINFORCE PER THE TYPICAL DETAILS.
9. [Symbol] INDICATES POUR STRIPS. WAIT 28 DAYS MINIMUM AFTER PLACING SLAB CONCRETE PRIOR TO CASTING POUR STRIPS. SEE "TYPICAL POST-TENSIONED DELAY STRIP" DETAIL FOR MORE INFORMATION.
10. [Symbol] INDICATES TYPICAL BUILT-UP SLAB ON RIGID FOAM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND ELEVATIONS OF ARCHITECTURAL BUILT-UP SLABS. SEE TYPICAL BUILT-UP SLAB DETAIL FOR ADDITIONAL INFORMATION.



principal architect \_\_\_\_\_  
 project manager \_\_\_\_\_  
 drawn by \_\_\_\_\_  
 checked by \_\_\_\_\_  
 job no. 20052  
 date 05/17/2024  
 revisions: \_\_\_\_\_

3 8/19/2024 ASI-004  
 04/08/2024 IFC SET 1 OF 3  
 11/18/2022 95% CD  
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TOWER A LEVEL 4  
FRAMING PLAN

S2.A.14

































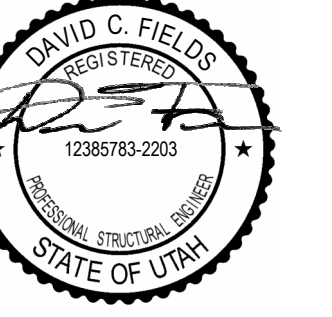










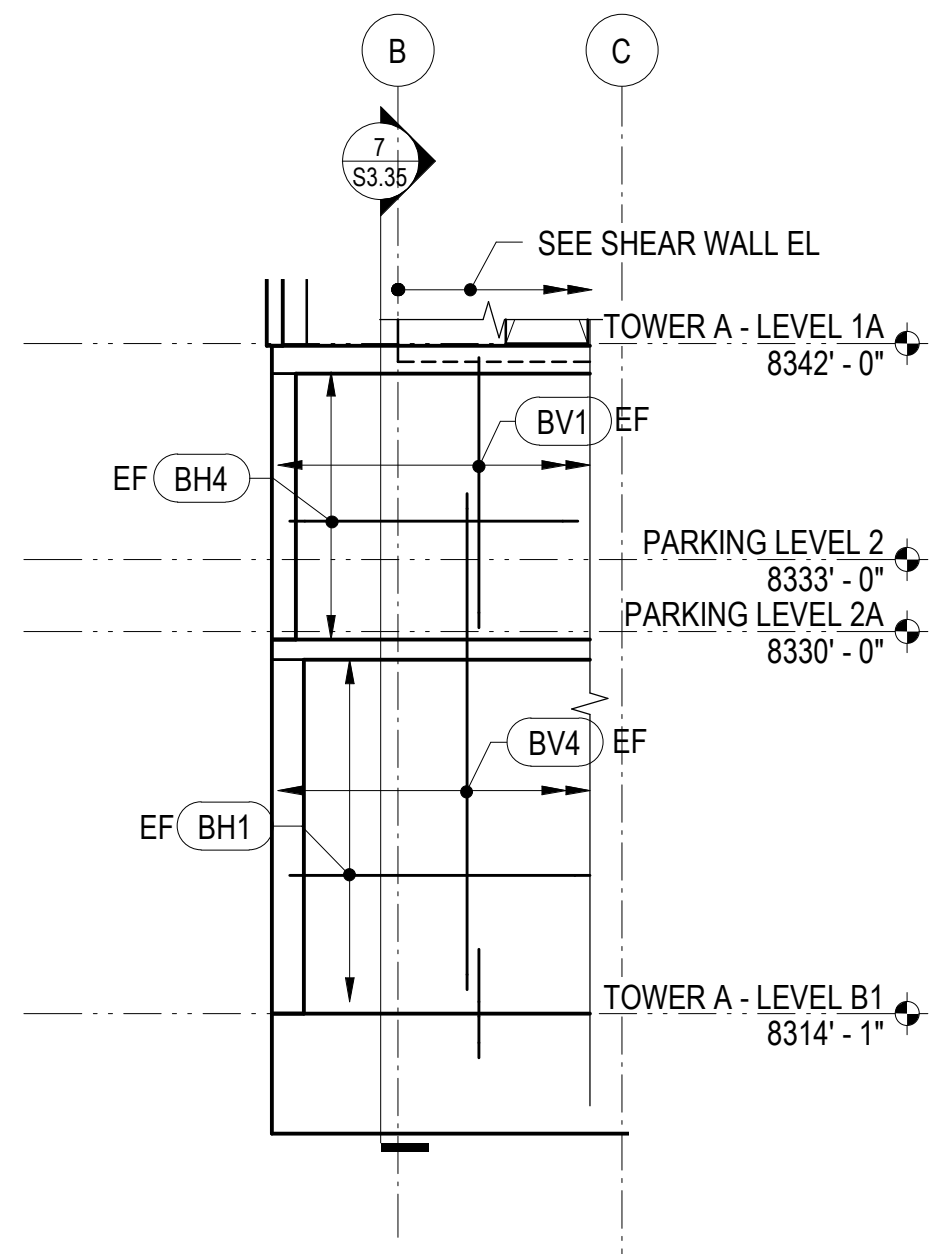


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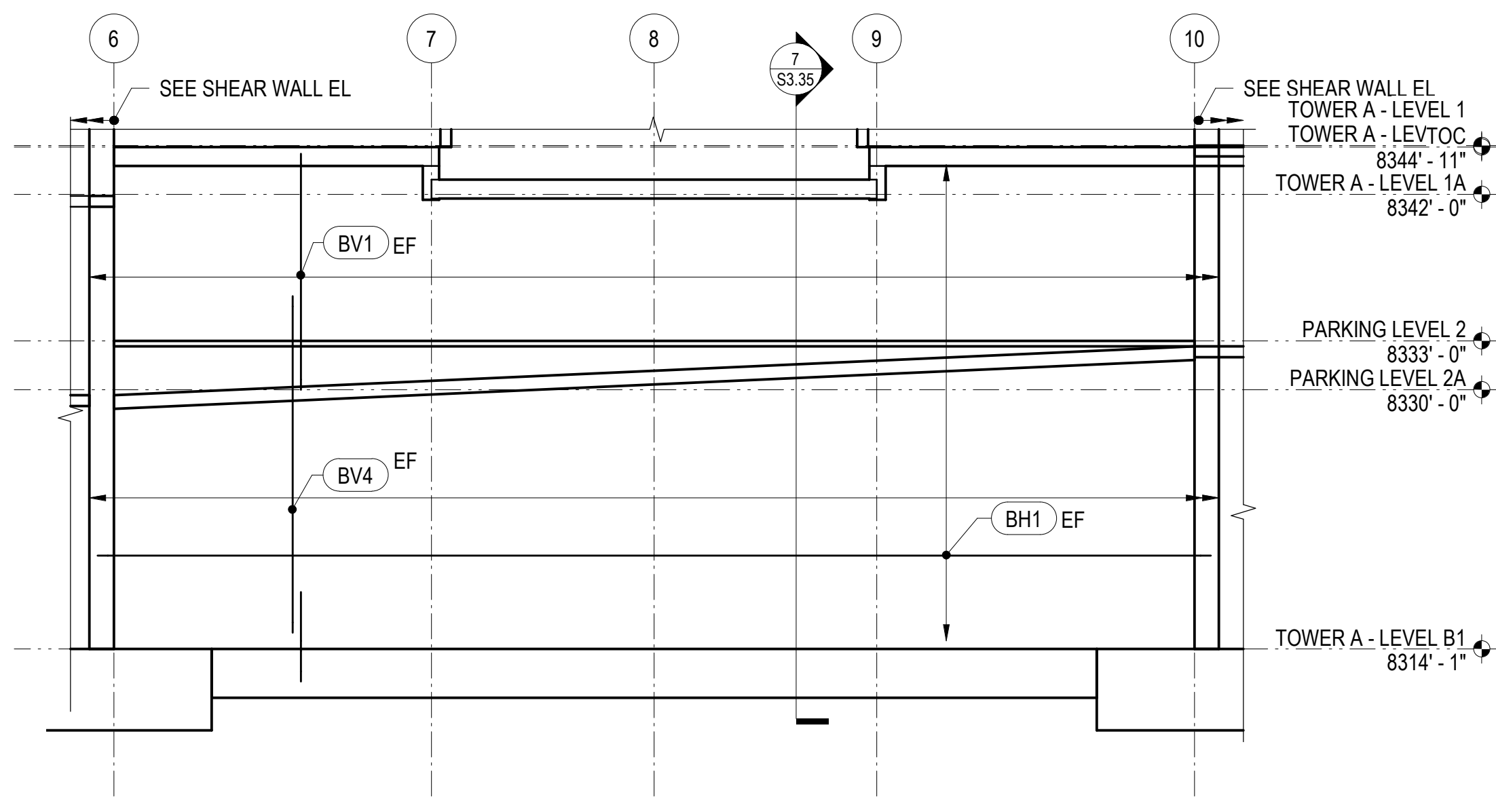
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project  
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DEER VALLEY, UTAH

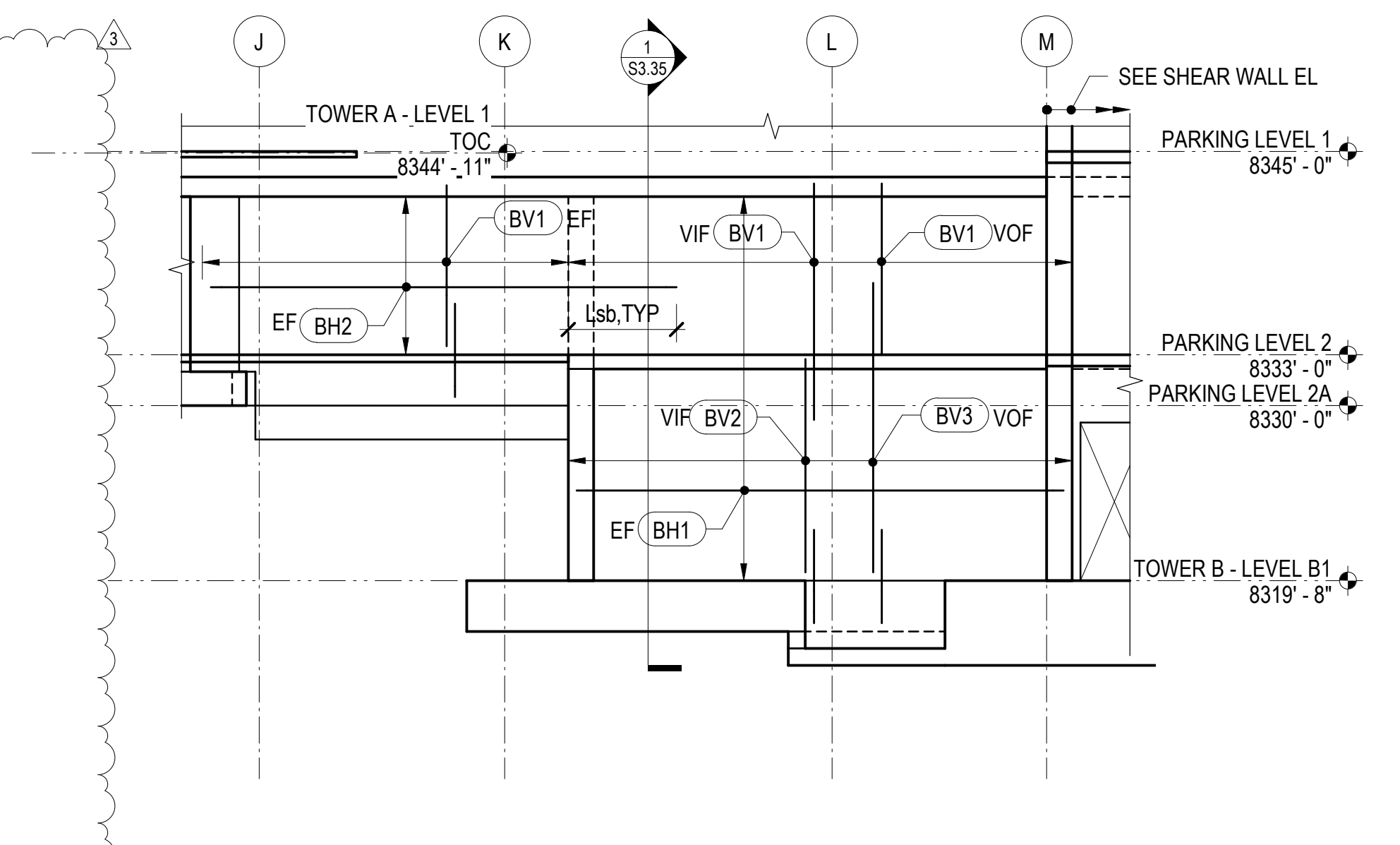
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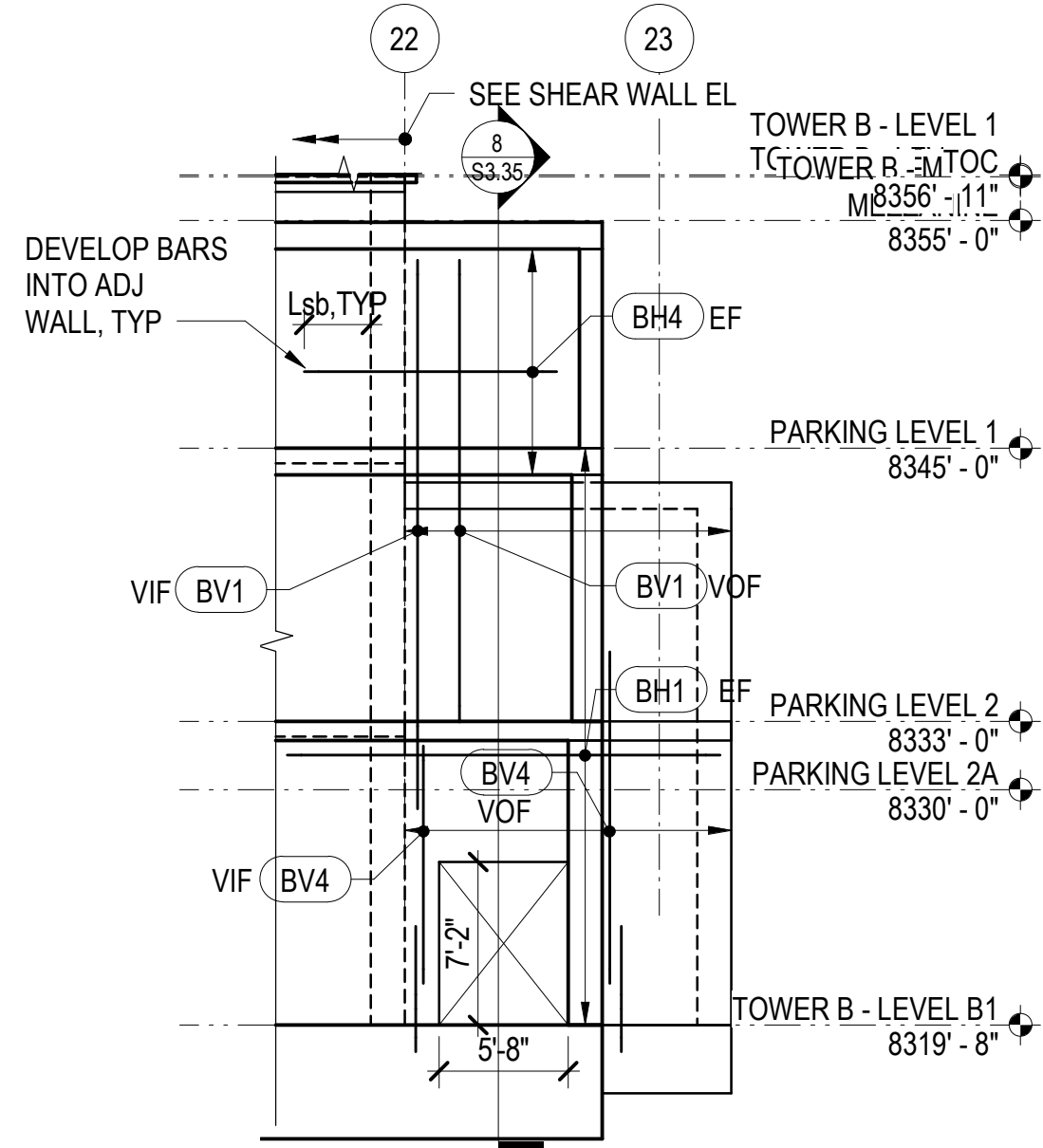
8 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



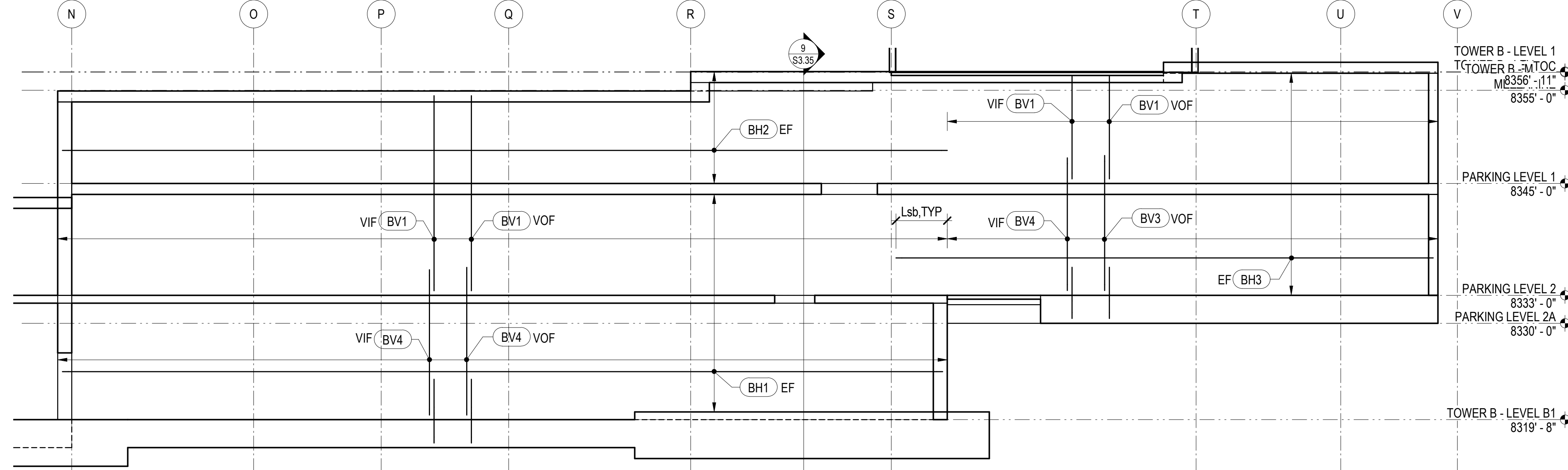
9 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



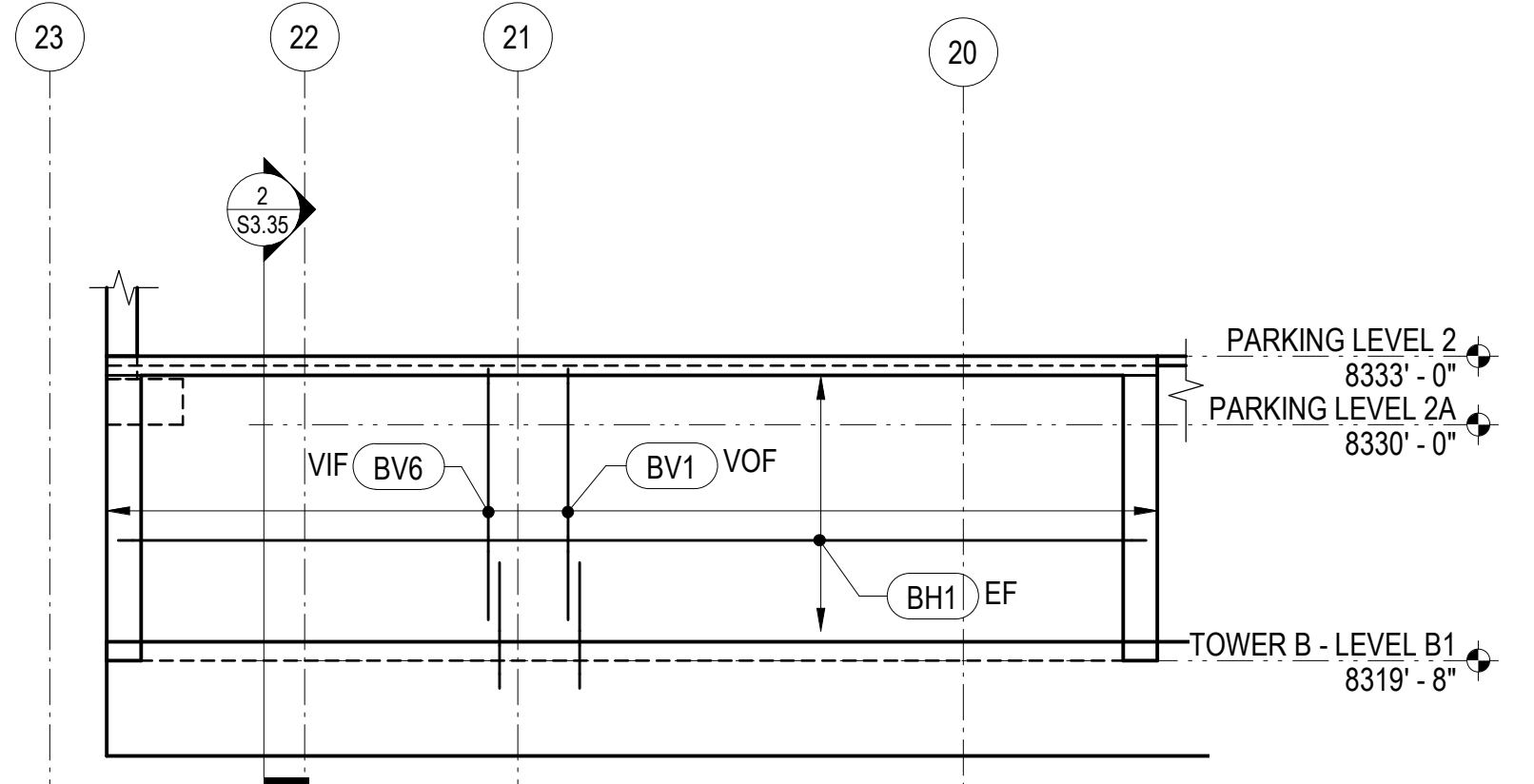
11 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



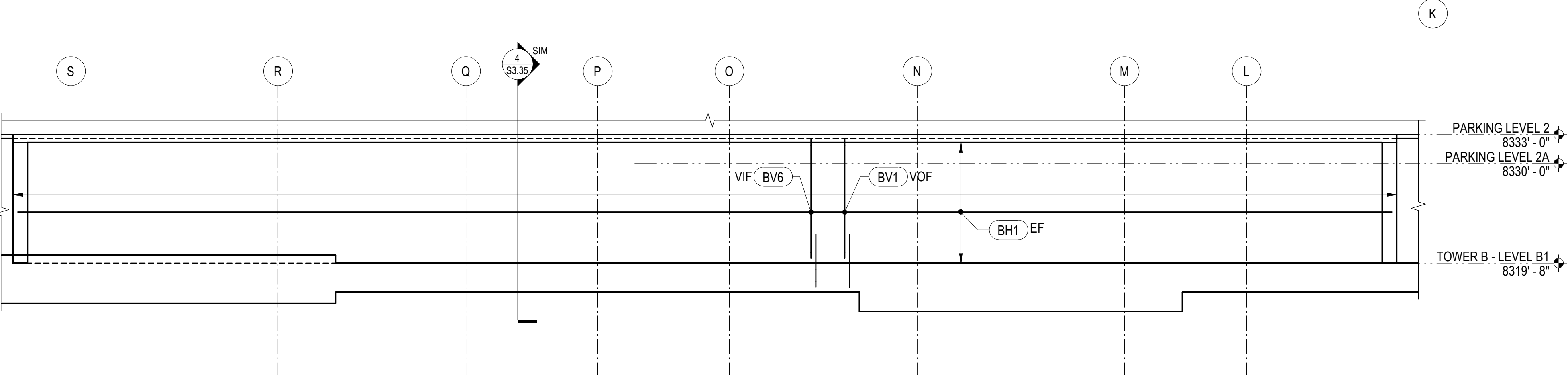
12 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



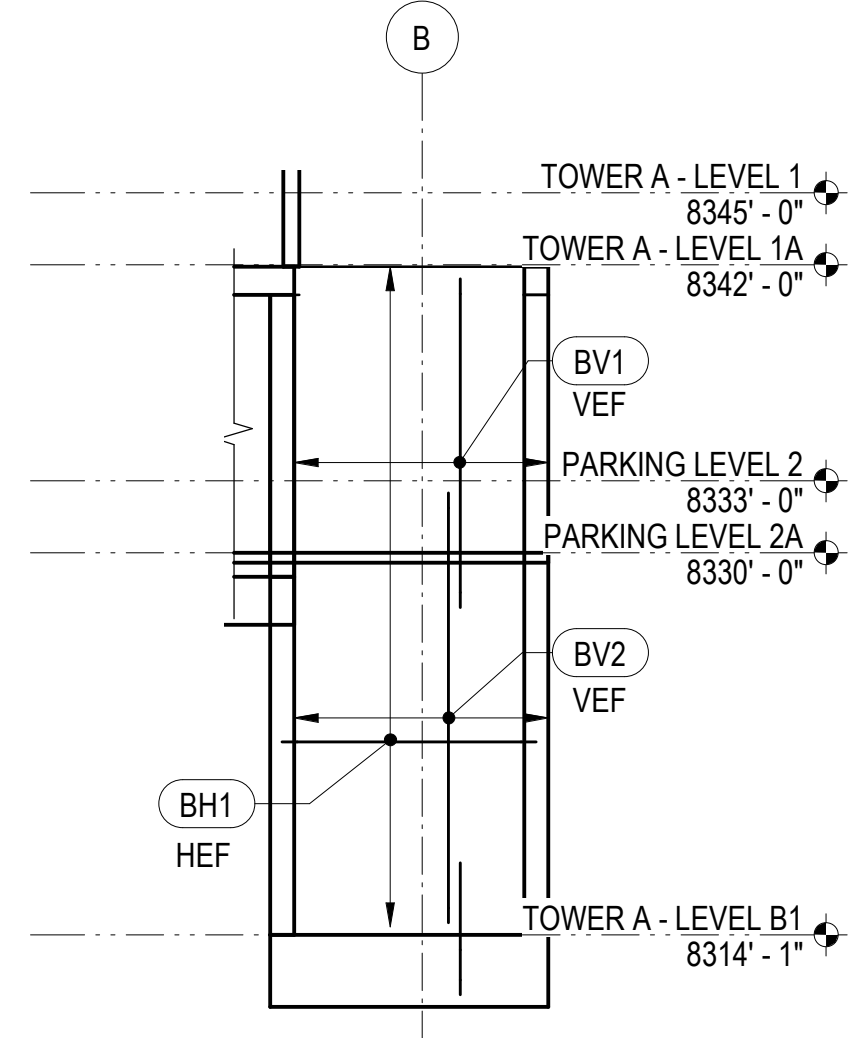
13 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



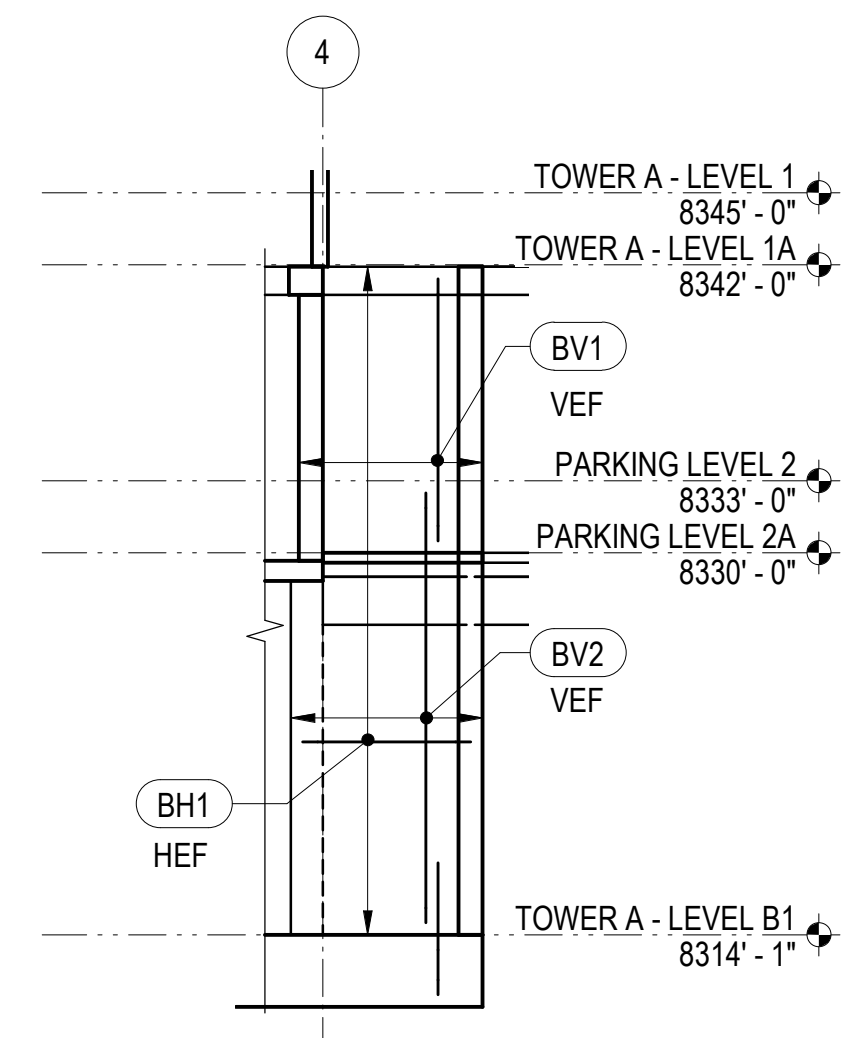
14 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



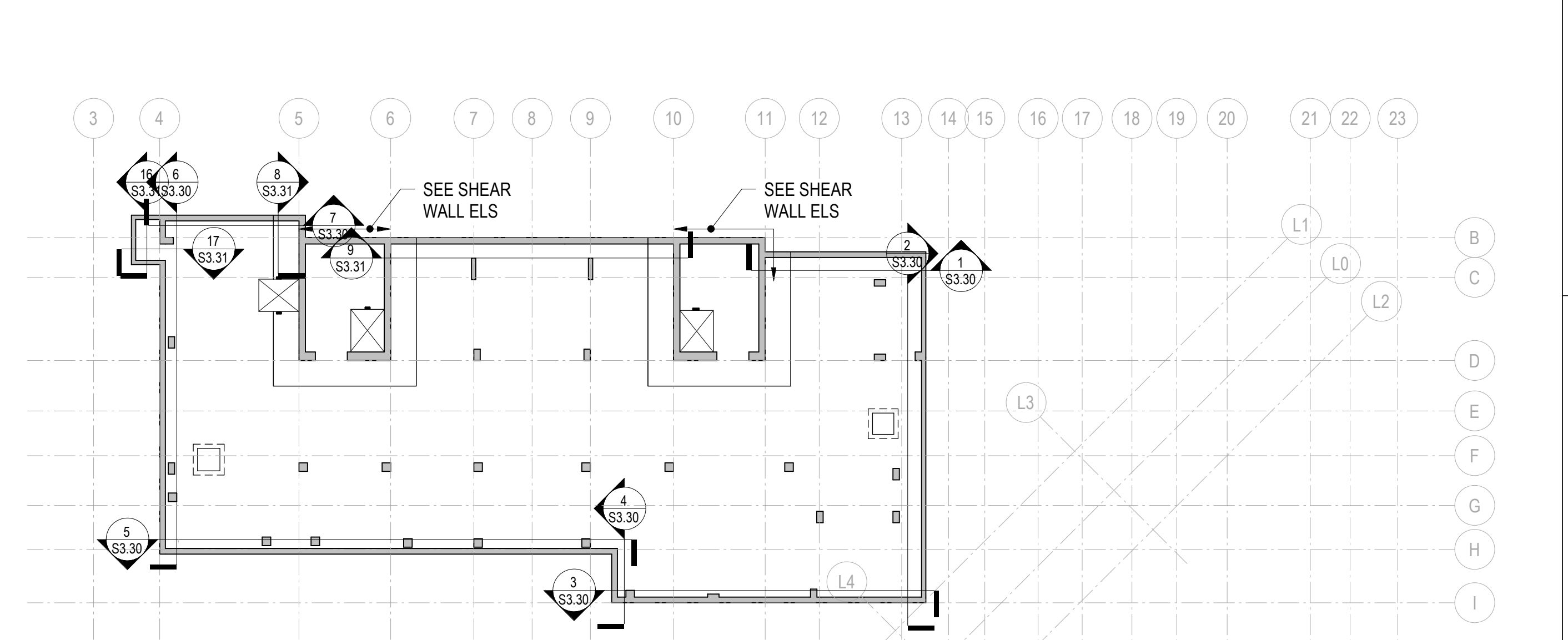
15 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



16 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



17 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV3	#8 @ 12"	
BV4	#7 @ 6"	
BV5	#8 @ 6"	
BV6	#9 @ 6"	

MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	
BH3	#4 @ 10"	
BH4	#7 @ 12"	

LEVEL B1 - KEY PLAN

principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
job no. 20052  
date 05/17/2024

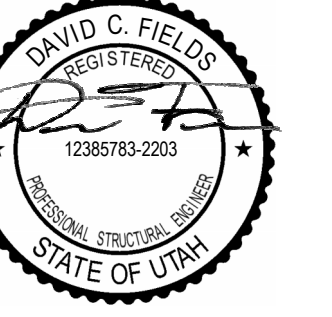
revisions:

no.	date	by
1	05/17/2024	ASJ-004
2	04/08/2024	ASJ-002
3	11/18/2022	95% CD

IFC SET 2 OF 3  
05/17/2024

TOWER A & B  
BASEMENT WALL  
ELEVATIONS

S3.31



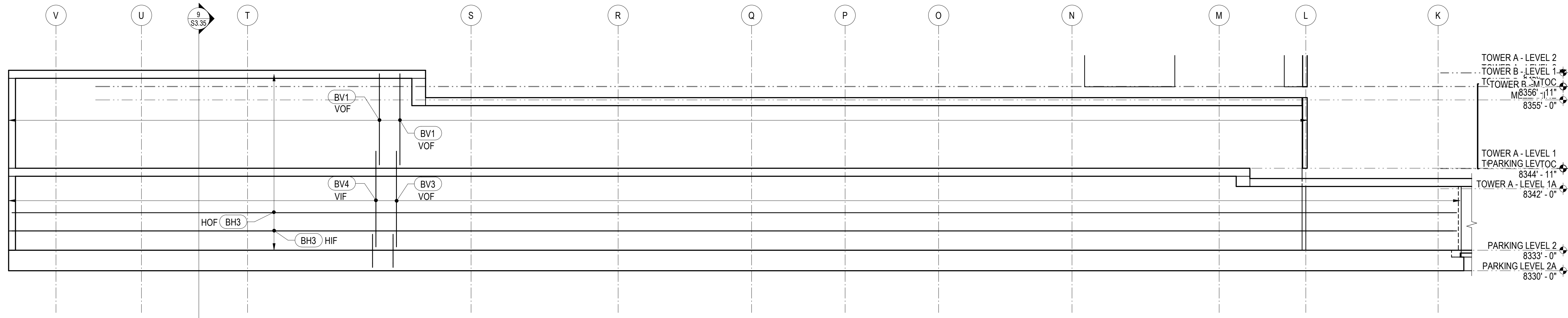
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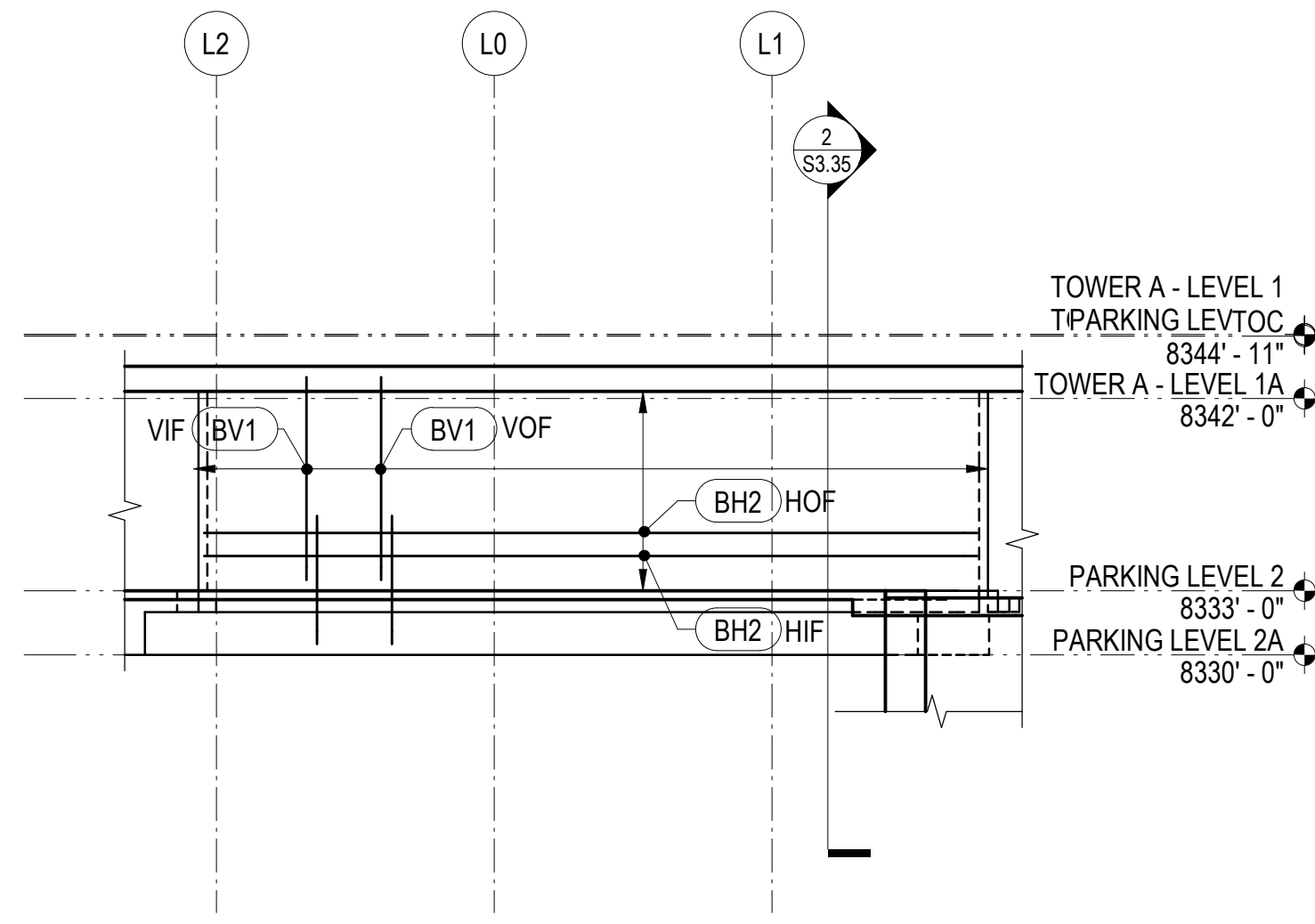
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project: **SOMMET BLANC - ABC**  
DEER VALLEY, UTAH

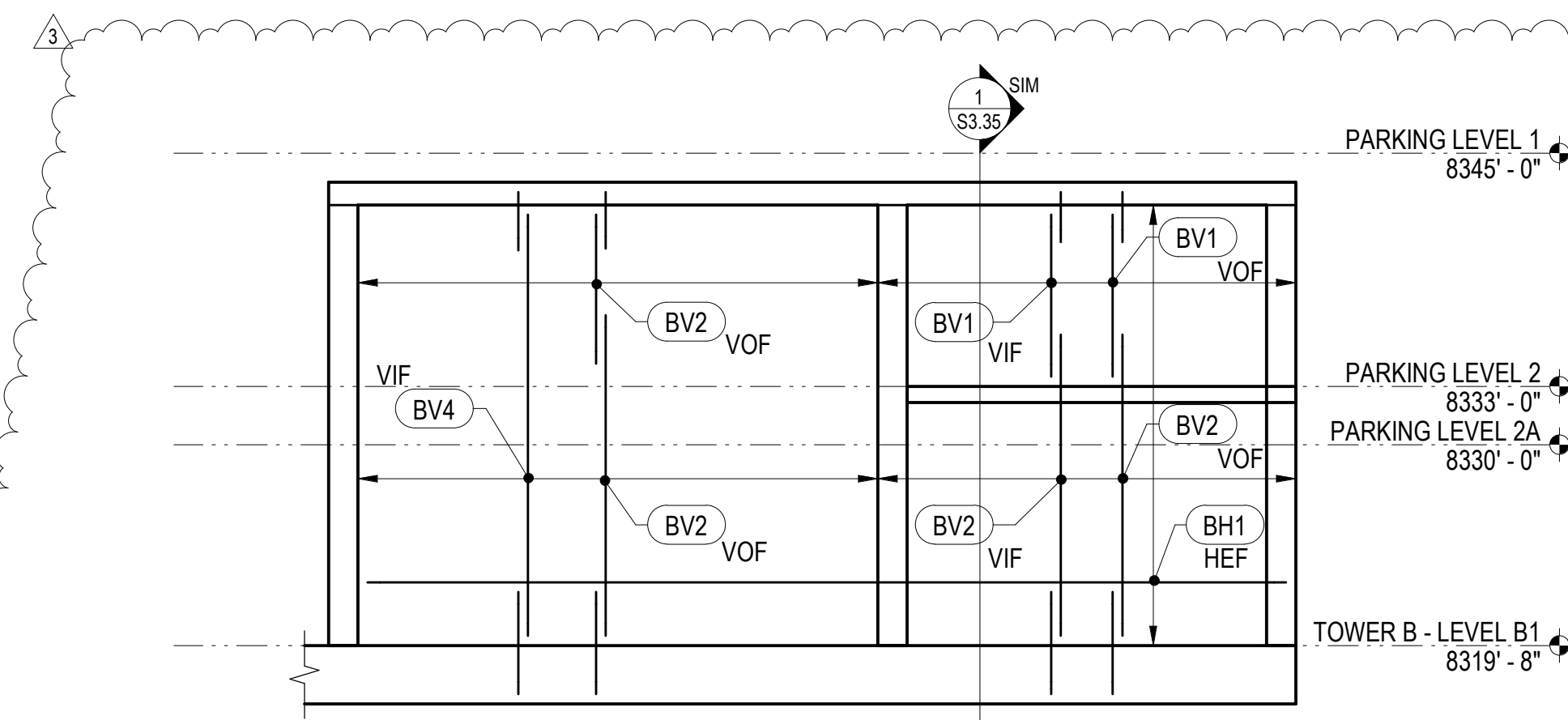
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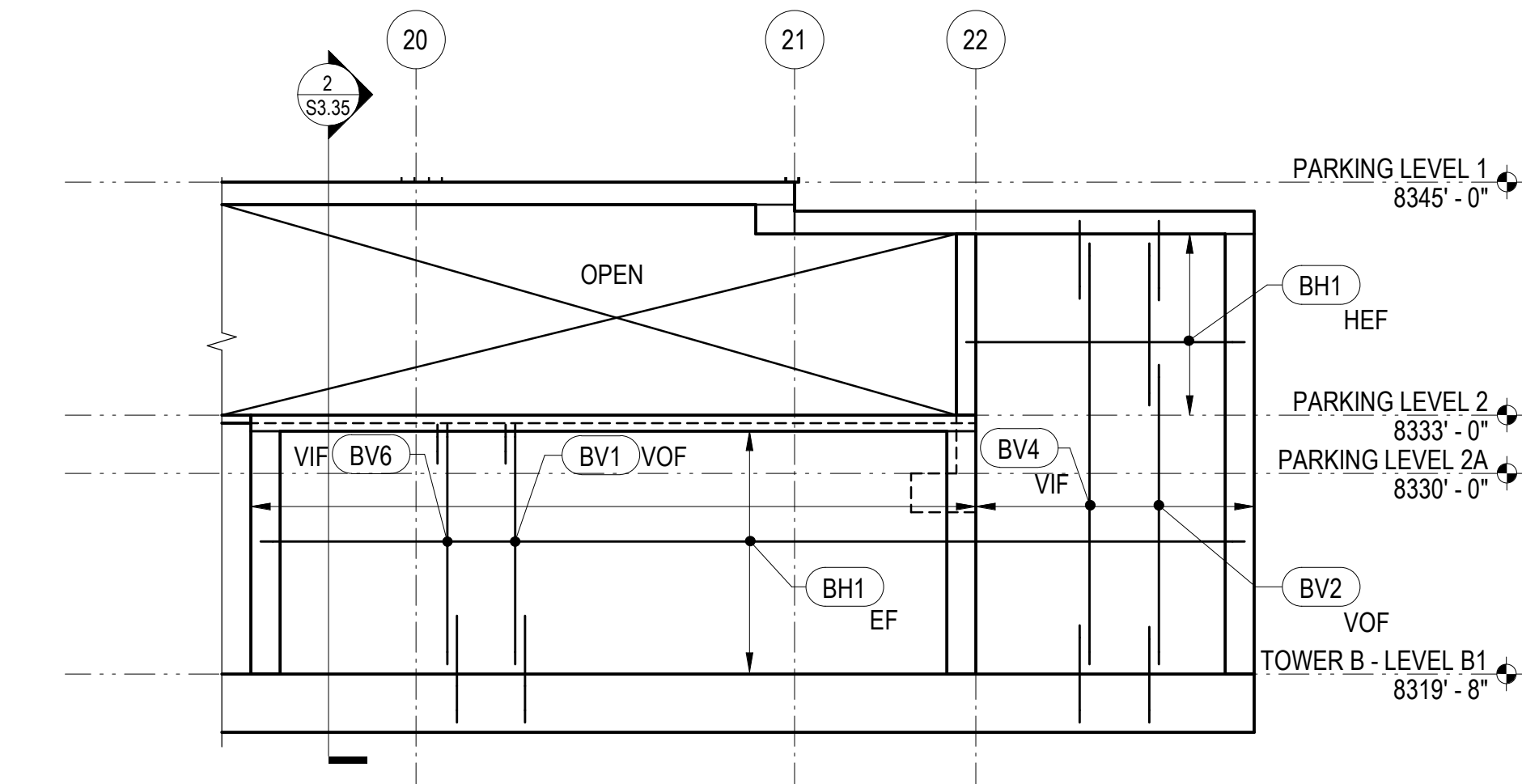
26 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



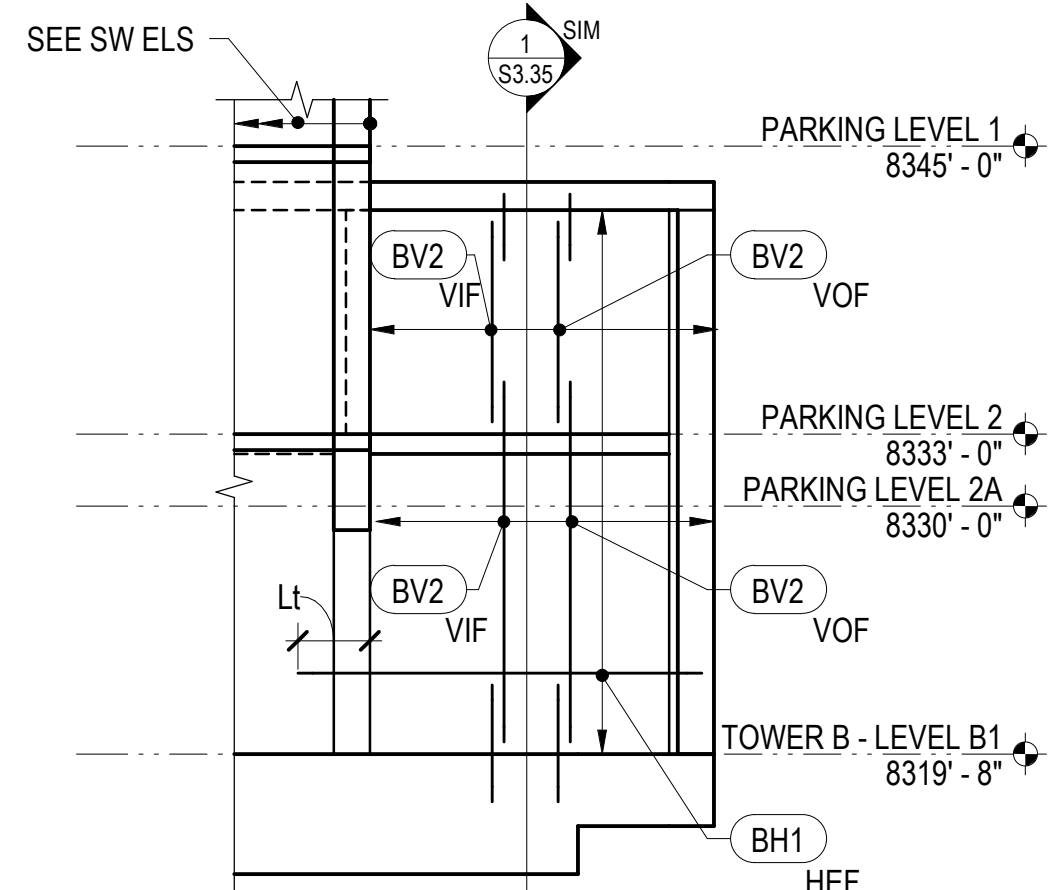
27 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



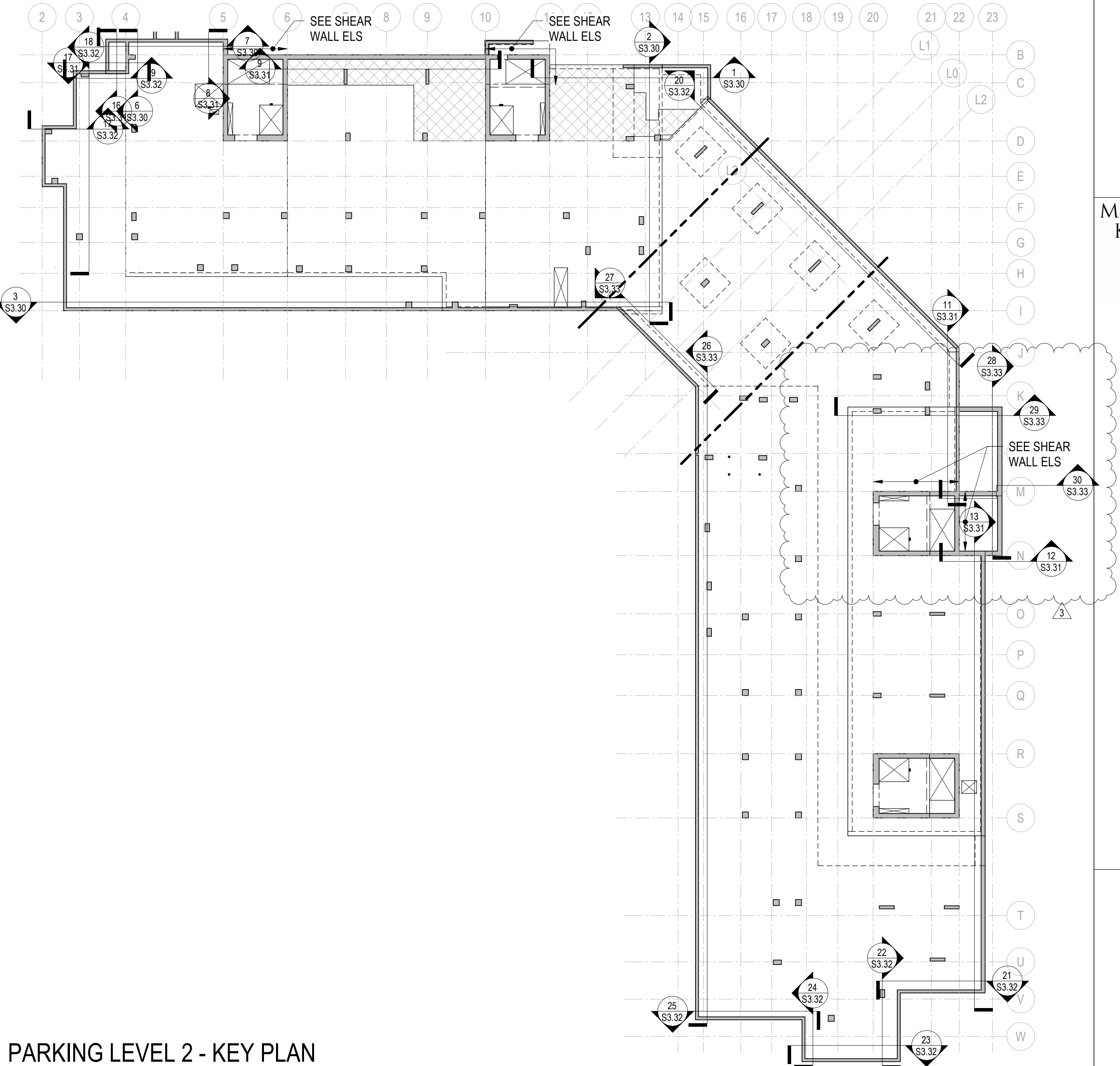
28 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



29 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



30 BASEMENT WALL ELEVATION  
1/8" = 1'-0"



PARKING LEVEL 2 - KEY PLAN

BASEMENT WALL VERTICAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BV1	#6 @ 12"	
BV2	#7 @ 12"	
BV3	#8 @ 12"	
BV4	#7 @ 6"	
BV5	#8 @ 6"	
BV6	#9 @ 6"	

BASEMENT WALL HORIZONTAL REINFORCEMENT		
MARK	REINFORCING	REMARKS
BH1	#5 @ 12"	
BH2	#4 @ 12"	
BH3	#4 @ 10"	
BH4	#7 @ 12"	

principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
job no. 20052  
date 05/17/2024

revisions:

3	8/19/2024	ASI-004
2	7/26/2024	ASI-002
	04/08/2024	IFC SET 1 OF 3
	11/18/2022	95% CD

no. date by

IFC SET 2 OF 3

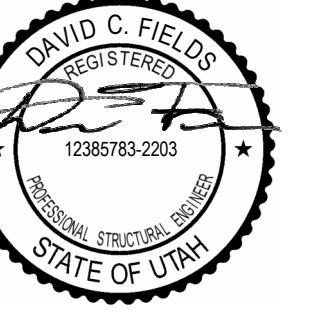
05/17/2024

TOWER A & B  
BASEMENT WALL  
ELEVATIONS

S3.33







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DEER VALLEY, UTAH

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principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
job no. 20052  
date 05/17/2024  
revisions: \_\_\_\_\_

3 8/19/2024 ASI-004  
2 7/26/2024 ASI-002  
1 03/17/2024 IFC 2  
04/08/2024 IFC SET 1 OF 3  
11/18/2022 95% CD  
no. date by

IFC SET 2 OF 3

05/17/2024

TYPICAL CONCRETE BEAM DETAILS AND SCHEDULE

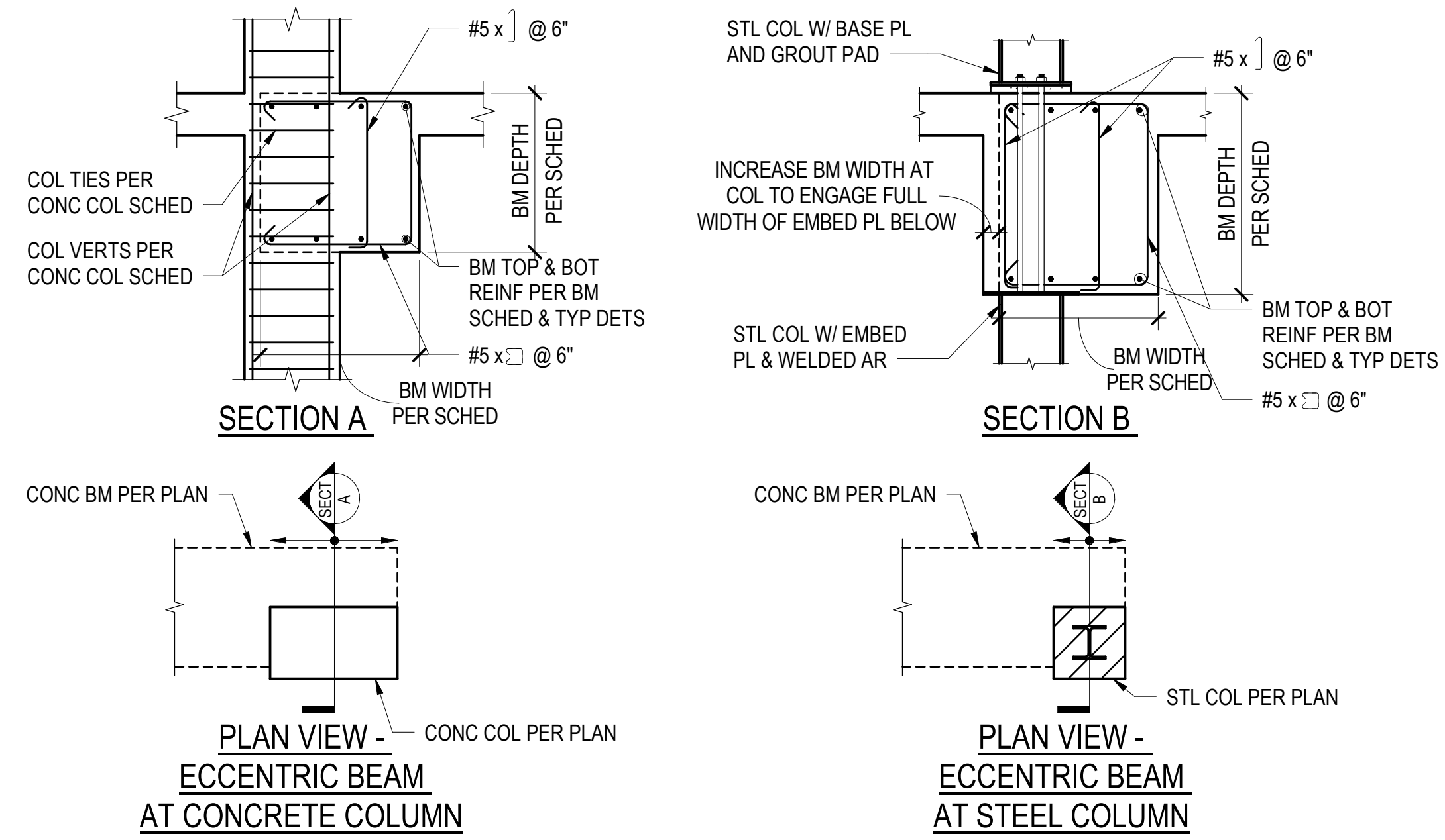
**S4.03**

MARK	SIZE (WIDTHxDEPTH)	CAMBER	BOTTOM BARS	TOP BARS			STIRRUPS		REMARKS
				LEFT	CONTINUOUS	RIGHT	LEFT	RIGHT	
B1	12"x18"		(2) #7	(2) #7	(2) #7	(2) #7	#4 @ 6" [2C]		
B2	24"x24"		(3) #8	(7) #8			#5 @ 9" [2C]		SEE 1/S4.03
B3	24"x24"		(4) #8	(4) #8	(3) #8	(4) #8	#5 @ 9" [2C]		SEE 1/S4.03
B5	24"x22" MIN		(3) #8	(9) #9			#5 @ 9" [2C]		SEE SECTION PER PLAN FOR DEPTH-SEE 1/S4.03
B6	24"x26 1/2"		(3) #8	(5) #9	(4) #9	(5) #9	#5 @ 9" [2C]		SEE SECTION PER PLAN FOR DEPTH-SEE 1/S4.03
B7	24"x24"		(3) #8	(10) #9			#5 @ 9" [3C]		SEE 1/S4.03
B8	24"x24"		(8) #10	(5) #10	(5) #10	(5) #10	#6 @ 5" [3C]		SEE 1/S4.03
B9	30"x24"		(4) #7	(7) #9			#5 @ 9" [3C]		ADD (2) #4 SIDE BAR EA SIDE; SEE 1/S4.03
B10	30"x24"		(4) #7	(7) #9			#5 @ 9" [3C]		ADD (3) #4 SIDE BAR EA SIDE; SEE 1/S4.03
B12	30"x22" MIN		(4) #7	(9) #10			#5 @ 9" [3C]		ADD (2) #4 SIDE BAR EA SIDE - SEE SECTION PER PLAN FOR DEPTH; SEE 1/S4.03
B13	30"x26 1/2"		(5) #7	(10) #10			#5 @ 9" [3C]		ADD (2) #4 SIDE BAR EA SIDE - SEE SECTION PER PLAN FOR DEPTH; SEE 1/S4.03
B16	30"x24"		(4) #7	(6) #8			#5 @ 6" [3C]		ADD (3) #5 SIDE BAR EA SIDE; SEE 1/S4.03
B17	30"x24"		(4) #7	(9) #8			#4 @ 9" [4C]		ADD (3) #4 SIDE BAR EA SIDE; SEE 1/S4.03
B21	30"x22" MIN		(4) #7	(10) #10			#6 @ 6" [3C]		ADD (2) #4 SIDE BAR EA SIDE - SEE SECTION PER PLAN FOR DEPTH; SEE 1/S4.03
B22	30"x26 1/2"		(6) #8	(10) #10			#6 @ 6" [3C]		ADD (2) #4 SIDE BAR EA SIDE - SEE SECTION PER PLAN FOR DEPTH; SEE 1/S4.03
B23	30"x26 1/2"		(4) #7	(8) #9			#5 @ 8" [3C]		ADD (2) #4 SIDE BAR EA SIDE - SEE SECTION PER PLAN FOR DEPTH; SEE 1/S4.03
B26	30"x30"		(3) #7	(4) #7			#5 @ 9" [3C]		SEE 1/S4.03
B27	30"x30"		(3) #7	(4) #7			#6 @ 6" [2C]		SEE 1/S4.03
B28	18"x24"		(3) #7	(3) #7			#5 @ 5" [2C]		
B29	30"x24"		(4) #7	(6) #8			#5 @ 9" [2C]		SEE 1/S4.03
B30	30"x24"		(4) #7	(9) #8			#5 @ 10" [2C]		SEE 1/S4.03
B33	24"x32"		(4) #8	(3) #7			#5 @ 14" [2C]		
B34	24"x24"		(4) #8	(4) #8			#5 @ 10" [2C]		SEE 1/S4.03
B35	24"x24"		(5) #8	(5) #8			#5 @ 10" [2C]		SEE 1/S4.03
B37	18"x32"		(3) #7	(3) #7			#5 @ 5" [2C]		
B38	24"x32"		(4) #7	(4) #8			#5 @ 14" [2C]		
B39	24"x32"		(4) #8	(4) #8			#5 @ 9" [3C]		
B40	24"x32"		(6) #9	(4) #8			#5 @ 9" [3C]		
B41	34"x30"		(6) #8	(6) #8			#5 @ 9" [3C]		
B42	32"x32"		(4) #9	(4) #9			#4 @ 14" [4C]		
B44	24"x48"		(3) #7	(3) #7			#4 @ 14" [3C]		
B45	24"x48"		(3) #8	(3) #8			#4 @ 14" [3C]		
B46	24"x32"		(3) #8	(3) #8			#4 @ 14" [3C]		
B47	24"x32"		(3) #9	(5) #9			#4 @ 14" [3C]		
B48	24"x50"		(3) #8	(3) #9			#4 @ 14" [3C]		
B49	24"x32"		(3) #9	(3) #9			#4 @ 14" [3C]		
B50	12"x39"		(3) #7	(3) #7			#4 @ 14" [3C]		
B51	18"x32"		(4) #8	(4) #8			#4 @ 10" [4C]		
B52	32"x72"		(14) #11	(4) #10	(3) #10	(3) #10	#5 @ 6" [4C]		
B53	18"x36"		(4) #8	(2) #8	(3) #8	(2) #8	#5 @ 12" [2C]		
B54	24"x32"		(3) #7	(3) #7			#4 @ 14" [3C]		
B56	18"x38"		(3) #9	(3) #9			#4 @ 14" [3C]		
B57	24"x33"		(3) #9	(3) #9			#4 @ 14" [3C]		
B58	30"x36"		(4) #9	(3) #7	(4) #9	(3) #7	#5 @ 14" [4C]	(13) #5 @ 6" [3C]	
B59	24"x62"		(4) #9	(4) #9			#4 @ 12" [4C]		
B61	24"x39"		(6) #10	(6) #8			#4 @ 14" [4C]		
B62	24"x74"		(3) #8	(3) #8			#4 @ 14" [3C]		
B63	24"x72"		(5) #11	(5) #11			#4 @ 14" [3C]		
B64	32"x72"		(8) #11	(8) #11			#5 @ 8" [4C]		
B65	24"x60"		(6) #11	(6) #9			#5 @ 14" [4C]		
B66	24"x74"		(5) #11	(5) #11			#4 @ 14" [3C]		
B67	24"x26"		(3) #8	(3) #8			#4 @ 14" [3C]		
B68	24"x61"		(4) #9	(4) #9			#4 @ 14" [4C]		
B69	30 1/2"x48"		(4) #11	(4) #11			#4 @ 14" [4C]		
B71	40"x42"		(11) #18	(6) #11	(6) #11	(6) #11	#6 @ 4" [4C]		
B72	60"x42"		(14) #11	(8) #9			#5 @ 6" [7C]		
B73	38 1/2"x24"		(3) #7	(3) #8			#5 @ 6" [2C]		SEE DETAIL 18/S5.05
B74	24"x67"		(5) #9	(5) #9			#5 @ 9" [3C]		
B75	36"x30"		(5) #8	(6) #10			#6 @ 6" [5C]		SEE 1/S4.03
B76	24"x29" MIN		(4) #9	(4) #8			#5 @ 9" [3C]		25" MINIMUM DEPTH; BOTTOM OF BEAM FLAT AT ELEVATION 8373'-9"; TOP OF BEAM STEPS WITH SLAB
B77	72"x18"		(6) #6	(2) #6			#5 @ 18" [2]		REINF SIM TO 18/S4.05. SEE PLAN FOR ADDED TOP BAR AT END
B78	32"x20"		(4) #9	(5) #9			#5 @ 6" [3C]		
B79	12" MIN x 33"		(3) #9	(3) #9			#5 @ 6" [2C]		CLOSED STIRRUPS ARE TO BE CONTINUOUS, NO CAPTIE PER [2C ALT] ALLOWED. SEE DETAIL 08/S5.02

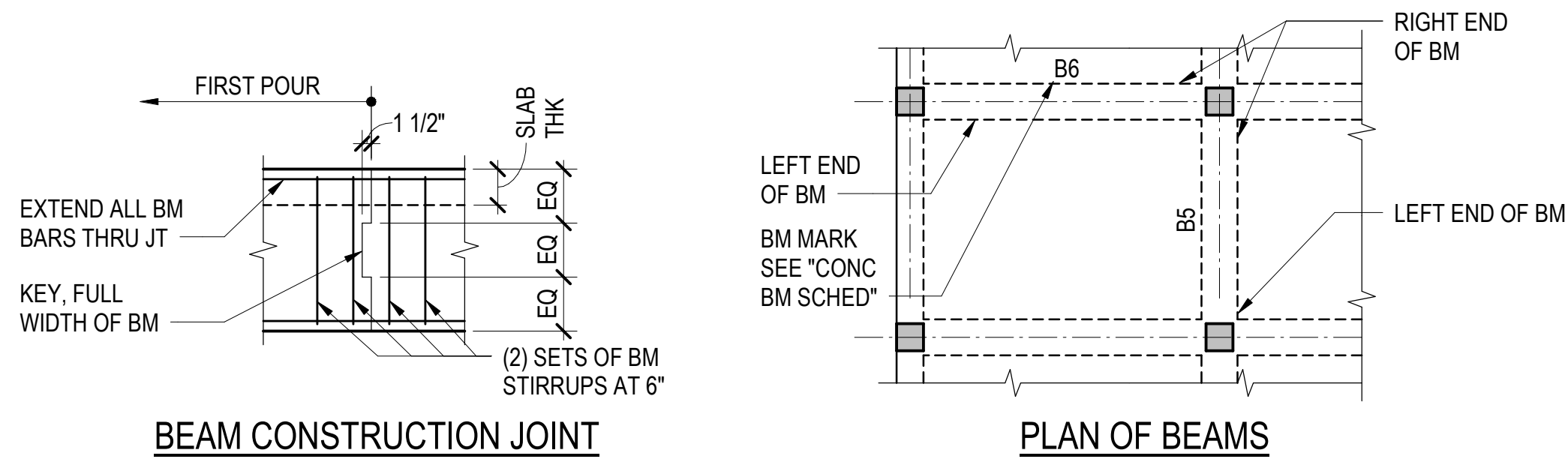
**NOTES:**

- AT CONTRACTOR'S OPTION, WHERE REQUIRED TO RELIEVE BAR CONGESTION, NOT MORE THAN 50 PERCENT OF THE AREA OF THE STRAIGHT BOTTOM BARS MAY BE TERMINATED AS SHOWN UNLESS NOTED OTHERWISE.
- BEAM SCHEDULES DO NOT INDICATE REQUIREMENTS FOR ARRANGING BARS. THE CONTRACTOR SHALL DETAIL AND PLACE REINFORCING STEEL IN A SINGLE LAYER WHENEVER POSSIBLE. A SECOND LAYER MAY BE USED ONLY WHERE REQUIRED TO PROVIDE PROPER CLEARANCES BETWEEN BARS IN A LAYER AND WHERE REQUIRED IN ORDER TO PROPERLY CLEAR COLUMN VERTICALS AND SIMILAR REINFORCING.
- EITHER 90 OR 180 DEGREE STANDARD HOOK BARS MAY BE USED FOR LONGITUDINAL BARS.
- WHERE TOP BARS ARE INDICATED AS CONTINUOUS AND RUN OVER 60 FEET IN LENGTH, BARS MAY BE LAPPED L<sub>d</sub> IN THE MIDDLE THIRD OF THE BEAM SPAN UNLESS NOTED OTHERWISE. CONTINUOUS TOP BARS SHALL NOT BE LAPPED IN THE SPAN ADJACENT TO A CANTILEVER, UNLESS NOTED OTHERWISE. WHERE BOTTOM BARS ARE SHOWN AS CONTINUOUS AND RUN IN EXCESS OF 60 FEET, A LAP SPICE MAY BE USED EQUAL TO L<sub>s</sub> AND SHALL BE OUTSIDE THE MIDDLE THIRD OF THE BEAM SPAN. SIDE BAR SPLICES MAY BE MADE WHERE CONVENIENT.
- LOCATE ALL CONSTRUCTION JOINTS WITHIN THE MIDDLE THIRD OF SPAN. JOINTS SHALL BE OFFSET AT A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS. SUBMIT LOCATION OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE BEFORE FORMING.
- ALL BARS IN SAME LAYER UNLESS NOTED OTHERWISE.

**3 TYPICAL CONCRETE BEAM NOTES**



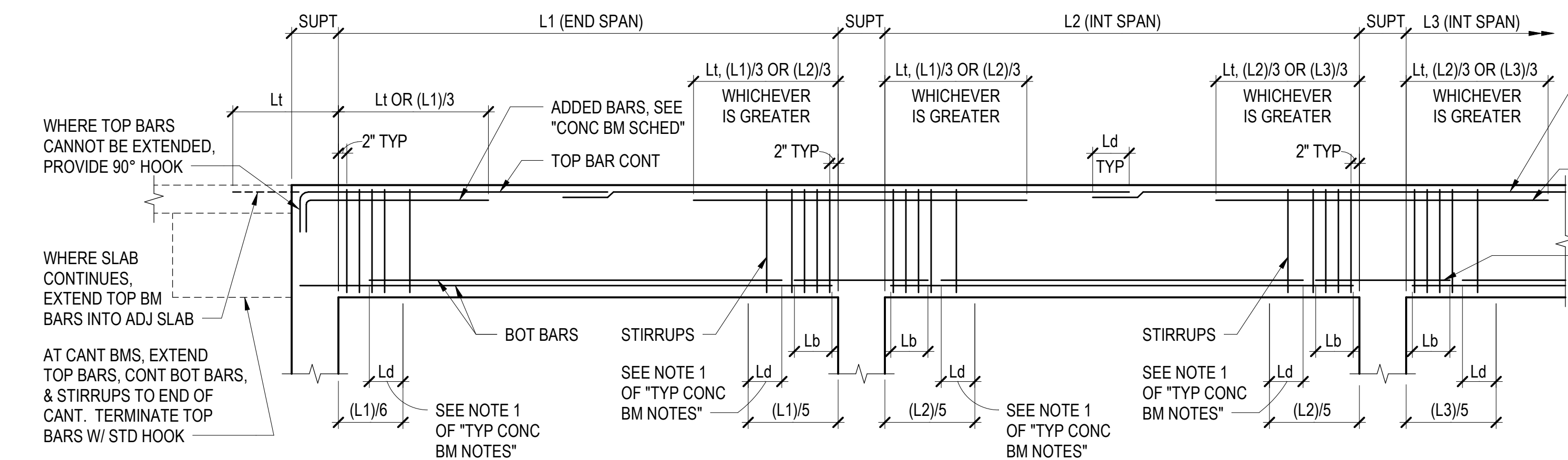
**1 ECCENTRIC BEAM AT COLUMN**



CONT BARS, SEE "CONC BM SCHED." PROVIDE HEAVIER OF L2 OR L3 CONT REINF. TYP

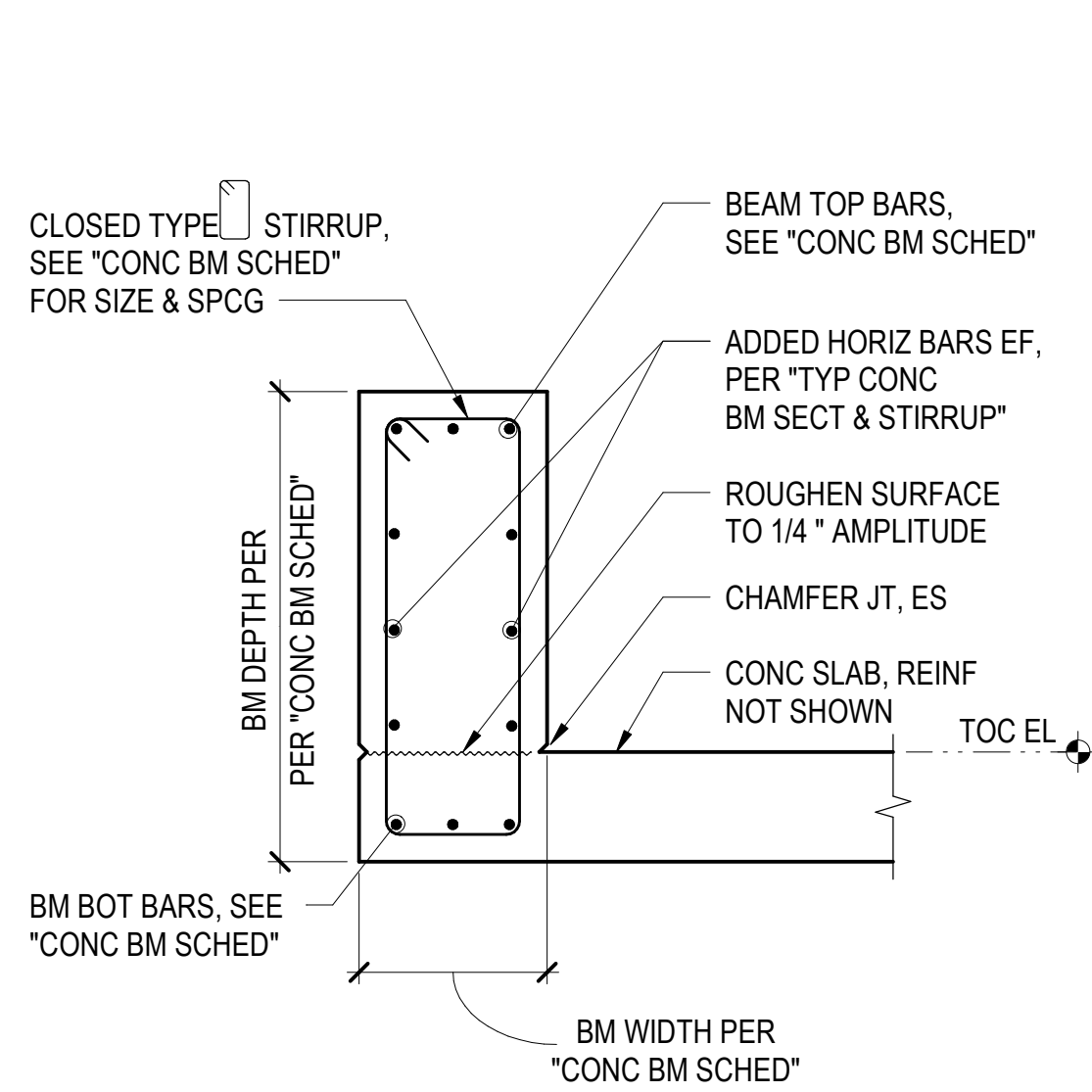
ADDED BARS SEE "CONC BM SCHED", TYP

DWLS TO MATCH LARGEST AREA OF BOT BARS, TYP

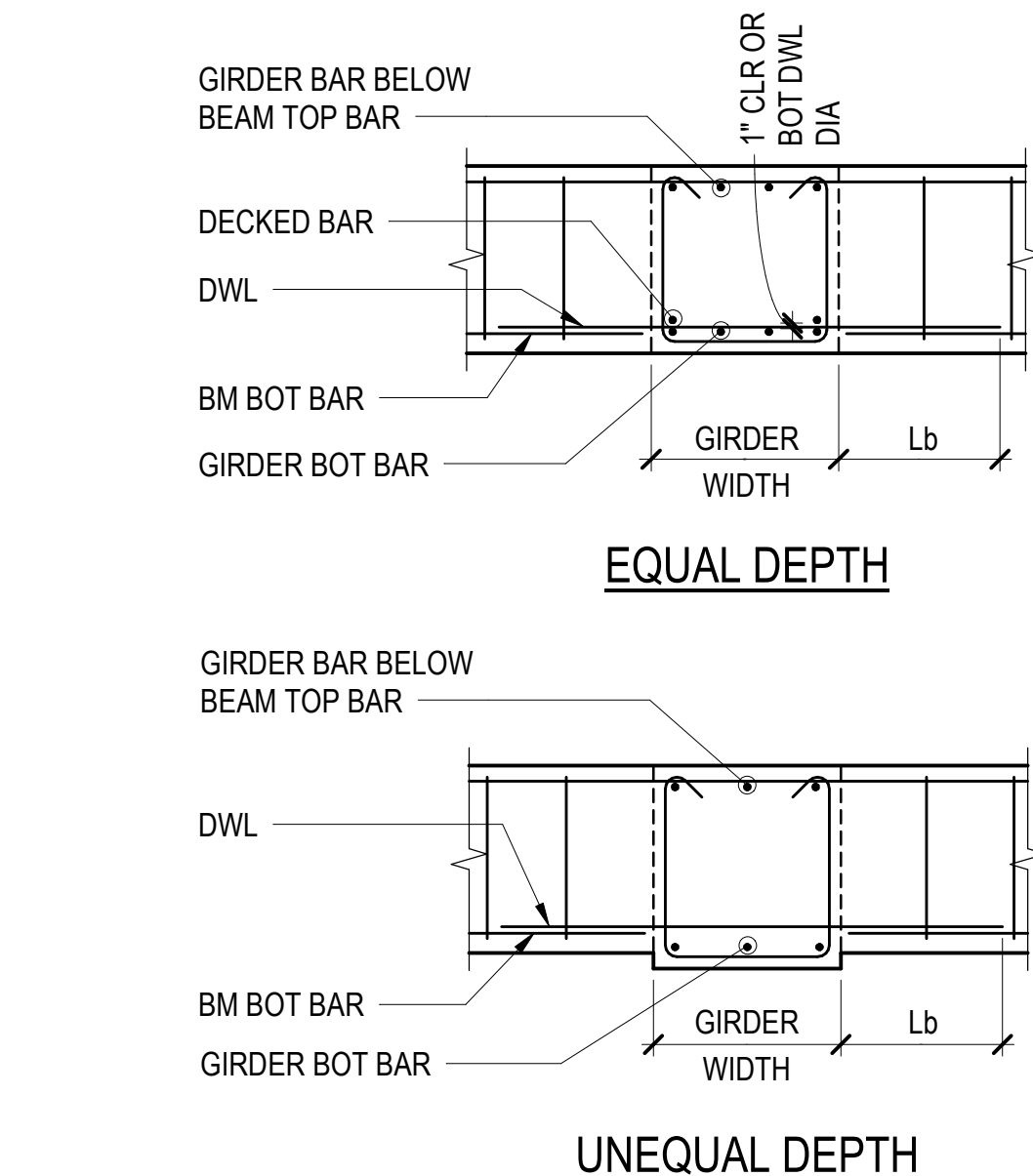


BEAM REINFORCING ELEVATION

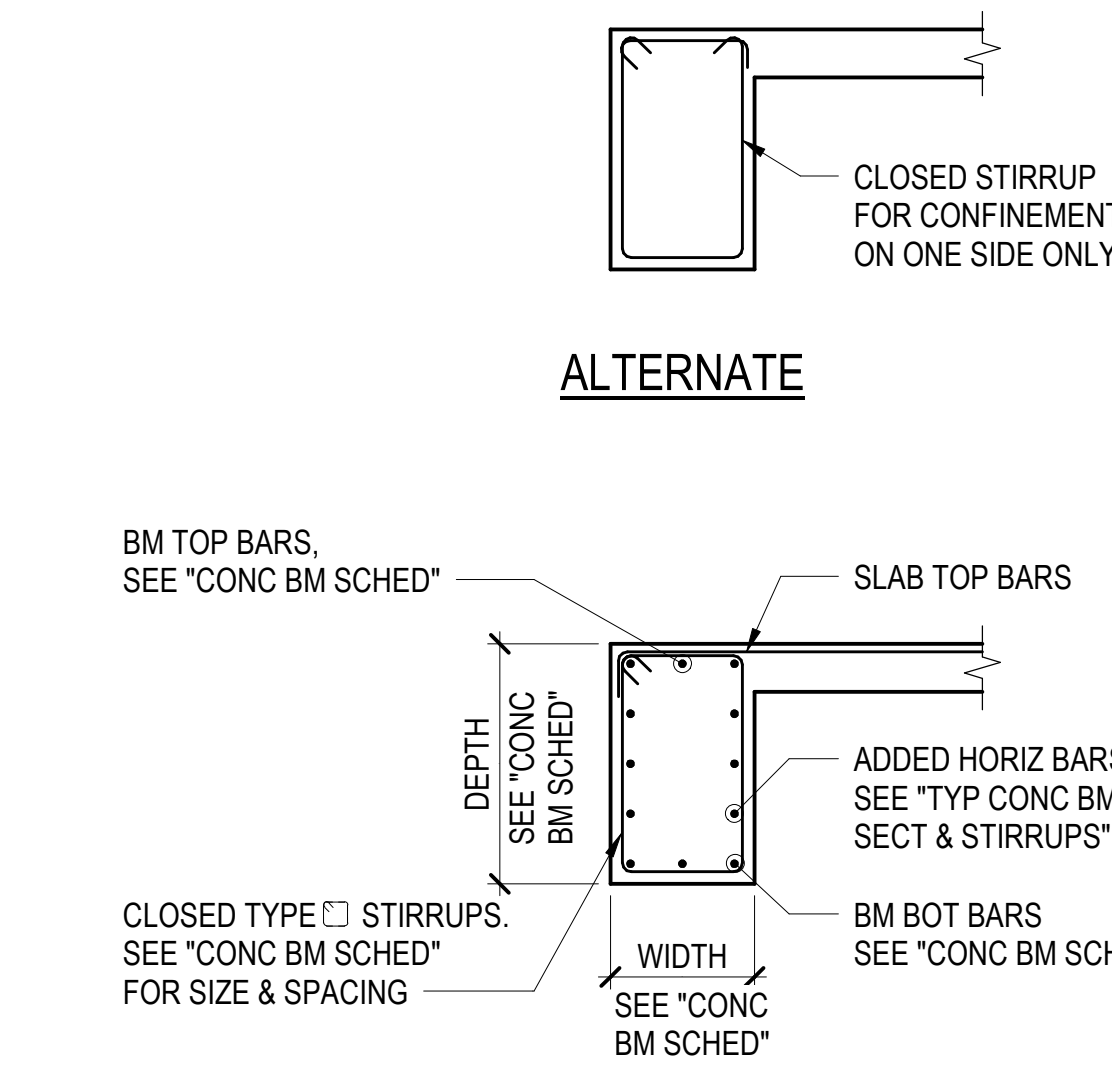
**11 TYPICAL CONCRETE BEAM**



**16 TYPICAL CONCRETE UPTURNED BEAM**

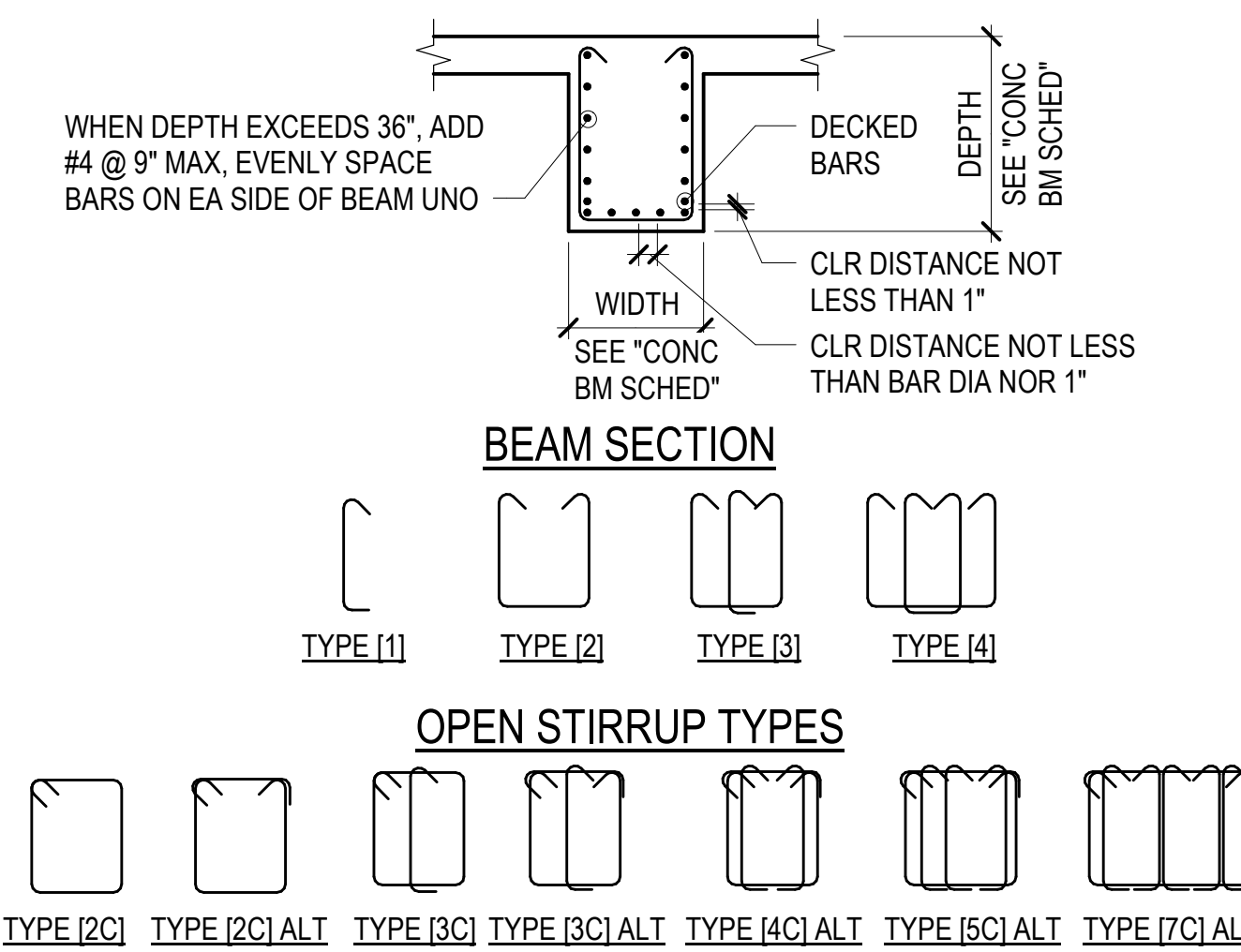


**17 TYP CONCR BM AND GIRDER INTERSECTION**



**18 TYPICAL CONCRETE EDGE BEAM**

**13 CONCRETE BEAM SCHEDULE**

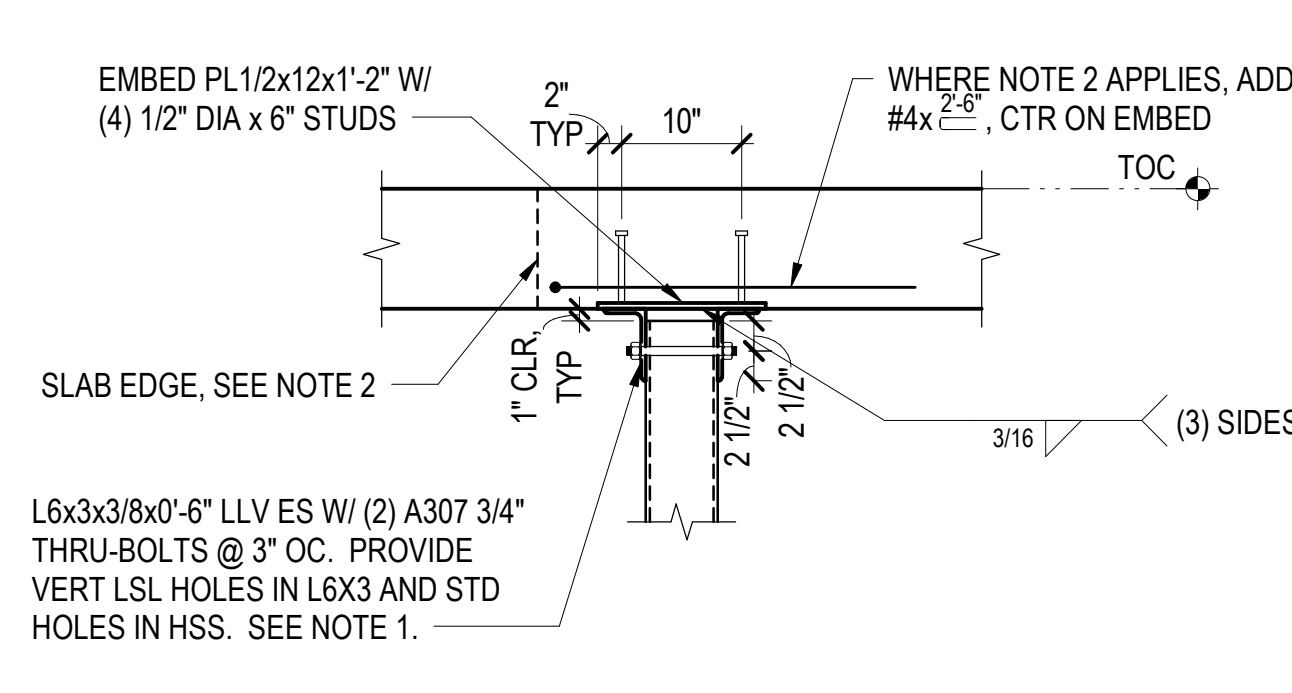


**NOTES:**

1. [ ] DENOTES TYPE OF STIRRUP REINFORCING CONFIGURATION. SEE "CONCRETE BEAM SCHEDULE."

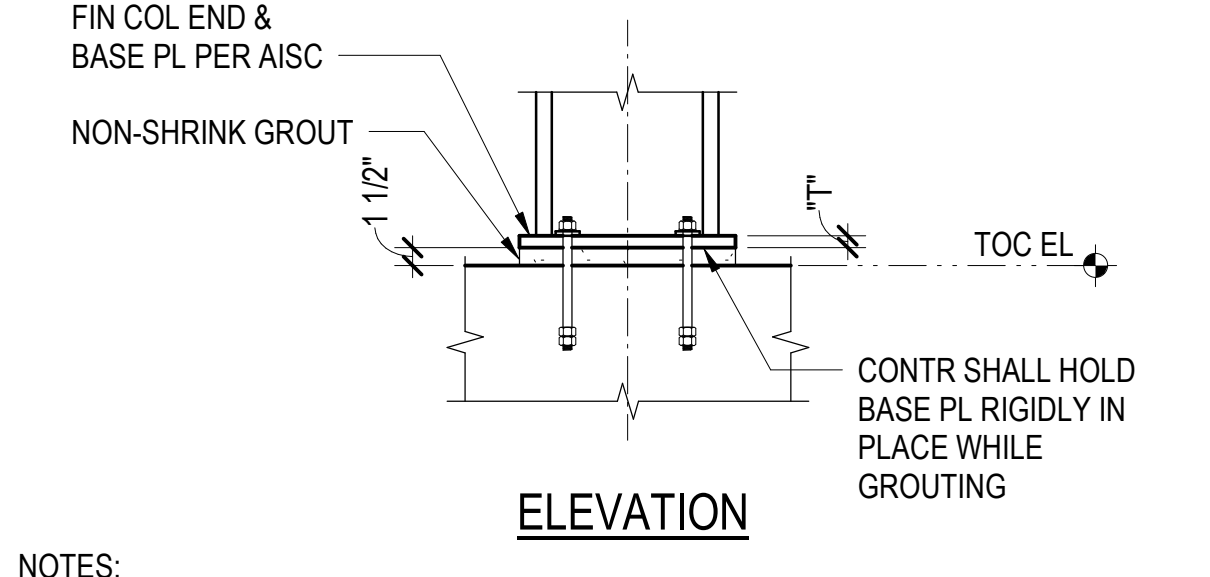
**19 TYP CONCR BEAM SECTION AND STIRRUPS**

- NOTES:**
- SEE "TYPICAL CONCRETE BEAM" DETAIL.
  - [ ] DENOTES TYPE OF REINFORCING CONFIGURATION. SEE "TYPICAL CONCRETE BEAM SECTION AND STIRRUPS" DETAIL FOR STIRRUP TYPE.



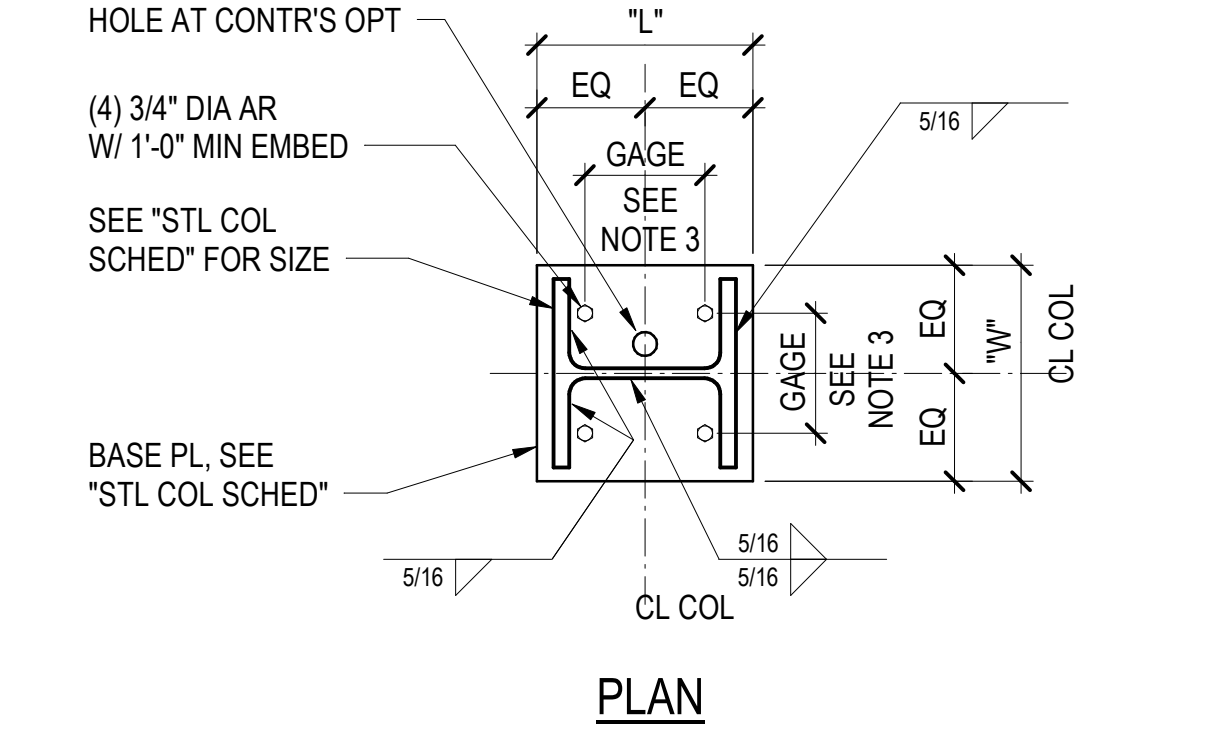
- NOTES:**
- WHERE NOTE APPLIES, BOLT TO BE CENTERED IN SLOTTED HOLE IN ANGLE. NUTS TO BE FINGER TIGHT. DAMAGE THREADS OF BOLT TO PREVENT BACK-OFF OF NUT.
  - PROVIDE U-BAR REINFORCEMENT PERPENDICULAR TO SLAB EDGE WHERE EMBED IS LOCATED 6" OR LESS FROM ADJACENT OPENING OR SLAB EDGE.

1 TYPICAL HSS POST TOP SLIP CONNECTION



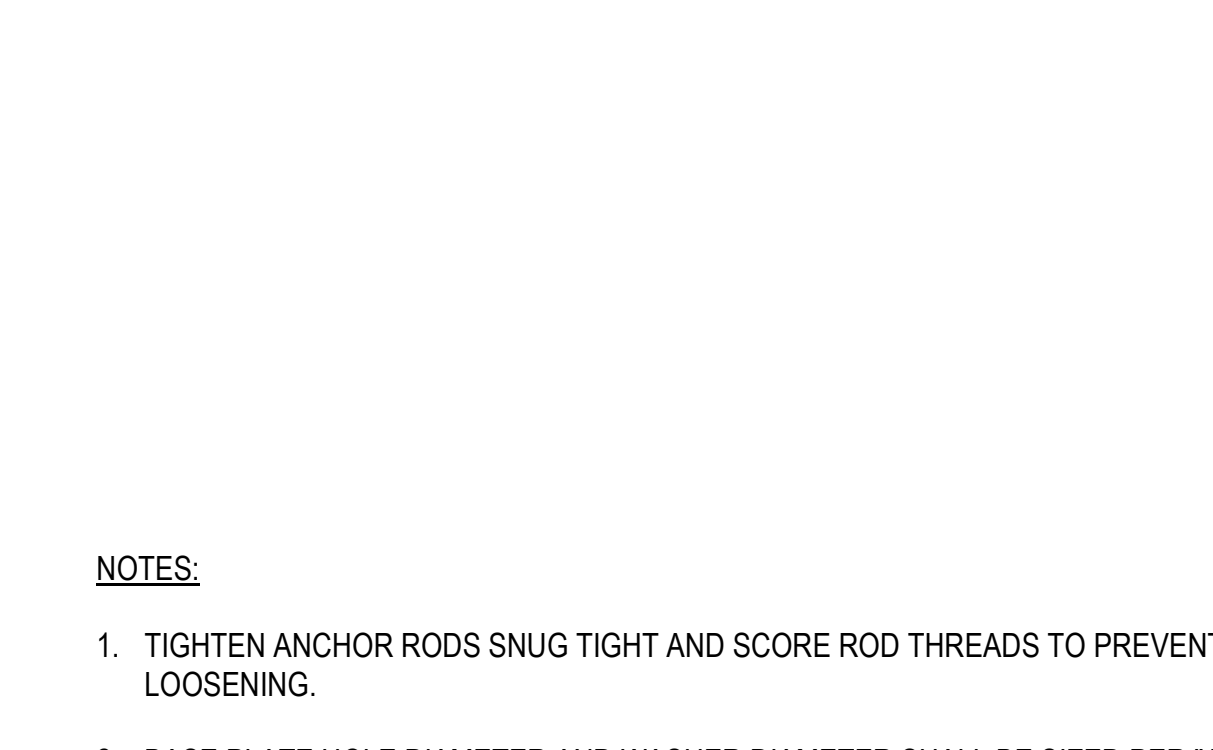
- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER AISC MANUAL - TABLE 14-2, UNLESS NOTED OTHERWISE.
  - ANCHOR ROD GAGE SHALL BE AS FOLLOWS:  
W10: 5 INCHES  
W12: 6 INCHES  
W14: 8 INCHES
- CONTRACTOR TO COORDINATE ANCHOR ROD GAGE WITH CONCRETE REINFORCING.

2 TYPICAL COLUMN BASE PLATE, TYPE 1



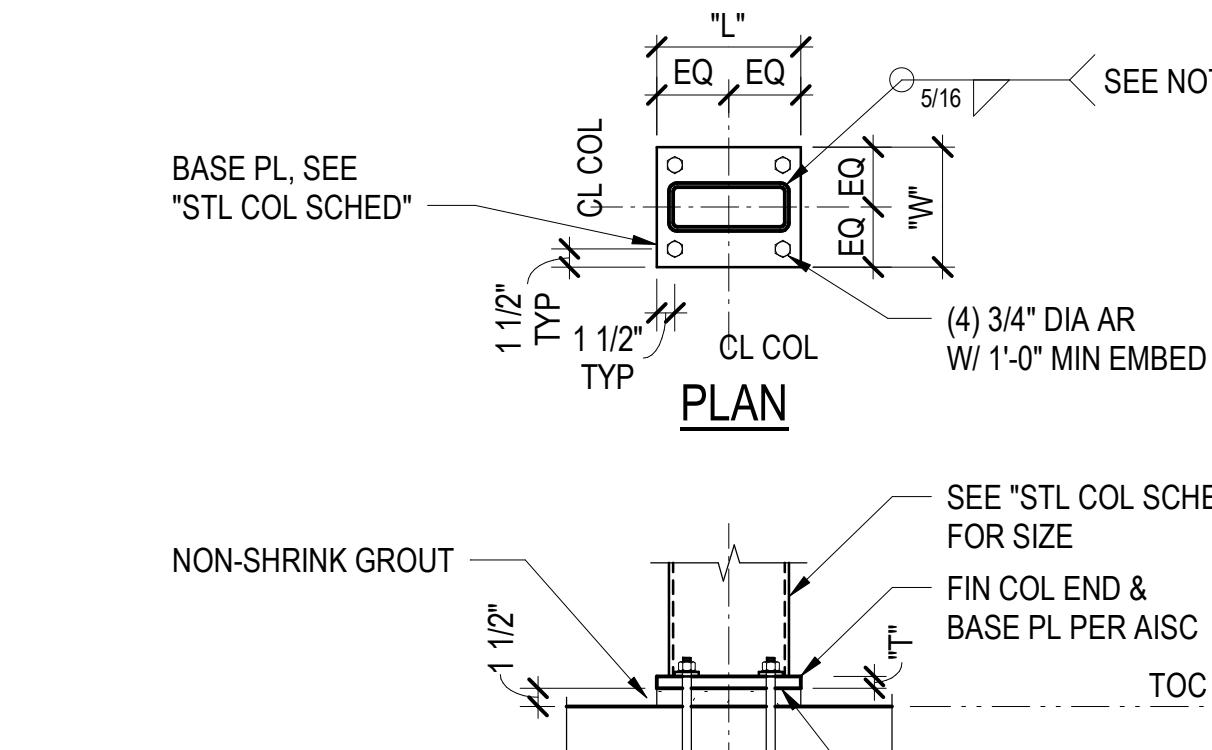
- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER AISC MANUAL-TABLE 14-2 UNLESS NOTED OTHERWISE.
  - DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
  - WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

3 TYPICAL COLUMN BASE PLATE, TYPE 2



- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER AISC MANUAL-TABLE 14-2 UNLESS NOTED OTHERWISE.
  - DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
  - WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

4 TYPICAL COLUMN BASE PLATE, TYPE 3



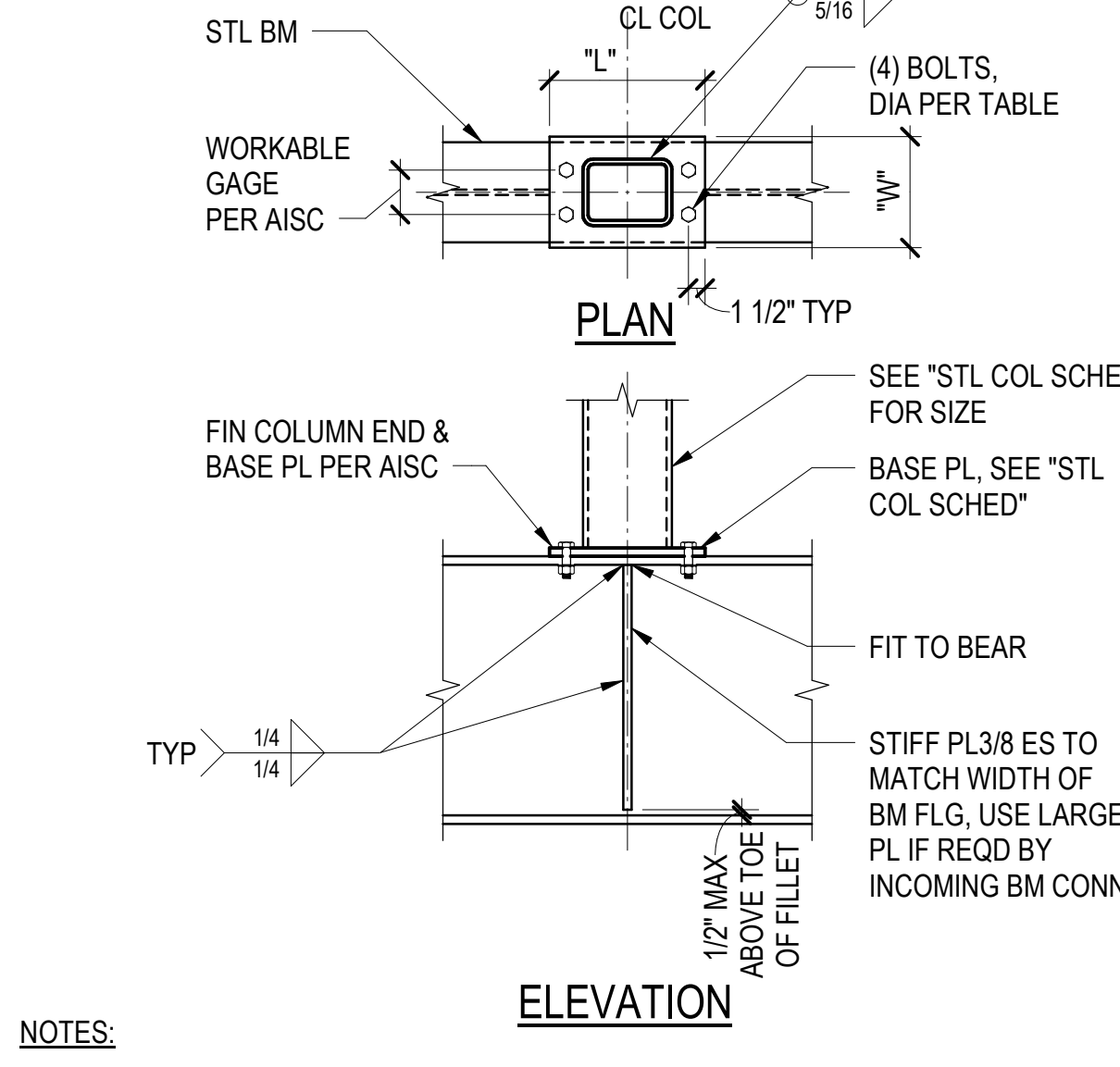
- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER AISC MANUAL-TABLE 14-2 UNLESS NOTED OTHERWISE.
  - DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
  - WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

5 TYPICAL COLUMN BASE PLATE, TYPE 4

**STEEL COLUMN SLAB PLATE SCHEDULE**

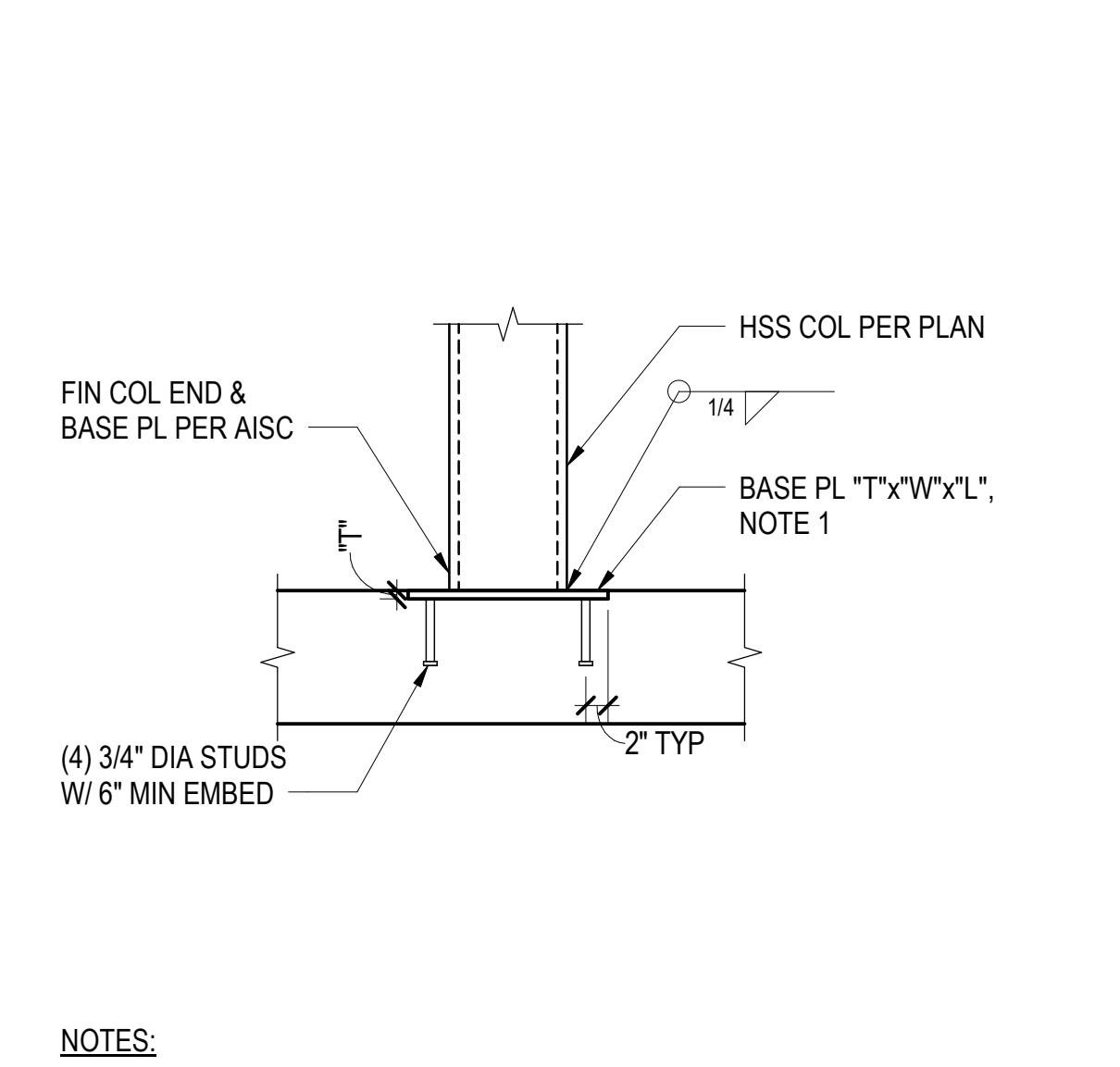
TYPE	BASE PLATE			EMBED PLATE		
	T1	L1	W1	T2	L2	W2
1	-	-	-	3/4	18	1'-6"
2	-	-	-	1	18	1'-6"
3	-	-	-	1 1/4	18	1'-6"
4	-	-	-	1 1/2	18	2'-0"
5	3/4	12	1'-0"	3/4	18	1'-6"
6	3/4	12	1'-0"	1 1/4	18	1'-6"
7	3/4	12	1'-0"	1	18	1'-6"
8	3/4	12	1'-2"	1 1/4	18	1'-6"
9	3/4	12	1'-0"	1 1/2	18	1'-6"
10	1	12	1'-2"	1 1/4	18	1'-6"
11	1	12	1'-2"	1 1/2	18	1'-6"
12	1	14	1'-4"	1 1/4	18	1'-6"
13	1	14	1'-4"	1 1/2	18	1'-6"
14	1	6	1'-2"	1	18	1'-6"
15	1	6	1'-2"	1 1/4	18	1'-6"
16	1 1/4	14	1'-4"	1 1/2	18	1'-6"
17	1 1/4	6	1'-2"	1 1/4	18	1'-6"
18	1 1/4	6	1'-6"	1 1/4	18	1'-6"
19	1 1/4	6	1'-6"	1 1/2	18	1'-6"
20	1 1/4	6	1'-6"	1 1/2	18	1'-8"
21	1 1/4	6	1'-6"	1 1/2	18	2'-0"
22	1 1/2	14	1'-4"	1 3/4	18	1'-6"
23	1 1/2	6	1'-8"	1 1/2	18	1'-8"
24	1 1/2	6	1'-8"	1 1/2	18	2'-0"
25	1 1/2	6	2'-0"	1 1/2	18	2'-0"
26	1 1/2	6	2'-0"	1 1/2	18	2'-2"
27	1 1/2	6	2'-0"	1 3/4	18	2'-0"
28	1 1/2	8	2'-2"	1 1/2	18	1'-6"
29	1 1/2	8	2'-2"	1 1/2	18	2'-2"
30	1 1/2	15	1'-7"	1 1/2	18	1'-6"
31	1 1/2	15	1'-7"	1 3/4	18	1'-6"
32	1	9	1'-3"	1 1/4	18	1'-6"

6 STEEL COLUMN SLAB PLATE SCHEDULE



- NOTES:**
- DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.

7 TYPICAL BASE PLATE, TYPE 6



- NOTES:**
- ALIGN BASE PLATE LONGER SIDE WITH LONGER SIDE OF HSS COLUMN.

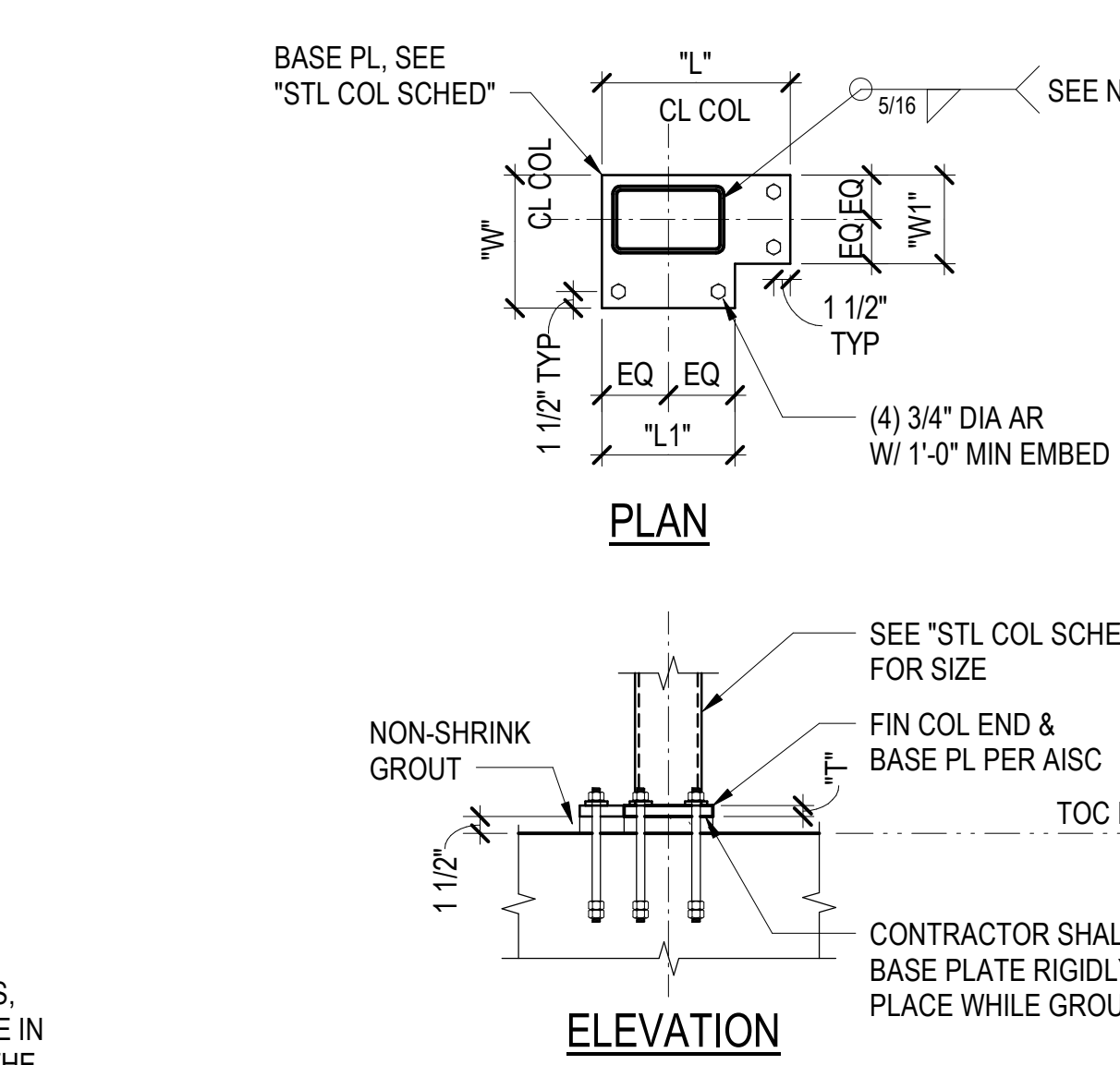
8 TYPICAL COLUMN BASE PLATE, TYPE 7

**TYPE 5 BASE PLATE SCHEDULE**

COLUMN SIZE	W1 (IN)	L1 (IN)
HSS8x8x1/2	10	10
HSS6x6x5/16	8	8

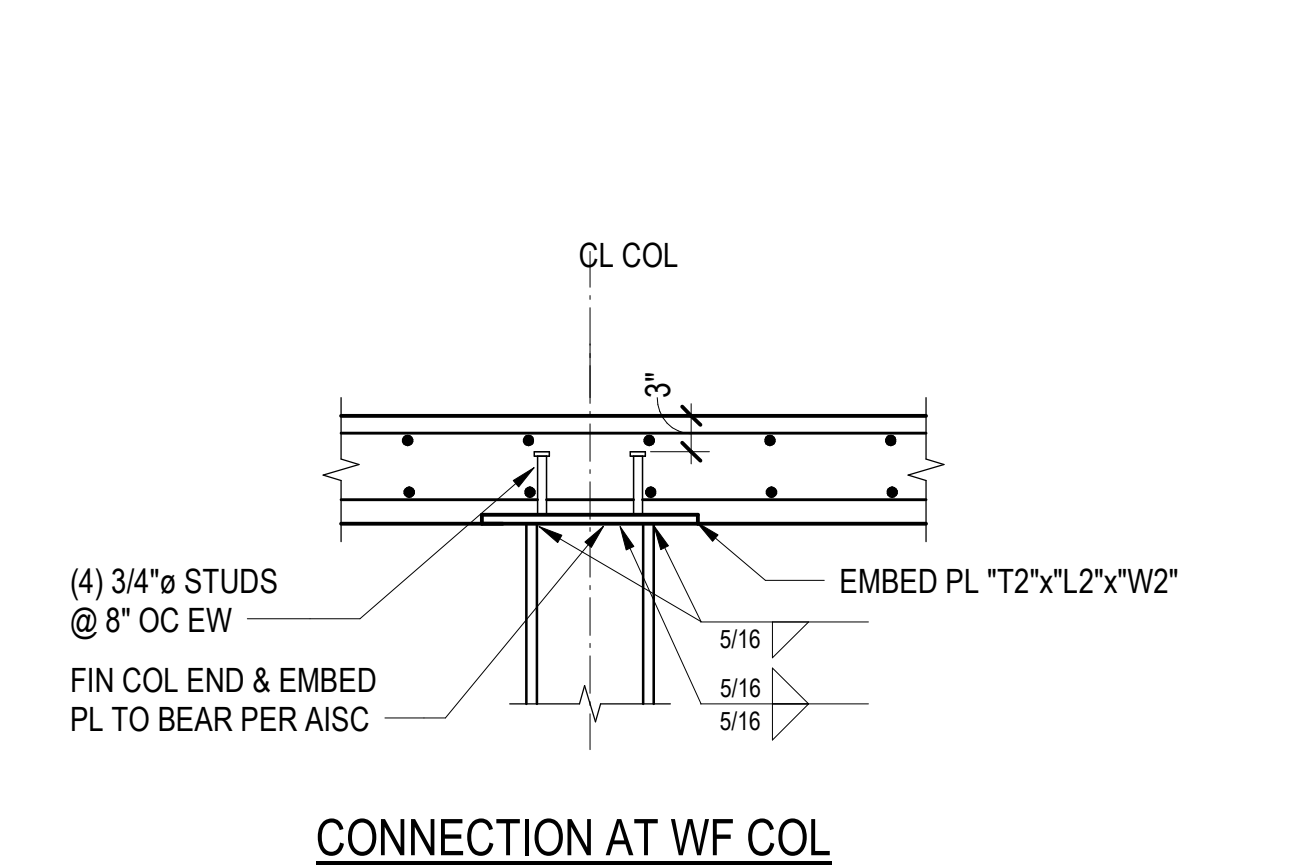
- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER AISC MANUAL-TABLE 14-2 UNLESS NOTED OTHERWISE.
  - DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
  - WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

9 TYPICAL COLUMN BASE PLATE, TYPE 5



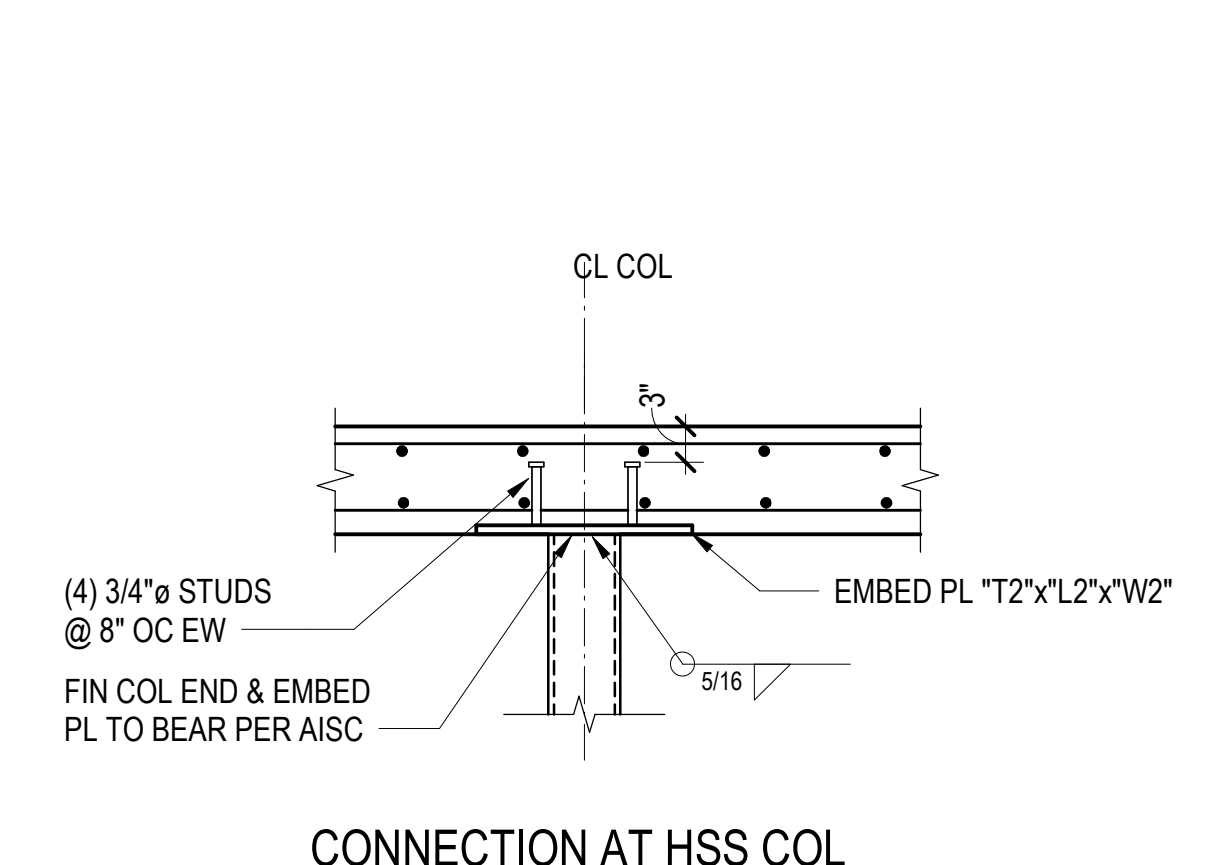
- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND WASHER DIAMETER SHALL BE SIZED PER AISC MANUAL-TABLE 14-2 UNLESS NOTED OTHERWISE.
  - DIMENSION "L" IS PARALLEL TO WIDE FACE OF HSS UNLESS NOTED OTHERWISE.
  - WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.

10 TYPICAL COLUMN BASE PLATE, TYPE 8



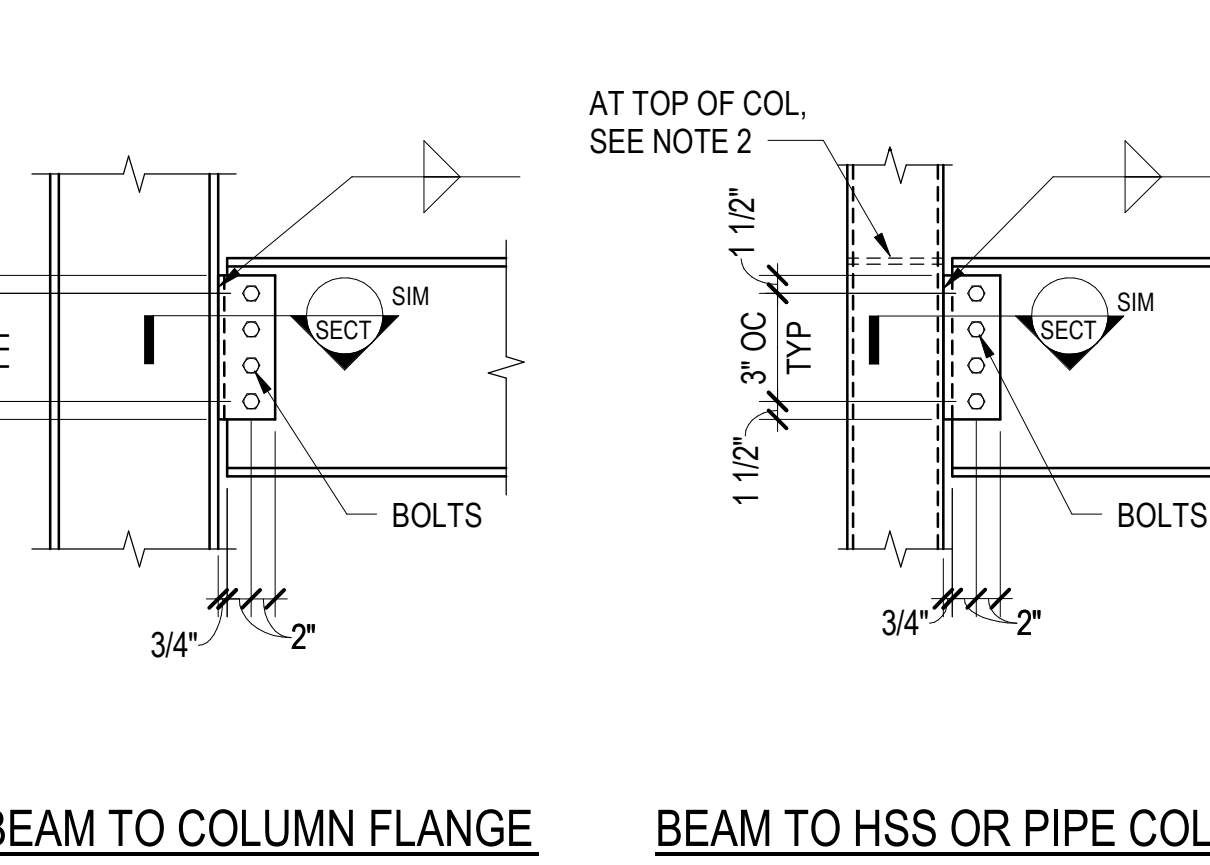
- NOTES:**
- SEE STEEL COLUMN SLAB PLATE SCHEDULE FOR PLATE DIMENSIONS.

11 TYPICAL TOP OF STEEL COLUMN SUPPORTING CONCRETE FRAMING



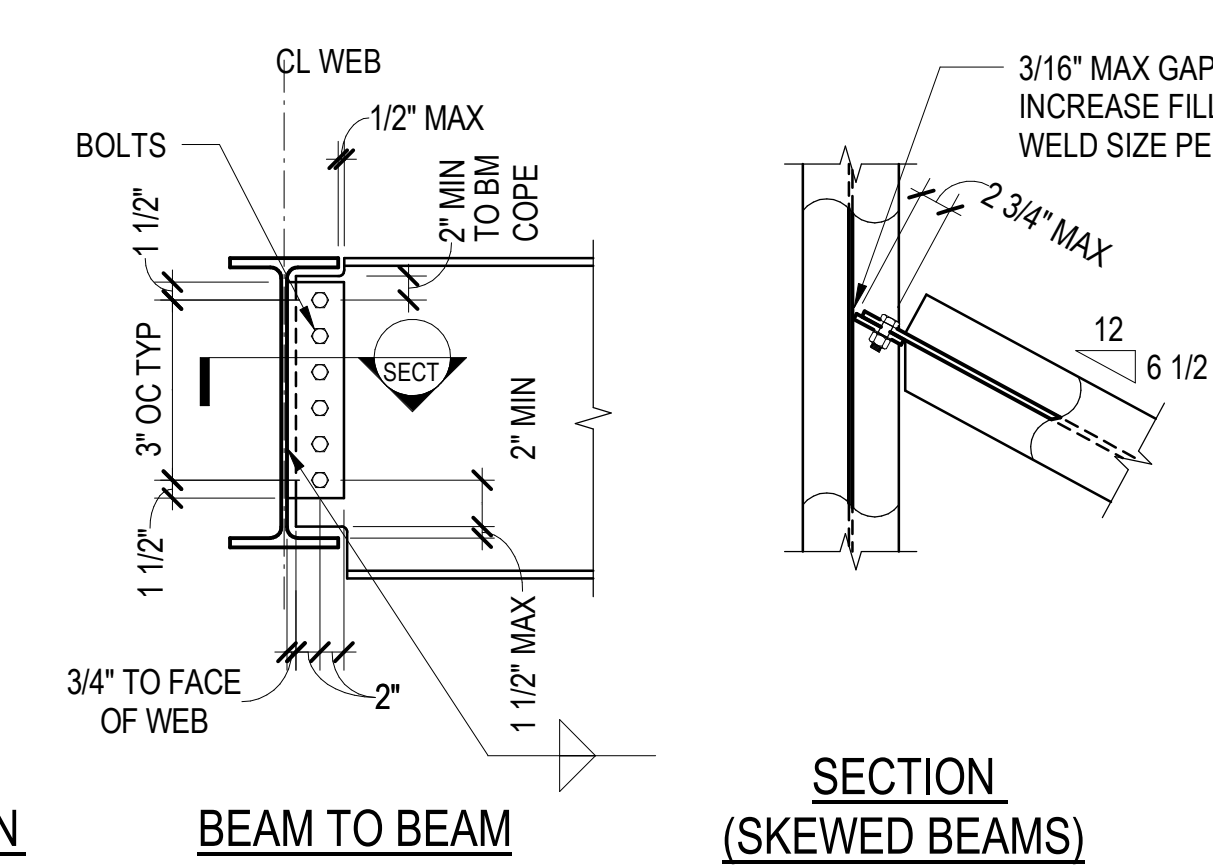
- NOTES:**
- SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
  - AT TOP OF HSS OR PIPE COLUMN, PROVIDE 1/2 INCH CAP PLATE WITH 5/16 INCH FILLET WELD ALL AROUND. IF BEAM IS SHOWN RUNNING OVER TOP OF COLUMN ON PLAN, SEE "TYPICAL BASE PLATE, TYPE 6" DETAIL.

12 TYPICAL BASE PLATE, TYPE 9



- NOTES:**
- SEE "GENERAL NOTES FOR STEEL CONNECTIONS" FOR ADDITIONAL INFORMATION.
  - AT TOP OF HSS OR PIPE COLUMN, PROVIDE 1/2 INCH CAP PLATE WITH 5/16 INCH FILLET WELD ALL AROUND. IF BEAM IS SHOWN RUNNING OVER TOP OF COLUMN ON PLAN, SEE "TYPICAL BASE PLATE, TYPE 6" DETAIL.

13 TYPICAL BEAM TO BEAM / BEAM TO COLUMN CONNECTION



- NOTES:**
- THIS DETAIL SHALL BE USED ONLY FOR BEAMS UP TO 8 INCHES DEEP AND WEIGHING AT LEAST 8 LB/FT. SEE "TYPICAL BEAM TO BEAM / BEAM TO COLUMN CONNECTION" FOR DEEPER BEAMS.
  - ALL PLATES SHALL HAVE Fy = 50 KSI MINIMUM.

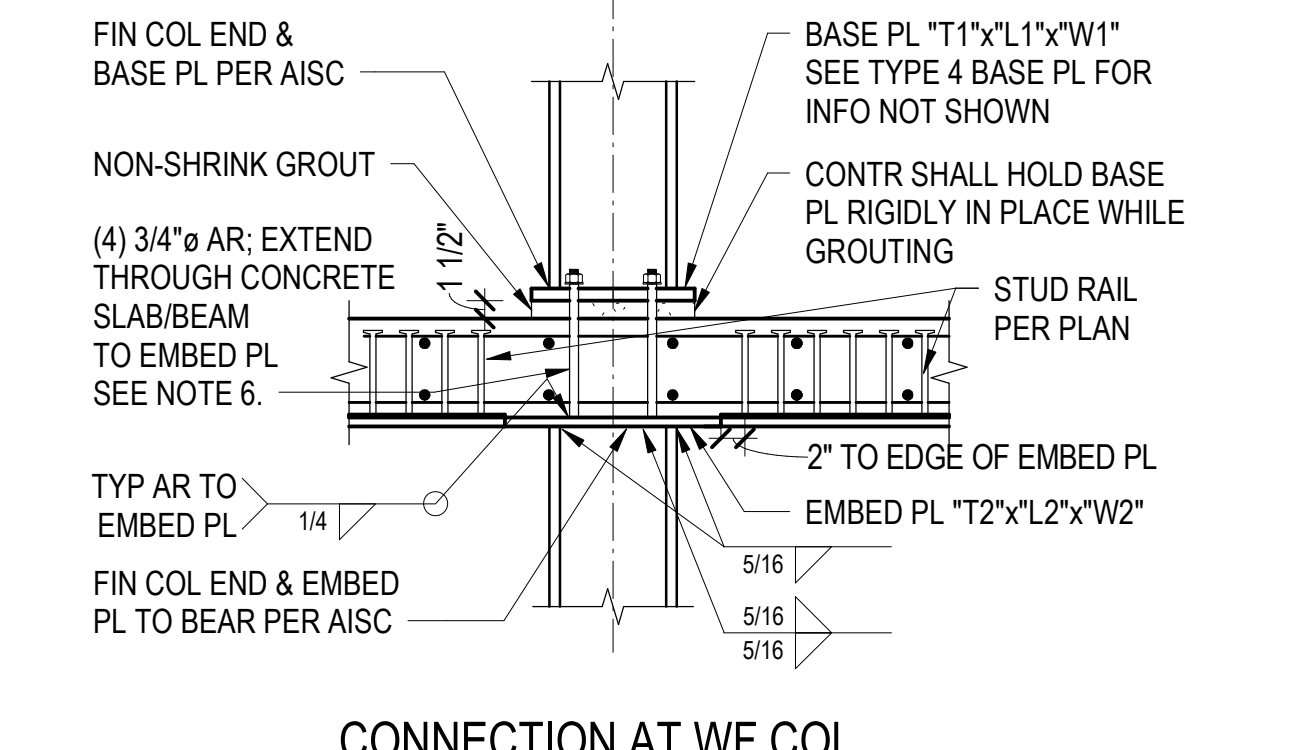
14 TYPICAL SHALLOW BEAM CONNECTION

**TABLE A**

WIDE-FLANGE BEAM DEPTH	NUMBER OF BOLTS REQUIRED	MAXIMUM REACTION (KIPS)
W10	2	27
W12, W14	3	40
W16, W18	4	65
W21	5	91
W24	6	124
W27	7	150
W30 - W44	8	175

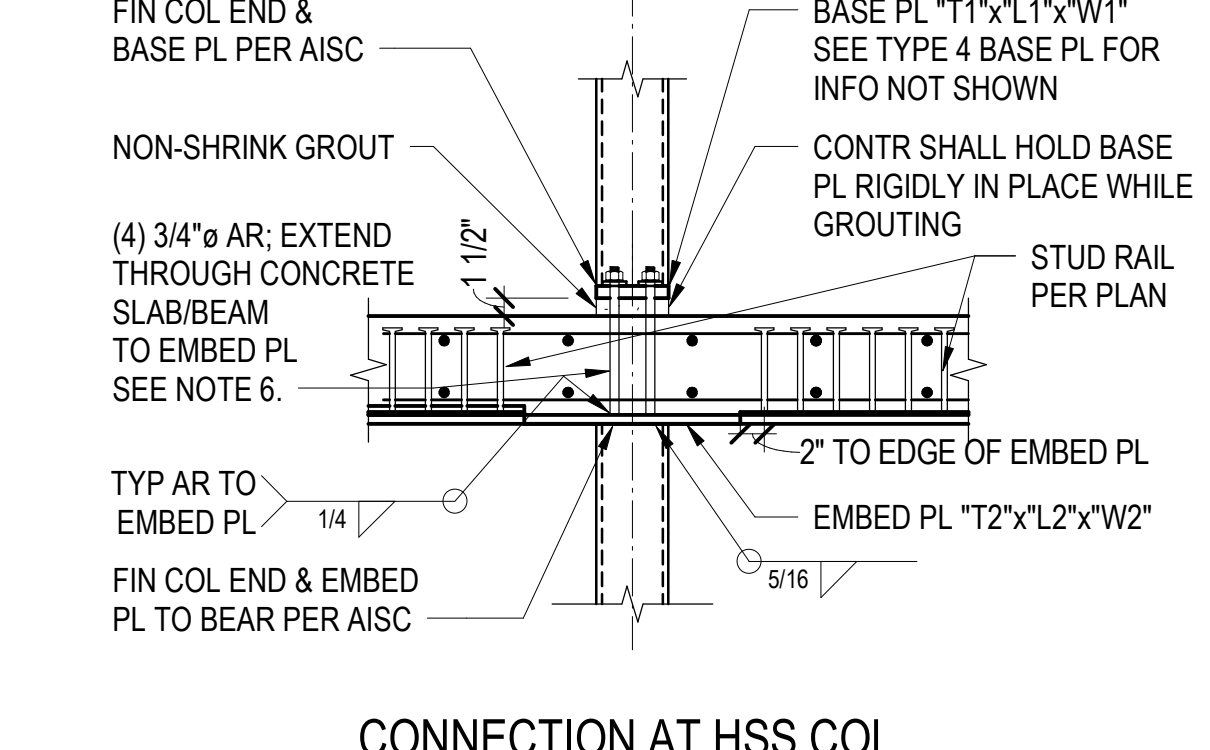
- NOTES BELOW APPLY TO ALL TYPICAL CONNECTIONS UNLESS NOTED OTHERWISE:**
- SEE PLANS FOR BEAM SIZE. UNLESS NOTED OTHERWISE, PROVIDE THE NUMBER OF 7/8 INCH DIAMETER GRADE A325 BOLTS SHOWN IN "TABLE A" BASED ON THE BEAM DEPTH.
  - SHEAR TAB PLATES SHALL BE GRADE 50 MATERIAL, AND BE 1/4 INCH THICK WITH 3/16 INCH WELD EACH SIDE FOR (2) BOLTS, 5/16 INCH THICK WITH 1/4 INCH WELD EACH SIDE FOR (3) BOLTS TO (5) BOLTS, AND 3/8 INCH THICK WITH 1/4 INCH WELD EACH SIDE FOR (6) BOLTS OR MORE.
  - BEAMS AND SHEAR TAB PLATES SHALL HAVE STANDARD ROUND HOLES (STD) UNLESS NOTED OTHERWISE. AT CONTRACTOR'S OPTION, HOLES IN SHEAR TAB PLATES MAY BE HORIZONTAL SHORT-SLOTTED HOLES.
  - WHEN CONDITIONS VARY FROM THOSE SHOWN IN THE TYPICAL DETAIL, DESIGN CONNECTIONS ACCORDING TO THE AISC MANUAL OF STEEL CONSTRUCTION.

15 GENERAL NOTES FOR STEEL CONNECTIONS



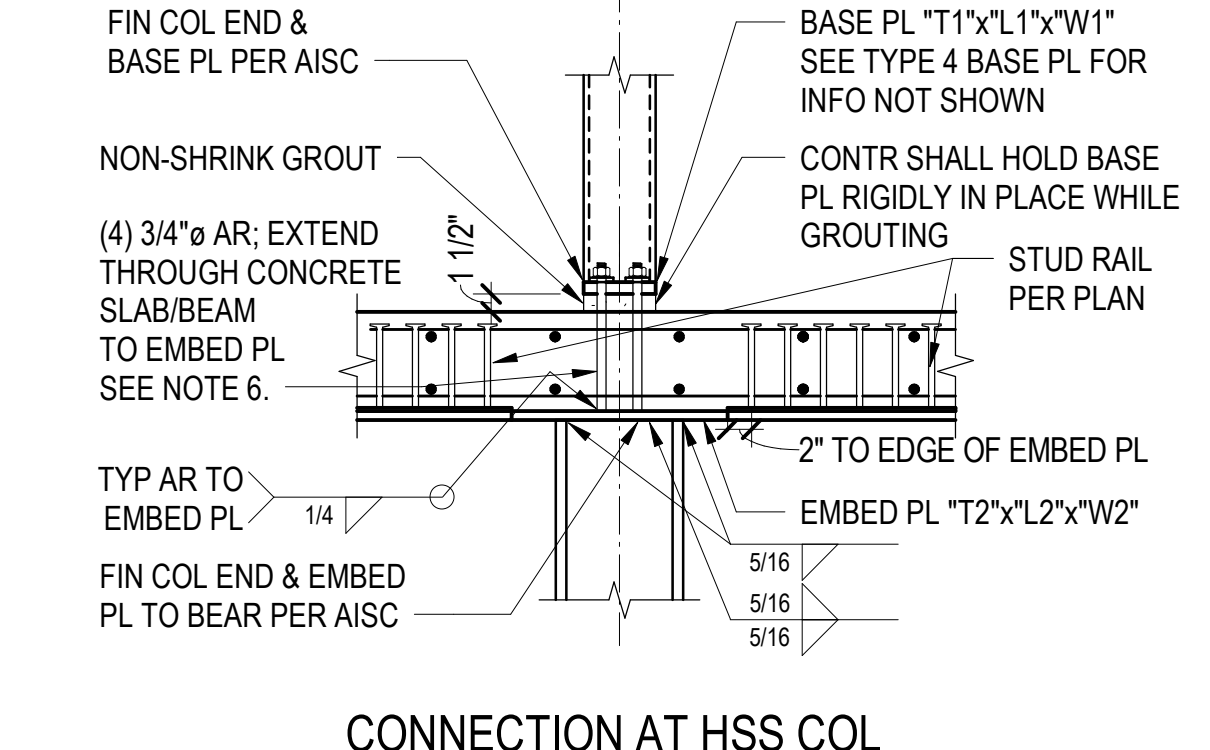
- NOTES:**
- TIGHTEN ANCHOR RODS SNUG TIGHT AND SCORE ROD THREADS TO PREVENT LOOSENING.
  - BASE PLATE HOLE DIAMETER AND PLATE WASHER SHALL BE SIZED PER AISC MANUAL -TABLE 14-2, UNLESS NOTED OTHERWISE.
  - SEE STEEL COLUMN SLAB PLATE SCHEDULE FOR PLATE DIMENSIONS.

16 TYPICAL STEEL COLUMN SUPPORTING CONCRETE FRAMING



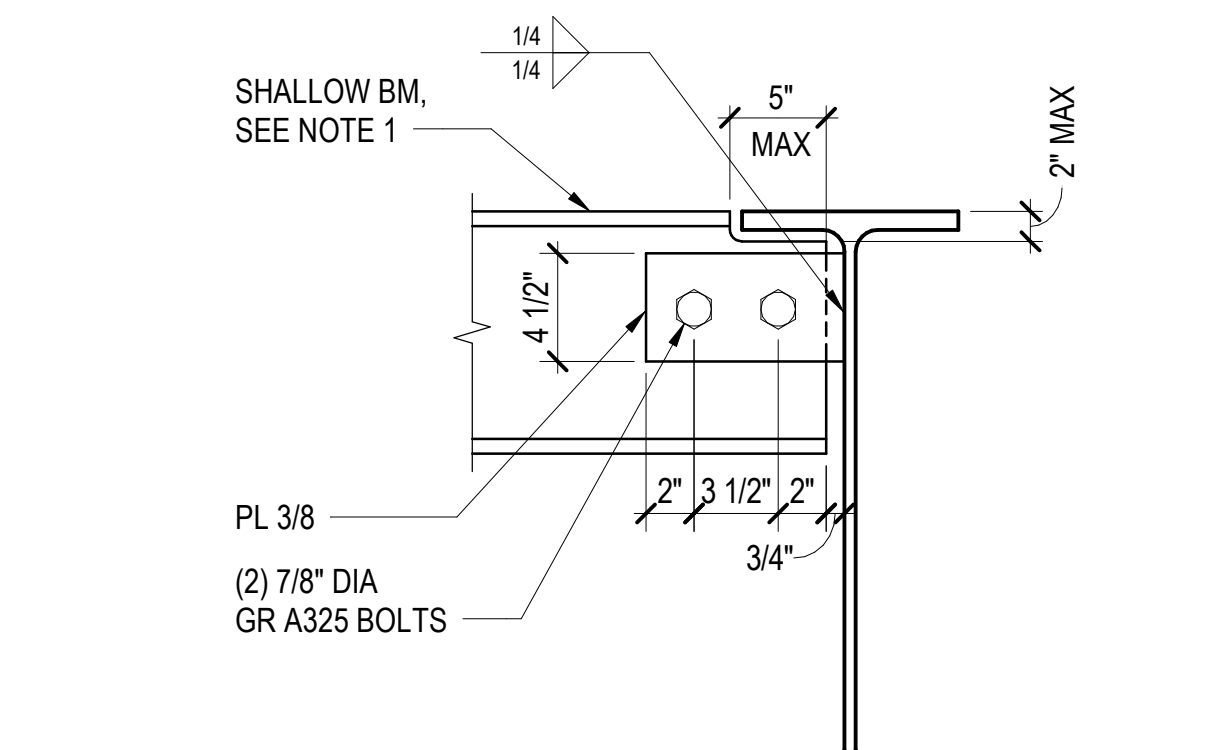
- NOTES:**
- WHERE COLUMN ABOVE IS WIDE FLANGE, ANCHOR ROD GAGE SHALL BE AS FOLLOWS:  
W10: 5 INCHES  
W12: 6 INCHES  
W14: 8 INCHES
  - WHERE COLUMN ABOVE IS HSS AND WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.
  - ANCHOR ROD TO BE SMOOTH SHANK THROUGH BEAM/SLAB THICKNESS.

17 TYPICAL STEEL COLUMN SUPPORTING CONCRETE FRAMING



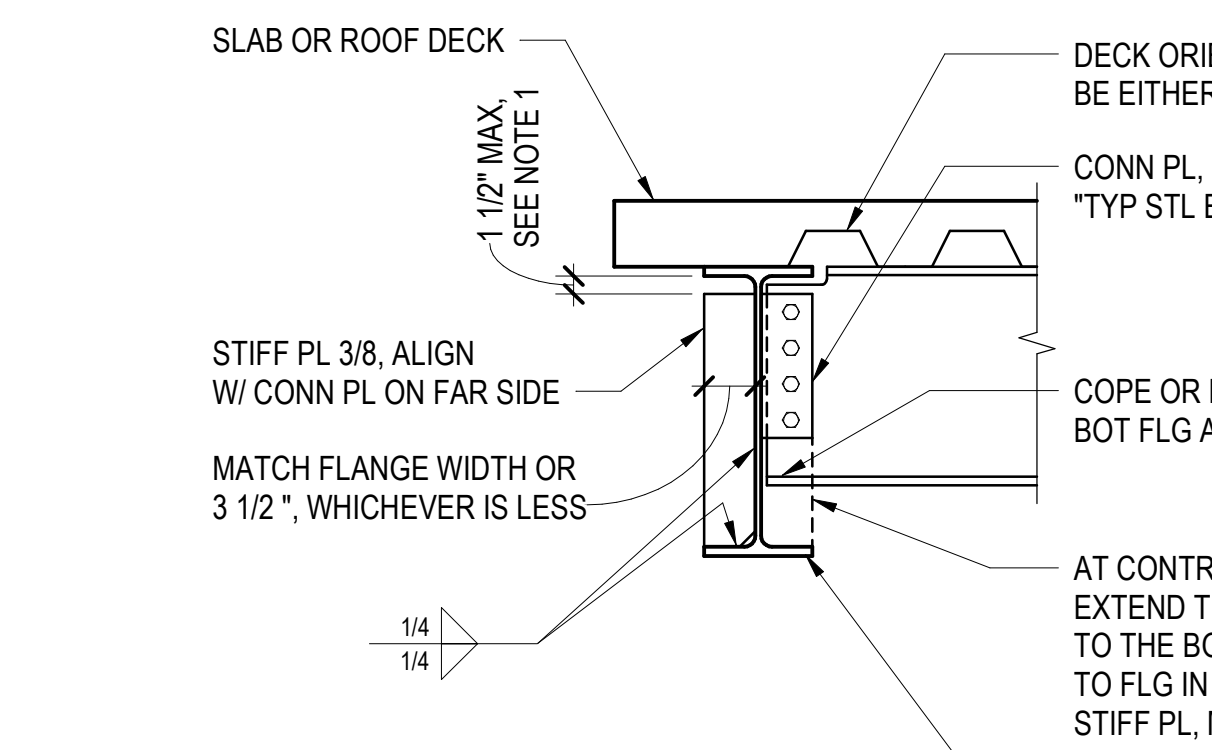
- NOTES:**
- WHERE COLUMN ABOVE IS WIDE FLANGE, ANCHOR ROD GAGE SHALL BE AS FOLLOWS:  
W10: 5 INCHES  
W12: 6 INCHES  
W14: 8 INCHES
  - WHERE COLUMN ABOVE IS HSS AND WHERE EDGE OF BASE PLATE IS LESS THAN 9/16 INCH FROM THE FACE OF THE HSS, PROVIDE A PARTIAL PENETRATION GROOVE WELD OF THE HSS TO THE BASE PLATE IN LIEU OF FILLET WELD ON THAT FACE OF THE HSS. GROOVE WELD SIZE SHALL BE THE THICKNESS OF THE HSS WALL OR 5/16 INCH, WHICHEVER IS LESS.
  - ANCHOR ROD TO BE SMOOTH SHANK THROUGH BEAM/SLAB THICKNESS.

18 TYPICAL STEEL COLUMN SUPPORTING CONCRETE FRAMING



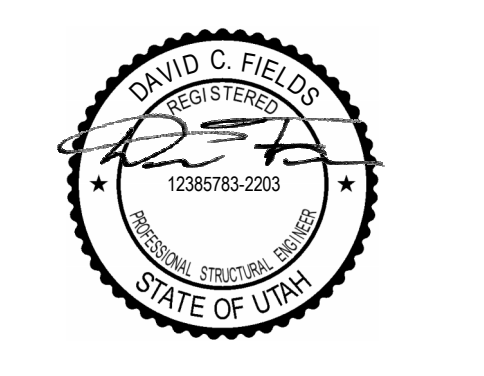
- NOTES:**
- AT LOCATIONS WHERE A CONCRETE SLAB DOES NOT EXIST AT EDGE BEAM, THE STIFFENER PLATE OR CONNECTION PLATE SHALL BE EXTENDED TO FULL DEPTH AND WELDED ON THREE SIDES.
  - THIS DETAIL APPLIES AT ALL EDGE OF SLAB CONDITIONS.

19 TYPICAL STEEL EDGE BEAM STIFFENER



- NOTES:**
- AT LOCATIONS WHERE A CONCRETE SLAB DOES NOT EXIST AT EDGE BEAM, THE STIFFENER PLATE OR CONNECTION PLATE SHALL BE EXTENDED TO FULL DEPTH AND WELDED ON THREE SIDES.
  - THIS DETAIL APPLIES AT ALL EDGE OF SLAB CONDITIONS.

20 TYPICAL STEEL EDGE BEAM STIFFENER



Reserved for permit stamp

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principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
job no. 20052  
date 05/17/2024

revisions:  
3 8/19/2024 ASI-004  
2 7/26/2024 ASI-002  
1 05/17/2024 IFC 2  
04/08/2024 IFC SET 1 OF 3  
11/18/2022 95% CD  
no. date by

IFC SET 2 OF 3  
05/17/2024

TYPICAL STEEL DETAILS

S4.11





TOWER A - ROOF STEEL COLUMN SCHEDULE																																	
TOWER A - ROOF																					TOWER A - ROOF												
8419' - 6"																					8419' - 6"												
TOWER B - LEVEL 5																					TOWER B - LEVEL 5												
8412' - 0"																					8412' - 0"												
TOWER A - LEVEL 6																					TOWER A - LEVEL 6												
8407' - 6"																					8407' - 6"												
TOWER A - LEVEL 5																					TOWER A - LEVEL 5												
8395' - 0"																					8395' - 0"												
Column Locations	SCA12	SCA13	SCA14	SCA15	SCA17	SCA18	SCA19	SCA20	SCA22	SCA23	SCA24	SCA25	SCA26	SCA27	SCA28	SCA29	SCA30	SCA31	SCA32	SCA33	SCA34	SCA35	SCA36	SCA37	SCA38	SCA39	SCA40	SCA41	SCA42	SCA43	SCA44	SCA45	SCA46

TOWER A - ROOF STEEL COLUMN SCHEDULE

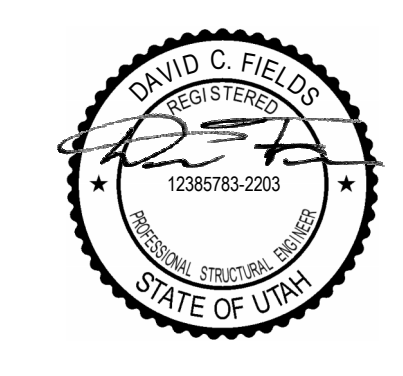
TOWER A - STEEL COLUMN SCHEDULE																					
TOWER A - LEVEL 6																					TOWER A - LEVEL 6
8407' - 6"																					8407' - 6"
TOWER A - LEVEL 5																					TOWER A - LEVEL 5
8395' - 0"																					8395' - 0"
TOWER A - LEVEL 4																					TOWER A - LEVEL 4
8383' - 0"																					8383' - 0"
TOWER A - LEVEL 3																					TOWER A - LEVEL 3
8371' - 0"																					8371' - 0"
TOWER A - LEVEL 2																					TOWER A - LEVEL 2
8359' - 0"																					8359' - 0"
TOWER A - LEVEL 1																					TOWER A - LEVEL 1
8345' - 0"																					8345' - 0"
PARKING LEVEL 2																					PARKING LEVEL 2
8333' - 0"																					8333' - 0"
Column Locations	SCA1	SCA2	SCA3	SCA4	SCA5	SCA6	SCA13	SCA7	SCA8	SCA9	SCA10	SCA11	SCA12								

- NOTES:
- BASE PLATES SHALL HAVE F<sub>y</sub> = 50 KSI, UNLESS NOTED OTHERWISE.
  - X INDICATES CONNECTION OF STEEL COLUMN TO CONCRETE SLAB. SEE TYPICAL TOP OF STEEL COLUMN SUPPORTING CONCRETE FRAMING DETAIL. TYPICAL STEEL COLUMN SUPPORTING CONCRETE FRAMING DETAIL, AND STEEL COLUMN SLAB PLATE SCHEDULE ON S4.11

TOWER A - STEEL COLUMN SCHEDULE

TOWER AB CONNECTOR - STEEL COLUMN SCHEDULE																																		
TOWER A - LEVEL 2																					TOWER A - LEVEL 2													
8359' - 0"																					8359' - 0"													
TOWER A - LEVEL 1																					TOWER A - LEVEL 1													
8345' - 0"																					8345' - 0"													
Column Locations	SC1	SC2	SC3	SC4	SC5	SC6	SC7	SC8	SC9	SC10	SC11	SC12	SC13	SC14	SC15	SC16	SC17	SC18	SC19	SC20	SC21	SC22	SC23	SC24	SC25	SC26	SC27	SC28	SC29	SC30	SC31	SC32	SC33	SC34

TOWER AB CONNECTOR - STEEL COLUMN SCHEDULE



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DEER VALLEY, UTAH

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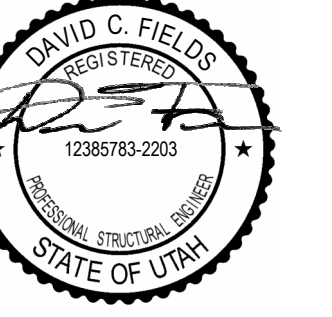
principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
job no. 20052  
date 05/17/2024

revisions:  
3 8/19/2024 AS-004  
1 03/12/2024 IFC 2  
04/08/2024 IFC SET 1 OF 3  
11/18/2022 95% CD

IFC SET 2 OF 3

05/17/2024

TOWER A STEEL COLUMN SCHEDULE  
**S4.A.10**



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**Olson Kundig**

project  
**SOMMET BLANC - ABC**  
DEER VALLEY, UTAH

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principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
job no. 20052  
date 05/17/2024

revisions:

3 8/19/2024 ASI.004  
04/08/2024 IFC SET 1 OF 3  
11/18/2022 95% CD

no. date by

IFC SET 2 OF 3

05/17/2024

**TOWER B STEEL COLUMN SCHEDULE**

**S4.B.10**

TOWER B - ROOF STEEL COLUMN SCHEDULE																														
TOWER B - ROOF 8448' - 9"	W10x45	HSS6x6x5/8	HSS6x6x5/8	W10x45	W10x45	HSS6x6x3/8	HSS6x6x5/8	HSS6x6x5/8	HSS6x6x5/16	W10x45	HSS6x6x3/8	W10x49	HSS6x6x3/8	W10x49	W10x45	W10x45	W10x45	W10x49	W10x49	HSS6x6x5/8	HSS6x6x5/8	HSS6x6x1/2	HSS6x6x1/2	HSS6x6x1/2	HSS6x6x5/8	HSS6x6x5/8	HSS6x6x5/16	HSS6x6x1/2	HSS6x6x1/2	TOWER B - ROOF 8448' - 9"
TOWER B - LEVEL 7 8436' - 6"	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x10x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x10x1'-0" 4/S4.11	TOWER B - LEVEL 7 8436' - 6"
Column Locations	SCB1	SCB2	SCB3	SCB4	SCB5	SCB6	SC7	SCB8	SCB9	SCB10	SCB11	SCB12	SCB13	SCB14	SCB15	SCB16	SCB17	SCB18	SCB19	SCB20	SCB21	SCB22	SCB23	SCB24	SCB25	SCB26	SCB27	SCB28	TOWER B - LEVEL 7 8436' - 6"	

TOWER B - ROOF STEEL COLUMN SCHEDULE																
TOWER B - ROOF 8448' - 9"	HSS6x6x3/8	HSS6x6x5/16	HSS6x6x1/4	HSS6x6x5/8	HSS6x6x5/16	W10x45	W10x45	W10x45	W10x45	W10x45	W10x45	W10x45	W10x45	W10x45	HSS6x6x3/8	TOWER B - ROOF 8448' - 9"
TOWER B - LEVEL 7 8436' - 6"	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 1x8x1'-0" 4/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 1x8x1'-0" 4/S4.11	TOWER B - LEVEL 7 8436' - 6"
Column Locations	SCB29	SCB30	SCB31	SCB32	SCB33	SCB34	SCB35	SCB36	SCB37	SCB38	SCB39	SCB40	SCB41	SCB42		

**TOWER B - ROOF STEEL COLUMN SCHEDULE**

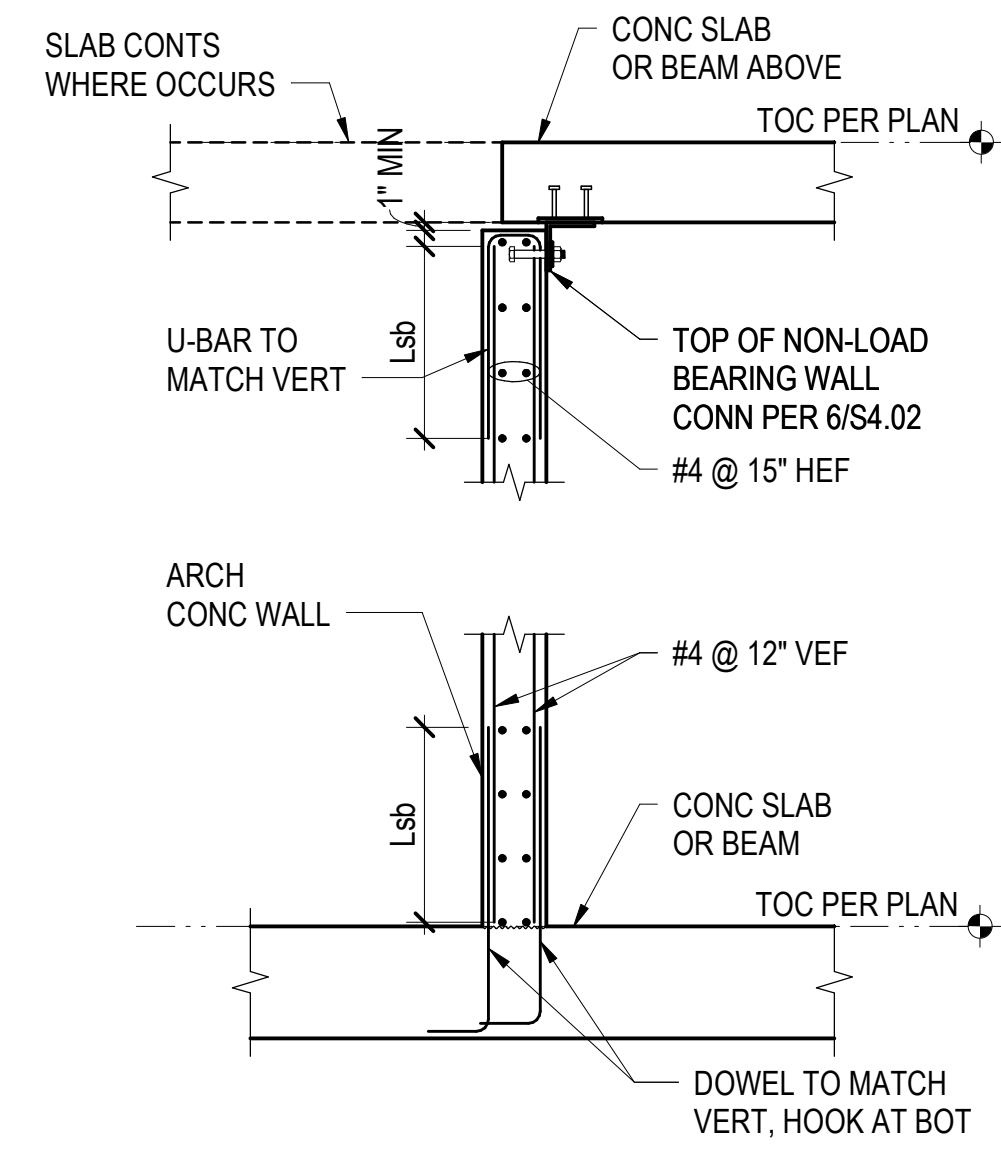
TOWER B - STEEL COLUMN SCHEDULE																	
TOWER B - LEVEL 7 8436' - 6"	W10x49	HSS6x6x1/2	HSS6x6x1/2	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x77	TOWER B - LEVEL 7 8436' - 6"
TOWER B - LEVEL 6 8424' - 0"	W10x49	HSS6x6x1/2	HSS6x6x1/2	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x112	TOWER B - LEVEL 6 8424' - 0"
TOWER B - LEVEL 5 8412' - 0"	W10x49	HSS12x6x1/2	HSS9x6x1/2	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W10x49	W12x120	TOWER B - LEVEL 5 8412' - 0"
TOWER B - LEVEL 4 8400' - 0"	W10x60	HSS12x6x1/2	HSS9x6x1/2	W10x60	W10x49	W10x60	W10x49	W10x60	W10x49	W10x60	W10x49	W10x60	W10x49	W10x60	W10x60	W12x152	TOWER B - LEVEL 4 8400' - 0"
TOWER B - LEVEL 3 8388' - 0"	W10x77	HSS12x6x3/8	HSS9x6x1/2	W10x68	W10x60	W10x68	W10x49	W10x68	W10x49	W10x68	W10x49	W10x68	W10x49	W10x68	W10x77	W12x152	TOWER B - LEVEL 3 8388' - 0"
TOWER B - LEVEL 2 8376' - 0"	W10x77	HSS6x6x3/8	HSS9x6x1/2	W10x112	W10x77	W10x112	W10x77	W10x112	W10x77	W10x112	W10x77	W10x112	W10x77	W10x112	W10x112	W12x170	TOWER B - LEVEL 2 8376' - 0"
TOWER B - LEVEL 1 8357' - 0"	W12x136	HSS6x6x3/8	HSS9x6x1/2	W10x112	W10x77	W10x112	W10x77	W10x112	W10x77	W10x112	W10x77	W10x112	W10x77	W10x112	W10x112	W12x170	TOWER B - LEVEL 1 8357' - 0"
PARKING LEVEL 1 8345' - 0"																	PARKING LEVEL 1 8345' - 0"
PARKING LEVEL 2 8333' - 0"	BASE PL 1 1/4x14x1'-3" 2/S4.11	BASE PL 1 1/2x6x1'-8" 4/S4.11	BASE PL 1 1/2x15x1'-3" 9/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 3/4x12x1'-0" 2/S4.11	BASE PL 1 1/2x14x1'-4" 2/S4.11	8333' - 0"	
Column Locations	SCB1	SCB2	SCB3	SCB4	SCB5	SCB6	SCB7	SCB8	SCB9	SCB10							

- NOTES:
- BASE PLATES SHALL HAVE F<sub>y</sub> = 50 KSI, UNLESS NOTED OTHERWISE.
  - X INDICATES CONNECTION OF STEEL COLUMN TO CONCRETE SLAB. SEE "TYPICAL TOP OF STEEL COLUMN SUPPORTING CONCRETE FRAMING" DETAIL, "TYPICAL STEEL COLUMN SUPPORTING CONCRETE FRAMING" DETAIL, AND "STEEL COLUMN SLAB PLATE SCHEDULE" ON S4.11.

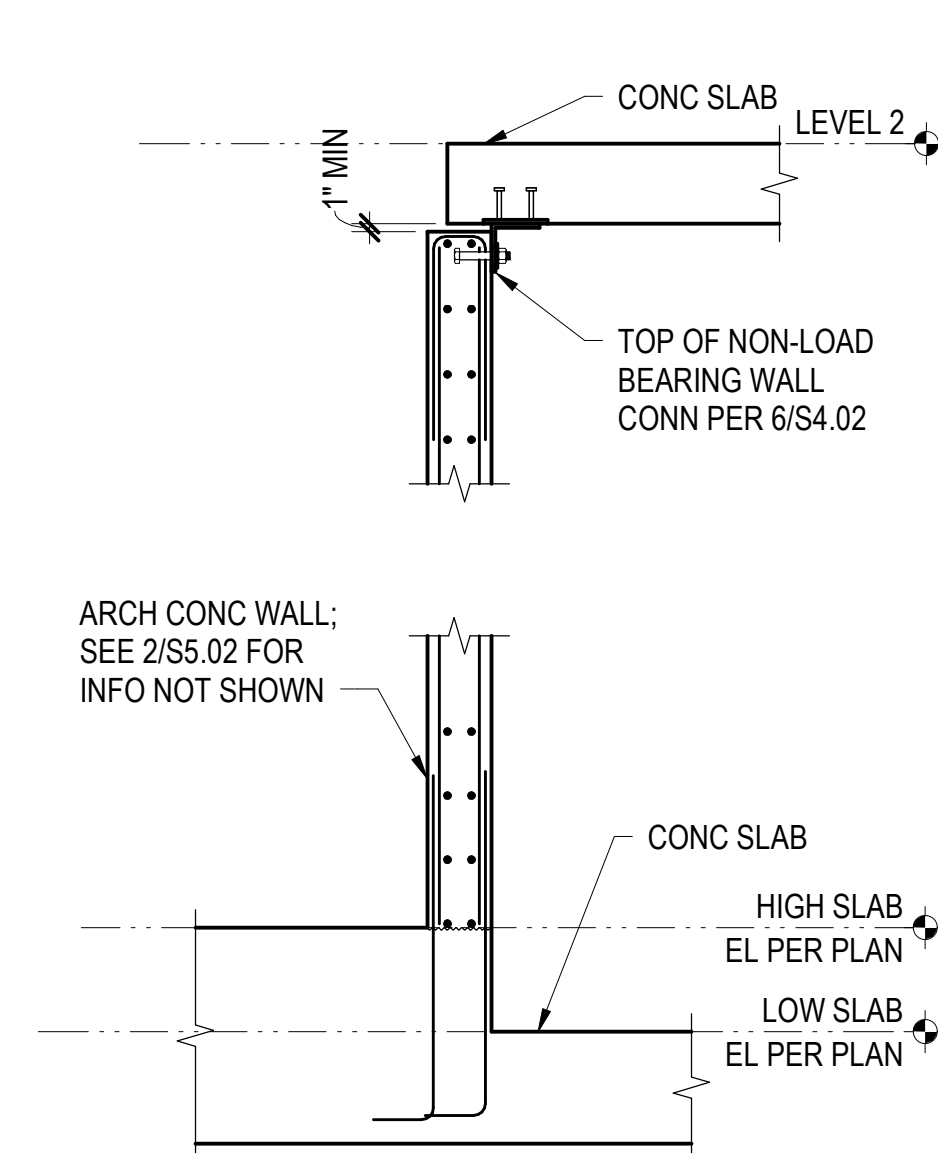
**TOWER B - STEEL COLUMN SCHEDULE**

**TOWER B STEEL COLUMN SCHEDULE**

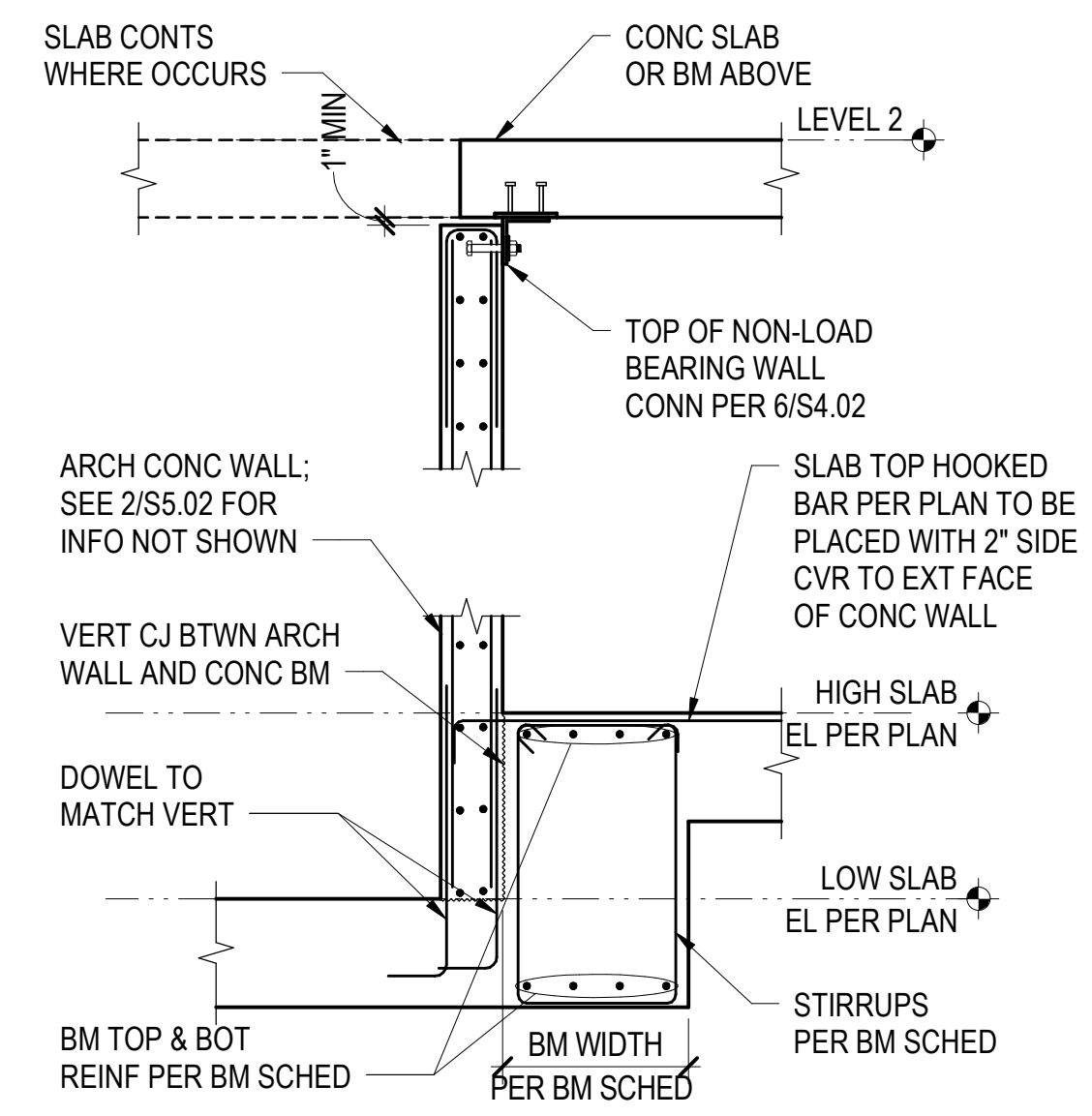
**S4.B.10**



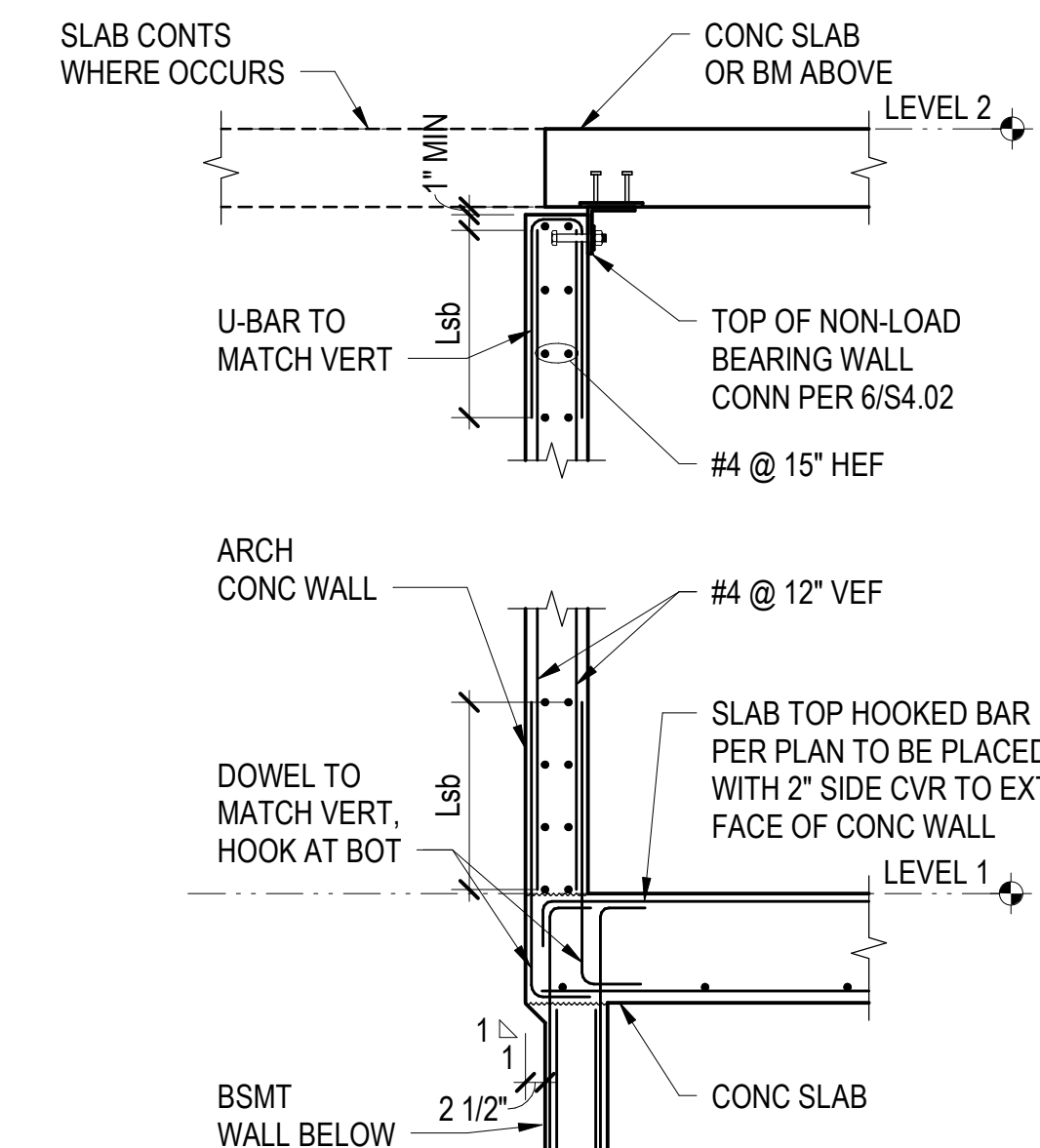
2 ARCHITECTURAL CONCRETE WALL  
1/2" = 1'-0"



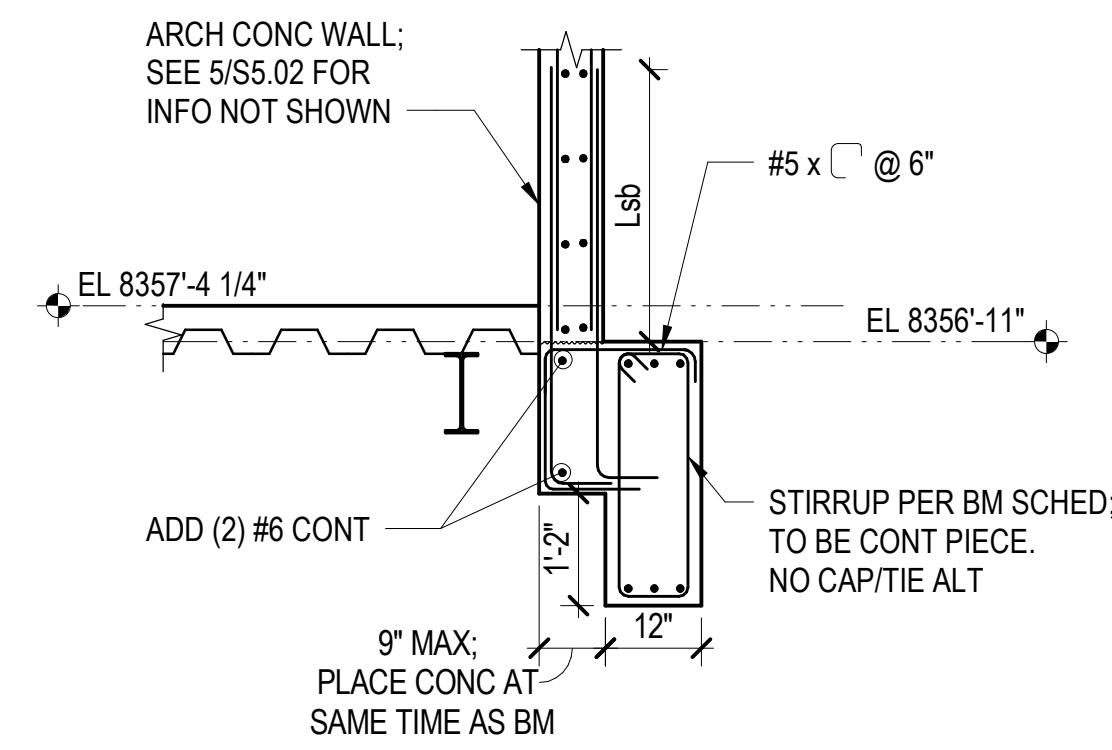
3 ARCHITECTURAL CONCRETE WALL AT SLAB STEP  
1/2" = 1'-0"



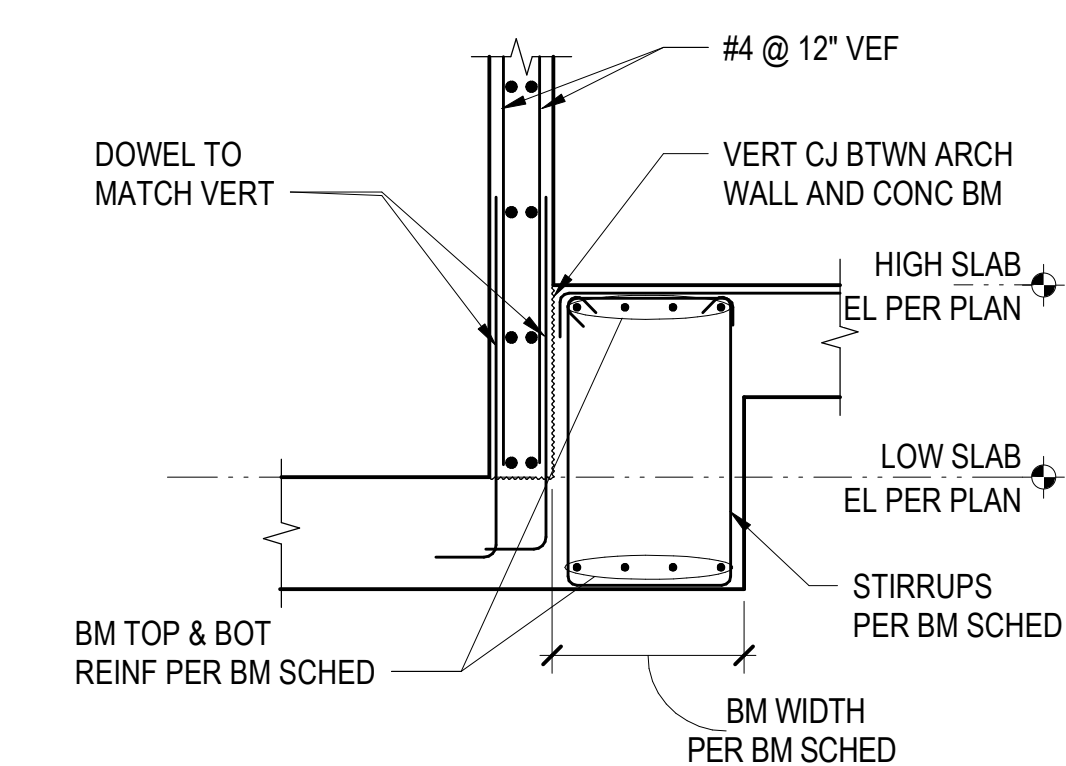
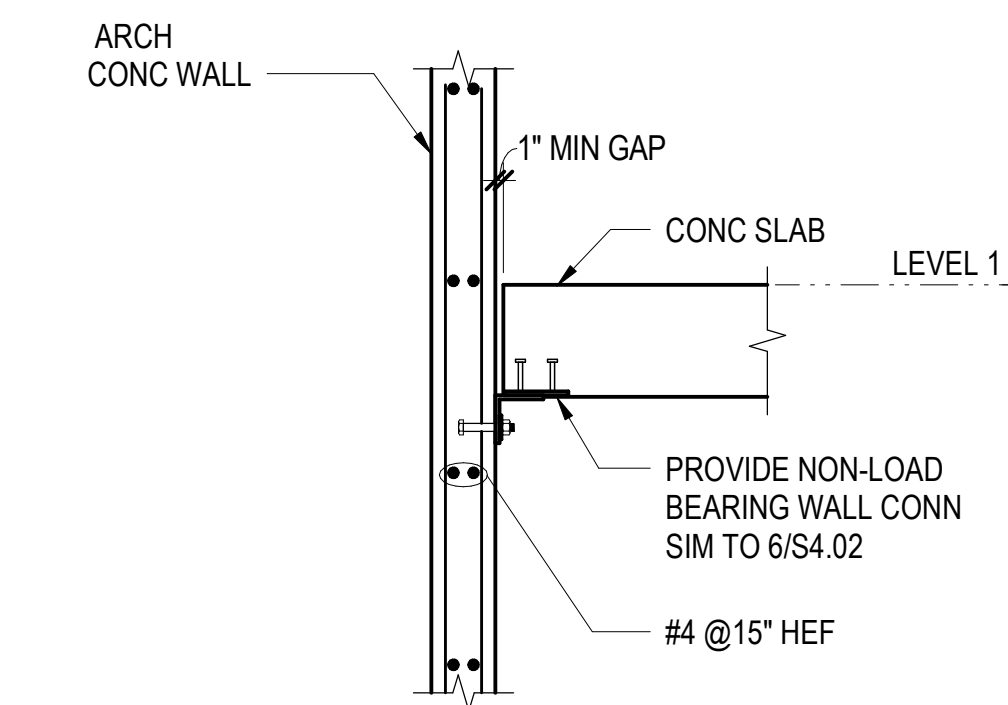
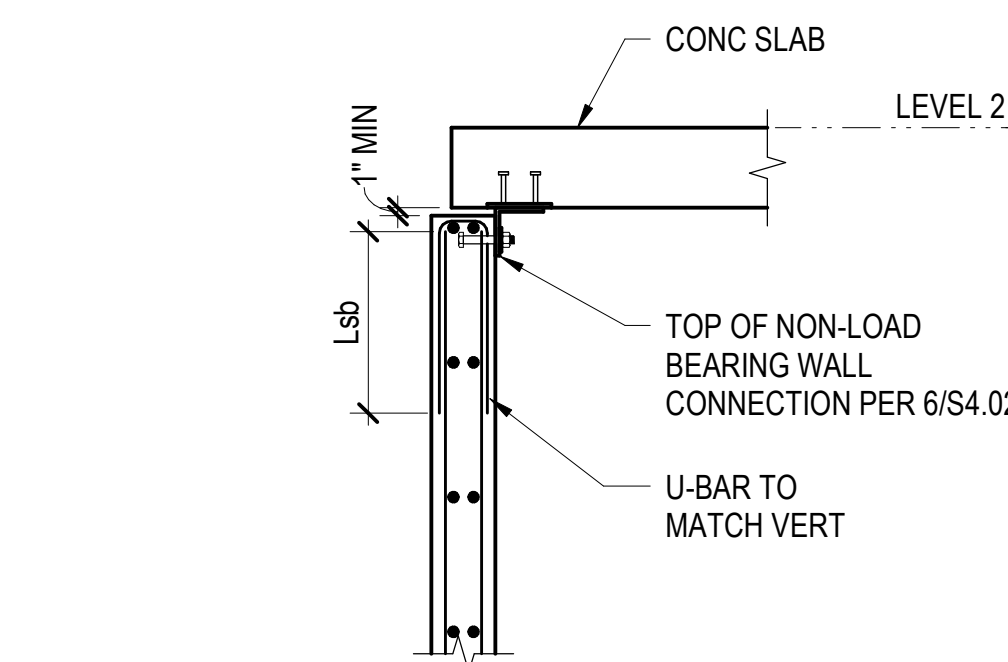
4 ARCHITECTURAL CONCRETE WALL AT STEP BEAM  
1/2" = 1'-0"



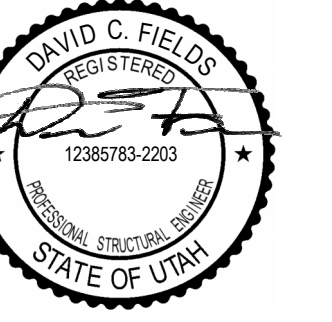
5 ARCHITECTURAL CONCRETE WALL ABOVE BASEMENT WALL  
1/2" = 1'-0"



8 ARCHITECTURAL CONCRETE WALL AT FLOATING BEAM  
1/2" = 1'-0"



20 TWO-STORY ARCHITECTURAL CONCRETE WALL  
1/2" = 1'-0"



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principal architect \_\_\_\_\_  
project manager \_\_\_\_\_  
drawn by \_\_\_\_\_  
checked by \_\_\_\_\_  
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TOWER A & B  
CONCRETE SECTIONS AND DETAILS

S5.02

